CITY OF DOVER DEPARTMENT OF COMMUNITY SERVICES CONSTRUCTION PLANS WHITTIER STREET OVER COCHECO RIVER

FEDERAL PROJECT NO. X-A002(794) N.H.D.O.T. PROJECT NO. 15402 FEBRUARY 2016

INDEX OF SHEETS

DESIGN SPEED

LENGTH OF PROJECT

- TITLE PAGE
- TYPICAL ROADWAY SECTION
- SUMMARY OF QUANTITIES AND NOTES

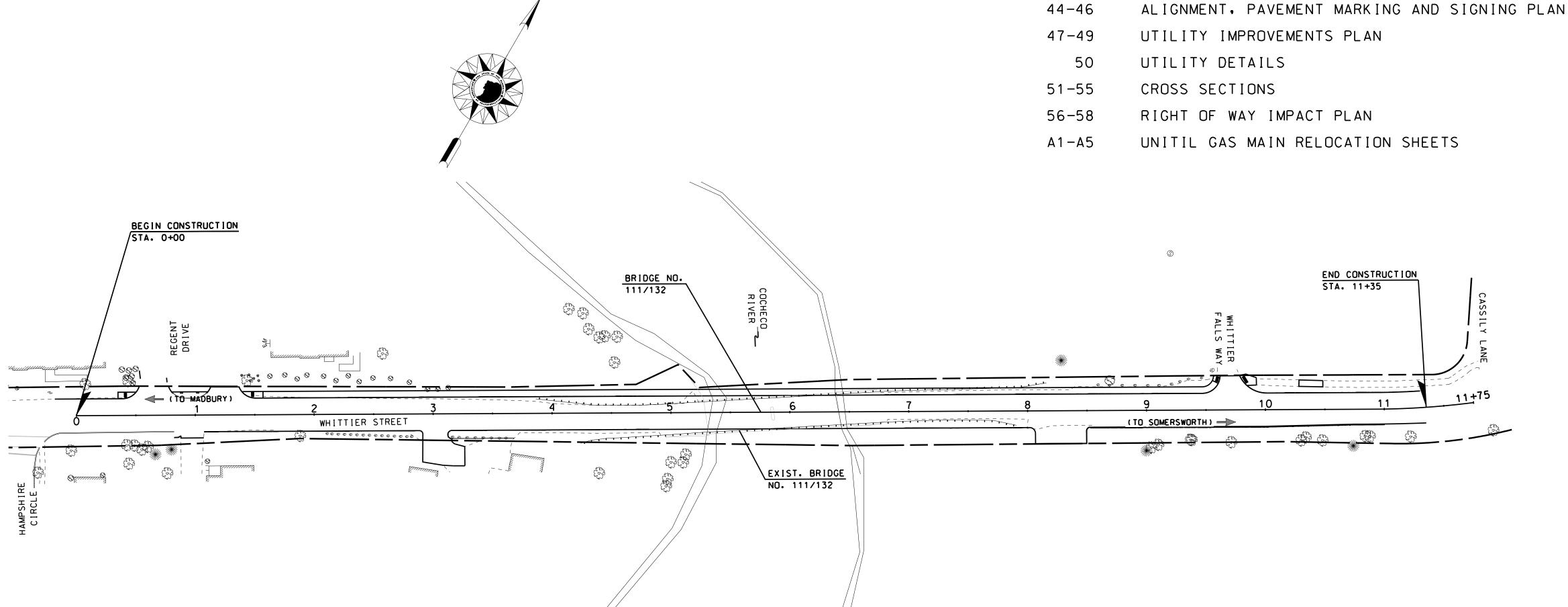
AVERAGE DAILY TRAFFIC 20 10
AVERAGE DAILY TRAFFIC 20 32
PERCENT OF TRUCKS

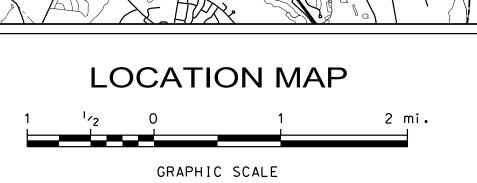
- DRIVEWAY MATCH DETAILS
- ROADWAY DETAILS
- BRIDGE PLANS 6-40
- GENERAL PLAN & PROFILE

DESIGN DATA

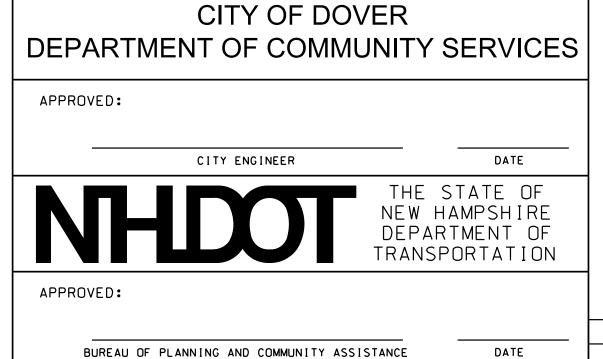
5,000 VPD 7,400 VPD 3% 30

0.22 MI





CITY OF DOVER COUNTY OF STRAFFORD NOT TO SCALE



BUREAU OF PLANNING AND COMMUNITY ASSISTANCE

ROADWAY PLANS PREPARED BY THE Louis Berger Group, INC. Manchester, New Hampshire

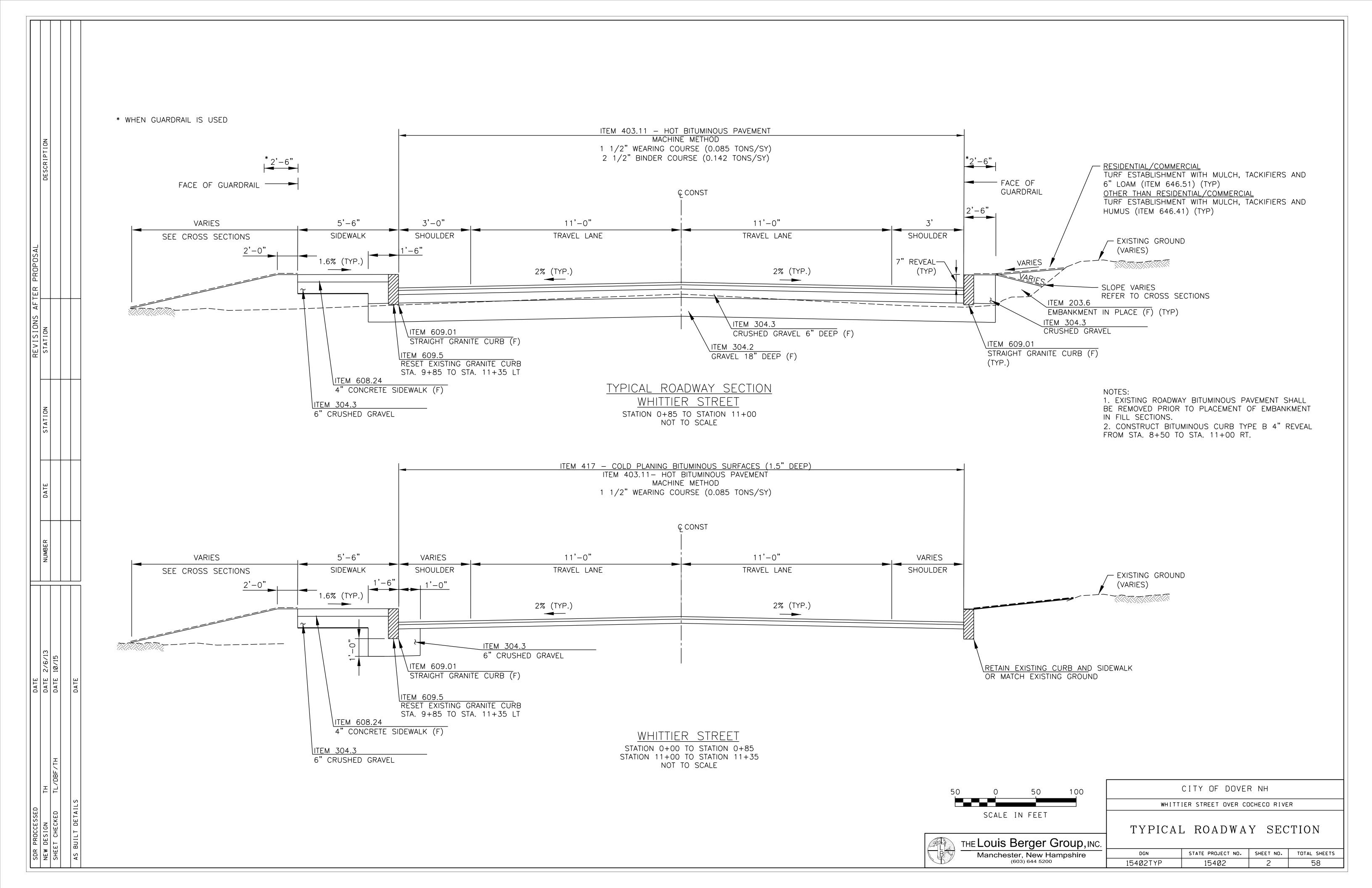
TIMOTHY C. HIGGINSON No. 11928

15402

BRIDGE PLANS PREPARED BY THE Louis Berger Group, INC. Manchester, New Hampshire



FEDERAL PROJECT NO. STATE PROJECT NO. SHEET NO. TOTAL SHEETS 58 X-A002(794) 15402



GENERAL NOTES 1. ALL WORK SHALL BE IN CONFORMANCE WITH CURRENT NHDOT STANDARD SPECIFICATIONS. STANDARD PLANS AND DETAILS. 2. ENGINEER SHALL BE DEFINED AS THE RESIDENT ENGINEER / OWNER'S REPRESENTATIVE. WHO IS RESPONSIBLE FOR ENGINEERING SUPERVISION OF THE CONSTRUCTION, ACTING DIRECTLY OR THROUGH HIS DULY AUTHORIZED REPRESENTATIVES. 3. THE CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY DIG-SAFE PRIOR TO CONSTRUCTION. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING THE HORIZONTAL AND VERTICAL CONTROL THROUGHOUT THE PROJECT. 5. THE CONTRACTOR SHALL BE RESPONSIBLE TO NOTIFY RESIDENTS OF ANY WORK RESTRICTING ACCESS TO ANY DRIVEWAY 24 HOURS IN ADVANCE. 6. SURVEY DATA FOR THIS PROJECT WAS COLLECTED BY PROMISED LAND SURVEY LLC. 25 NASHUA ROAD, SUITE B1, LONDONDERRY, NH 03053 IN JUNE 2011. COORDINATES ARE NEW HAMPSHIRE STATE PLANE COORDINATES OF N.A.D.1983/1986 AND THE BEARINGS ARE GRID. ELEVATIONS ARE REFERENCED TO NGVD 29. 7. REFER TO NHOOT STANDARD PLANS FOR MAILBOX ASSEMBLY AND CONSTRUCTION DETAILS. 8. THE COCHECO RIVER IS ALSO ACCEPTABLY SPELLED AS COCHECHO RIVER. SIDEWALK NOTES 1. THE MAXIMUM RUNNING SLOPE OF ANY SIDEWALK CURB RAMP IS 12:1, THE MAXIMUM CROSS SLOPE IS 2%. THE SLOPE OF THE LANDING SHALL NOT EXCEED 2% IN ANY DIRECTION. 2. TRANSITIONS SHALL BE FLUSH AND FREE OF ABRUPT CHANGES. ROADWAY SHOULDER SLOPES ADJOINING SIDEWALK CURB RAMPS SHALL BE A MAXIMUM OF 5% (FULL WIDTH) FOR A DISTANCE OF 2' FROM THE ROADWAY CURBLINE. 3. INTERCEPT DRAINAGE ALONG THE CURB IN ADVANCE OF SIDEWALK CURB RAMPS OR LANDINGS. CATCH BASINS, MANHOLES, ETC, SHALL NOT BE LOCATED IN, OR AT THE BASE OF, SIDEWALK CURB RAMPS OR LANDINGS. 4. THE BOTTOM OF THE SIDEWALK CURB RAMP OR LANDING, EXCLUSIVE OF THE FLARED SIDES, SHALL BE WHOLLY CONTAINED WITHIN THE CROSSWALK MARKINGS. 5. THE SURFACE OF A PERPENDICULAR SIDEWALK CURB RAMP OR THE LANDING OF A PARALLEL SIDEWALK CURB RAMP SHALL CONTRAST VISUALLY WITH THE ADJOINING SIDEWALK SURFACE, EITHER ASPHALT/LIGHT-COLORED CONCRETE OR LIGHT-COLORED CONCRETE/DARK-STAINED CONCRETE. THE CONCRETE SURFACE SHALL BE SLIP RESISTANT. 6. INSTALL DETECTABLE WARNINGS (TRUNCATED DOMES) AT THE LOCATIONS SHOWN. DETECTABLE WARNINGS SHALL MEASURE 2' IN THE DIRECTION OF TRAVEL AND EXTEND THE FULL WIDTH OF THE SIDEWALK RAMP, AND THE EDGE OF THE NEAREST CURBLINE SHALL BE LOCATED 1.5' - 2.0' FROM THE FACE OF THE CURBLINE. MAINTENANCE OF TRAFFIC (ITEM 619.1) 1. CONTRACTOR SHALL DEVELOP A DETOUR PLAN AND SIGN PACKAGE SUBMITTAL FOR CITY REVIEW AND APPROVAL. ALL ITEMS ASSOCIATED WITH DETOUR PLAN AND SIGN PACKAGE SHALL BE SUBSIDIARY TO ITEM 619.1. 2. THE ANTICIPATED DETOUR ROUTE WILL UTILIZE 6TH STREET, GROVE STREET, 4TH STREET AND WASHINGTON STREET. EROSION CONTROL NOTES 1. SILT FENCE SHALL BE INSTALLED AS DIRECTED BY THE ENGINEER PRIOR TO CONSTRUCTION. 2. EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE CLEANED, REPLACED AND AUGMENTED AS NECESSARY TO PREVENT SEDIMENTATION BEYOND PROJECT LIMITS THROUGHOUT THE PROJECT DURATION. 3. EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSPECTED AND REPAIRED AFTER EACH RAIN EVENT. 4. TEMPORARY EROSION AND SEDIMENTATION CONTROL MEASURES SHALL REMAIN UNTIL DATE DATE DATE PERMANENT VEGETATION HAS BEEN ESTABLISHED AND APPROVED BY THE ENGINEER. N H H

DRAINAGE NOTES

NOTES FOR DRAINAGE CONSTRUCTION:

- ALL STATIONS AND OFFSETS PROVIDED IN THESE NOTES
 ARE TO THE CENTERLINE OF THE STRUCTURE AND DO NOT
 ALWAYS REFLECT THE CENTER OF THE GRATE OR COVER.
- IN GUARDRAIL AREAS THE STRUCTURES ARE DESIGNED TO USE ECCENTRIC CONES AND HAVE THE STRUCTURE CENTERLINE SET OFF THE FACE OF GUARDRAIL AS FOLLOWS:
 - 1.7' FOR 4' DIAMETER STRUCTURES2.2' FOR 5' DIAMETER STRUCTURES
- ALL TOP OF GRATE ELEVATIONS IN PAVEMENT ARE SET 0.5" BELOW NORMAL ELEVATION AT CENTERLINE OF GRATE.
- REFER TO NHOOT STANDARD PLANS AND SPECIFICATIONS FOR DETAILS REGARDING GRATE AND CATCH TYPE AND DETAILS.



 $\left\langle \begin{array}{c} 2 \end{array} \right\rangle$

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3 STA 4+08.00, LT 87.0' TO STA 3+70.60, LT 13.0'
CONSTRUCT 81 LF X 15" PE PIPE (TYPE S)
CONSTRUCT 18" ALUMINIZED STEEL END SECTION @ +08.00, LT 87.0'
15" INV. OUT = 50.05

CONSTRUCT CB-B 5' DIA. @ 3+70.60, LT 13.0'
15" INV. OUT = 50.70
15" INV. IN = 52.10 (SW)
15" INV. IN = 52.10 (NE)
15" INV. IN = 52.10 (SE)

GRATE ELEV. = 58.30

CONSTRUCT 4' WIDE X 16' LONG STONE LINED CHANNEL WITH STONE FILL CLASS C

(SEE ROADWAY DETAILS SHEET)

58.30

4 STA 3+70.60, LT 13.0' TO STA 3+70.60, RT 13.0'
CONSTRUCT 22 LF X 15" RCP 2000D
CONSTRUCT CB-B 4' DIA. @ 3+70.60, RT 13.0'
15" INV. OUT = 52.40
15" INV. IN = 52.65 (SW)
15" INV. IN = 52.65 (NE)

5 STA 3+70.60, RT 13.0' TO STA 3+96.00, RT 12.3'
CONSTRUCT 22 LF X 15" PE PIPE (TYPE S)
CONSTRUCT CB-B 4' DIA. @ 3+96.00, RT 12.3'
WITH ECCENTRIC CONE
15" INV. OUT = 52.80
GRATE ELEV. = 58.38

GRATE ELEV. =

STA 3+70.60, LT 13.0' TO STA 3+96.00, LT 13.0'
CONSTRUCT 22 LF X 15" PE PIPE (TYPE S)
CONSTRUCT CB-B 4' DIA. @ 3+96.00, LT 13.0'
15" INV. OUT = 52.80
GRATE ELEV. = 58.38

GUARDRAIL CONSTRUCTION NOTES

1 STA 3+77.32, RT TO STA 4+52.32, RT
CONSTRUCT 25FT EAGRT STA 3+77.32, RT TO STA 4+02.32, RT
CONSTRUCT W-BEAM GUARDRAIL STA 4+02.32, RT TO STA 4+52.32, RT
CONNECT TO BRIDGE RAIL AT STA 4+52.32, RT

2 STA 7+72.18, RT TO STA 7+97.18, RT
CONSTRUCT 25FT EAGRT STA 7+72.18, RT TO STA 7+97.18, RT
CONSTRUCT W-BEAM GUARDRAIL STA 7+22.18, RT TO STA 7+72.18, RT
CONNECT TO BRIDGE RAIL AT STA 7+22.18, RT

3 STA 2+64.82, LT TO STA 4+52.32, LT
CONSTRUCT G2 END UNIT STA 2+64.82, LT TO STA 2+77.32, LT
CONSTRUCT W-BEAM GUARDRAIL STA 2+77.32, LT TO STA 4+52.32, LT
CONNECT TO BRIDGE RAIL AT STA 4+52.32, LT

STA 7+22.18, LT TO STA 8+97.18, LT

CONSTRUCT 25FT EAGRT STA 8+72.18, LT TO STA 8+97.18, LT

CONSTRUCT W-BEAM GUARDRAIL STA 7+22.18, LT TO STA 8+72.18, LT

CONNECT TO BRIDGE RAIL AT STA 7+22.18, LT

7 STA 3+70.60, RT 13.0' TO STA 3+46.00, RT 13.0' CONSTRUCT 21 LF X 15" PE PIPE (TYPE S) CONSTRUCT CB-B 4' DIA. @ 3+46.00, RT 13.0' 15" INV. OUT = 52.80 15" INV. IN = 53.05

GRATE ELEV. = 58.39

TA STA 3+46.00, RT 13.0' TO STA2+78.00, RT 13.0' CONSTRUCT 64 LF X 15" PE PIPE (TYPE S) CONSTRUCT CB-B 4' DIA. @ 2+78.00, RT 13.0' 15" INV. OUT = 53.75

GRATE ELEV. = 59.32

8 STA 3+70.60, LT 13.0' TO STA 3+46.00, LT 13.0' CONSTRUCT 21 LF X 15" PE PIPE (TYPE S) CONSTRUCT CB-B 4' DIA. @ 3+46.00, LT 13.0' 15" INV. OUT = 52.80 GRATE ELEV. = 58.39

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STA 6+85.00, RT 25.0' TO STA 7+00.00, RT 11.8'

CONSTRUCT 18 LF X 15" PE PIPE (TYPE S)

CONSTRUCT 18" ALUMINIZED STEEL END SECTION @ +85.00, RT 25.0'

15" INV. OUT = 59.50

CONSTRUCT CB-F 4' DIA. @ 7+00.00, RT 11.8'

WITH ECCENTRIC CONE

15" INV. OUT = 59.75

15" INV. IN = 60.00

GRATE ELEV. = 66.43

11 STA 7+00.00, RT 11.8' TO STA 7+00.00, LT 13.0'
CONSTRUCT 21 LF X 15" RCP 2000D
CONSTRUCT CB-F 4' DIA @ 7+00.00, LT 13.0'
15" INV. OUT = 60.25
15" INV. IN = 60.50
GRATE ELEV. = 66.42

STA 7+00.00, LT 13.0' TO STA 8+70.00, LT 13.0'
CONSTRUCT 166 LF X 15" PE PIPE (TYPE S)
CONSTRUCT CB-F 4' DIA @ 8+70.00, LT 13.0'
15" INV. OUT = 68.95
15" INV. IN = 69.20 (SE)
15" INV. IN = 69.20 (NE)

GRATE ELEV. = 74.96

13 STA 8+70.00, LT 13.0' TO STA 10+03.00, LT 13.0' CONSTRUCT 129 LF X 15" PE PIPE (TYPE S) CONSTRUCT CB-F 4' DIA @ 10+03.00, LT 13.0' 15" INV. OUT = 80.20 GRATE ELEV. = 86.32

14 STA 8+70.00, LT 13.0' TO STA 8+70.00, RT 13.0'
CONSTRUCT 22 LF X 15" RCP 2000D
CONSTRUCT CB-F 4' DIA @ 8+70.00, RT 13.0'
15" INV. OUT = 69.40
GRATE ELEV. = 74.96

DRAINAGE REMOVAL NOTES

R1 STA. 4+07.0, LT 83.5' TO STA. 3+63.0, RT 14.8'
REMOVE 28 LF X 12" EXIST. HDPE
REMOVE 78 LF X 12" EXIST.HDPE (SUBSIDIARY)
REMOVE EXIST. CB @ 3+63.0, RT 14.8'

HEIVI NO.	HEIVI DESCRIPTION	UNII	QUANTITY
201.1	CLEARING AND GRUBBING (F)	A	0.13
201.21	REMOVING SMALL TREES	EA	4
202.41	REMOVE EXISTING PIPE 0-24" DIA	LF	30
202.5	REMOVAL OF CATCH BASINS, DROP INLETS, AND MANHOLES	EA	1
202.7	REMOVAL OF GUARD RAIL	LF	525
203.1	COMMON EXCAVATION	CY	1,600
203.2	ROCK EXCAVATION	CY	160
203.5555	GUARDRAIL 25' EAGRT PLATFORM	U	3
203.6	EMBANKMENT-IN-PLACE (F)	CY	1,415
206.1	COMMON STRUCTURE EXCAVATION	CY	11
206.19	COMMON STRUCTURE EXCAVATION - EXPLORATORY	CY	12
206.2	ROCK STRUCTURE EXCAVATION	CY	4
214	FINE GRADING	U	1
304.2	GRAVEL (F)	CY	1,300
304.3	CRUSHED GRAVEL (F)	CY	567
304.35	CRUSHED GRAVEL FOR DRIVES	CY	80
403.11	HOT BITUMINOUS PAVEMENT, MACHINE METHOD	TON	690
403.12	HOT BITUMINOUS PAVEMENT, HAND METHOD	TON	80
	· · · · · · · · · · · · · · · · · · ·		
403.6	PAVEMENT JOINT ADHESIVE	LF 6Y	2,650
417	COLD PLANING BITUMINOUS SURFACES	SY	450
585.2	STONE FILL, CLASS B	CY	190
585.3	STONE FILL, CLASS C	CY	8
593.411	GEOTEXTILE; PERM CONTROL CL.1, NON-WOVEN	SY	1,550
603.00215	15" R.C. PIPE, 2000D	LF	70
603.36118	18" ALUMINZED STEEL END SECTION	EA	2
603.82215	15" PE PIPE (TYPE S)	LF	555
604.0007	POLYETHYLENE LINER	EA	12
604.12	CATCH BASINS TYPE B, 4 FOOT DIAMETER	U	6.1
604.125	CATCH BASINS TYPE B, 5 FOOT DIAMETER	U	1.3
		U	
604.16	CATCH BASINS TYPE F, 4 FOOT DIAMETER		5.5
604.51	RECONSTRUCTING/ADJUSTING SEWER MANHOLES	LF	4.0
606.12	BEAM GUARDRAIL (STANDARD SECTION - STEEL POST)	LF	425
606.12551	BEAM GUARDRAIL (TERM. UNIT TYPE EAGRT, TL2-25')(STEEL POST)	U	3
606.127	BEAM GUARDRAIL (TERMINAL UNIT TYPE G-2) (STEEL POST)	U	1
606.417	PORTABLE CONCRETE BARRIER FOR TRAFFIC CONTROL	LF	60
607.5108	WOOD STOCKADE FENCE, 8' HIGH	LF	60
608.24	4" CONCRETE SIDEWALK (F)	SY	400
608.54	DETECTABLE WARNING DEVICE CAST IRON	SY	6
609.01	STRAIGHT GRANITE CURB	LF	1,160
609.02	CURVED GRANITE CURB	LF	1,100
609.5	RESET GRANITE CURB	LF 	108
609.811	BITUMINOUS CURB, TYPE B (4" REVEAL)	LF	250
614.342	4" 2-DUCT STEEL CONDUIT	LF	95
614.512	CONCRETE PULL BOX - 18 INCH	EA	2
615.034	RELOCATING TRAFFIC SIGN - TYPE C	U	5
618.61	UNIFORMED OFFICERS WITH VEHICLE	\$	1
618.7	FLAGGERS	HR	80
619.1	MAINTENANCE OF TRAFFIC	U	1
619.253	PORTABLE CHANGEABLE MESSAGE SIGN (UNIT WEEK)	UWK	32
622.1	STEEL WITNESS MARKERS	EA	3
628.2	SAWED BITUMINOUS PAVEMENT	LF	250
632.0104	RETROREFLECT. PAINT PAVE. MARKING, 4" LINE	LF	4,450
632.0106	RETROREFLECT. PAINT PAVE. MARKING, 6" LINE	LF	210
632.0112	RETROREFLECT. PAINT. PAVE. MARKING, 12" LINE	LF	25
645.3	EROSION STONE	TON	100
645.531	SILT FENCE	LF	2,000
645.7	STORMWATER POLLUTION PREVENTION PLAN	U	1
645.71	MONITORING SWPPP AND EROSION AND SEDIMENT CONTROLS	HR	250
646.41	TURF ESTABLISHMENT WITH MULCH, TACKIFIERS & HUMUS	SY	800
646.51	TURF ESTABLISHMENT WITH MULCH, TACKIFIERS & LOAM	SY	600
	·		
650.2	LANDSCAPING MALL DOVI CLIPDOPT ASSEMBLIES	U	1
670.066	MAILBOX SUPPORT ASSEMBLIES	EA	2
670.95	TEMPORARY SAFETY FENCE	LF	1,000
692	MOBILIZATION	U	1
693	ON-THE-JOB TRAINING OF UNSKILLED WORKERS	\$	1
698.13	FIELD OFFICE TYPE C	MON	12
698.11913	FIELD OFFICE (EQUIPMENT AND SUPPLIES)	MON	12
	MISCELLANEOUS TEMPORARY EROSION AND SEDIMENT CONTROL	\$	1
699	TIND A LITEDIA COSTITUTE CINENTE INCISTON AND SELJOVENI LOUNTRO	1 2	1 1
699 1010.15	FUEL ADJUSTMENT	\$	1

SUMMARY OF ROADWAY QUANTITIES - PARTICIPATING

UNIT | QUANTITY

ITEM DESCRIPTION

ITEM NO.

SUMN	SUMMARY OF ROADWAY QUANTITIES - NON PARTICIPATING												
ITEM NO.	ITEM NO. ITEM DESCRIPTION												
608.13	3" BITUMINOUS SIDEWALK (F)	SY	15										
611.05212	12" CEMENT LINED DUCTILE IRON WATER PIPE , CL.52	LF	590										
611.70012	12" FITTING	EA	13										
611.71001	REPLACE NUTS ON EXISTING WATER VALVE	U	2										
611.71012	12" GATE VALVE WITH BOX	EA	2										
611.74	CHLORINE INJECTION TAP	EA	4										
611.90001	ADJUSTING WATER GATES AND SHUTOFFS SET BY OTHERS	EA	6										
612.99	UTILITY PIPE BRIDGE - GAS MAIN	U	1										

15402QUANT

CITY OF DOVER, NEW HAMPSHIRE

WHITTIER STREET OVER COCHECO RIVER

SUMMARY OF QUANTITIES

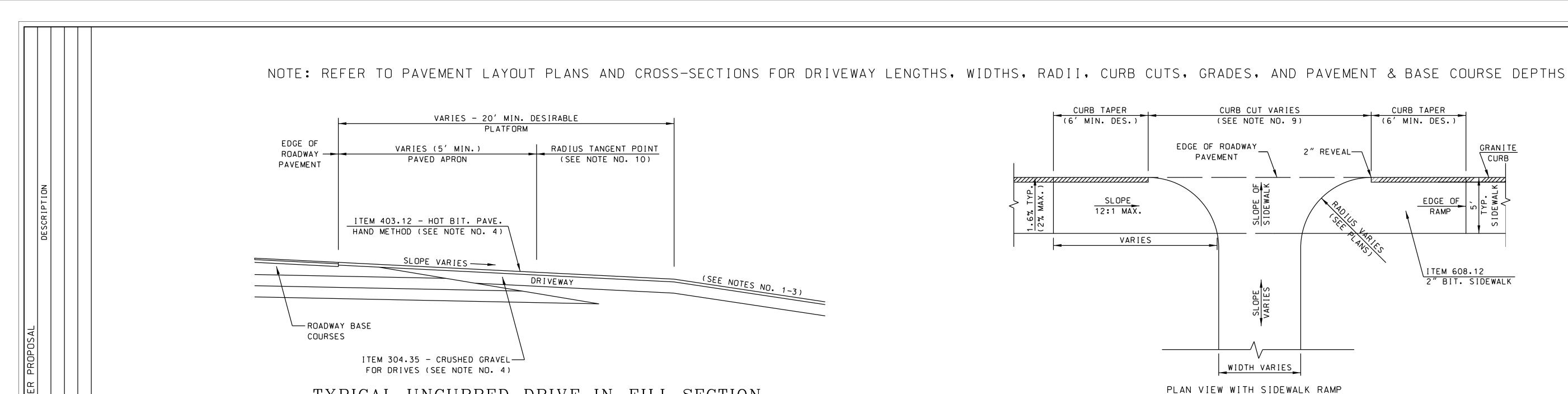


SUMMARY OF QUANTITIES
AND NOTES

DGN STATE PROJECT NO. SHEET NO. TOTAL SHEETS

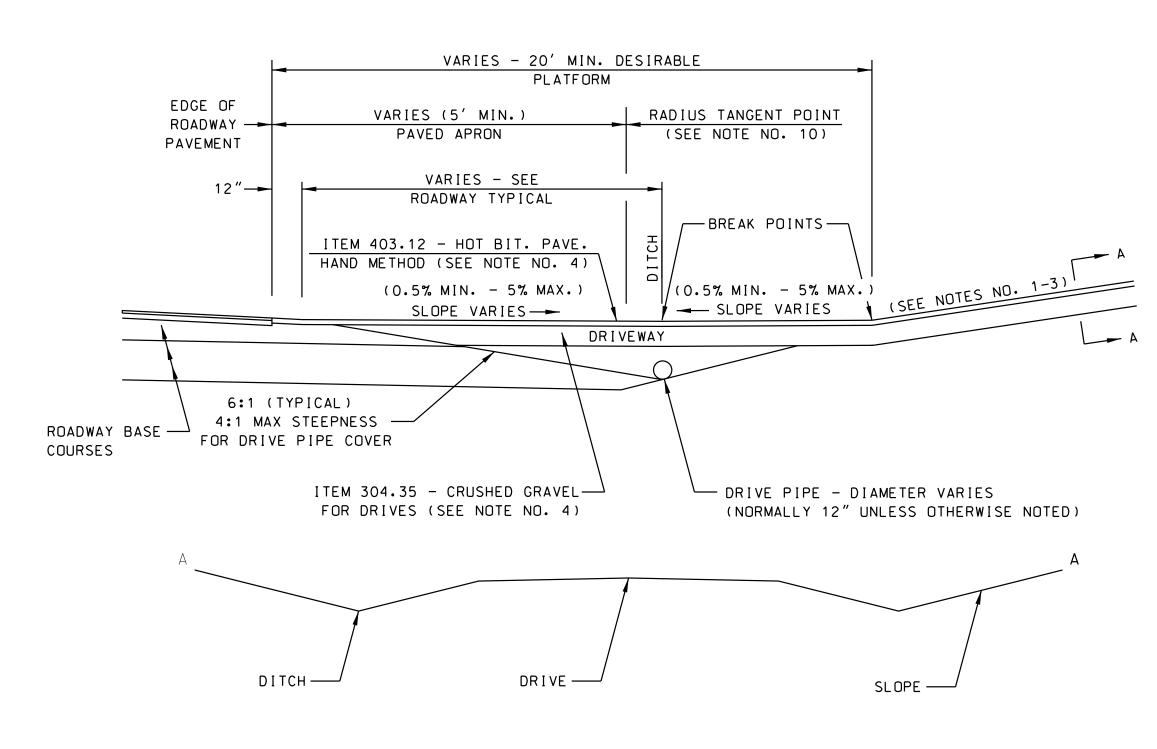
58

15402



TYPICAL UNCURBED DRIVE IN FILL SECTION

NOT TO SCALE



A - A DRIVEWAY CROSS-SECTION

TYPICAL UNCURBED DRIVE IN CUT SECTION

NOT TO SCALE

5. FOR DESIGN CRITERIA AND OTHER ADDITIONAL INFORMATION, REFER TO THE NHDOT DRIVEWAY MANUAL.

ROADWAY BASE —

COURSES

- 6. DITCHES ARE RECOMMENDED FOR UNCURBED DRIVEWAYS IN CUT SLOPES.
- 7. USE SLOPED END SECTIONS ON DRIVE PIPES FOR UNCURBED DRIVEWAYS.
- 8. CURBING CAN BE FLARED TO FIT DRIVE RADII IF APPROPRIATE OR ENDED AS DETAILED ABOVE.
- 9. CURB CUTS FOR RESIDENTIAL DRIVES WITH ANGLES OF ENTRY OF 75°-90° ARE TYPICALLY 25 FT.

GENERAL NOTES

REVISIONS STATION

DATE DATE DATE

- 1. GRADES OF MAJOR ENTRANCES BEYOND THE PLATFORM SHOULD NOT EXCEED 8%.
- 2. GRADES OF OTHER DRIVES BEYOND THE PLATFORM SHOULD NOT EXCEED 15%.
- 3. THE ALGEBRAIC DIFFERENCE BETWEEN TWO ADJACENT GRADES SHOULD NOT EXCEED 10%.
- 4. PAVEMENT & BASE COURSE DEPTHS FOR RESIDENTIAL DRIVES ARE TYPICALLY 8" CRUSHED GRAVEL WITH 3" H.B.P. (HAND METHOD) PLACED IN TWO COURSES. FOR COMMERCIAL/ INDUSTRIAL DRIVES WITH FREQUENT HEAVY TRUCK TRAFFIC. 12" CRUSHED GRAVEL WITH 3" H.B.P. (PLACED IN TWO COURSES) SHOULD BE USED. UNLESS NOTED OTHERWISE IN THE DRIVEWAY CONSTRUCTION NOTES BELOW.

DRIVEWAY CONSTRUCTION NOTES

- 1. STA. 0+65 LT AND STA. 1+25 LT SHALL BE PAVED WITH 3" H.B.P. AND 12" CRUSHED GRAVEL.
- 2. STA. 1+00 RT SHALL BE PAVED WITH 3" H.B.P. AND 8" CRUSHED GRAVEL.
- 3. STA. 3+00 RT SHALL BE PAVED WITH 3" H.B.P. AND 8" CRUSHED GRAVEL.
- 4. STA. 8+25 RT SHALL BE GRAVEL ITEM 304.35 12" THICK.



CURB TAPER

(6' MIN. DES.)

__ 2" REVEAL

∟2″ LIP

VARIES

TRANSITION LIP

IN WIDTH OF CURB

GRANITE

CURB

LIP DETAIL

EDGE OF

ROADWAY

CUT SECTION*

FILL SECTION*

*SEE NOTES NO. 1-3

L BREAK POINT

LITEM 304.35 - CRUSHED GRAVEL FOR

DRIVES (SEE NOTE NO. 4)

PAVEMENT -

GRANITE CURB

CITY OF DOVER, NEW HAMPSHIRE WHITTIER STREET OVER COCHECO RIVER DRIVEWAY MATCH DETAILS DGN STATE PROJECT NO. SHEET NO. TOTAL SHEETS 15402RD00.dgn 15402 58

TYPICAL URBAN CURBED DRIVE IN CUT/FILL SECTION

CURB CUT

DRIVE

END VIEW

VARIES

PLATFORM

VARIES

SIDEWALK

ITEM 403.12 - HOT BIT. PAVE.

→ SLOPE OF SIDEWALK

DRIVEWAY

HAND METHOD (SEE NOTE NO. 4)

NOT TO SCALE

7" REVEAL

CURB TAPER

EDGE OF

PAVEMENT

ROADWAY ---

(SEE DETAIL)

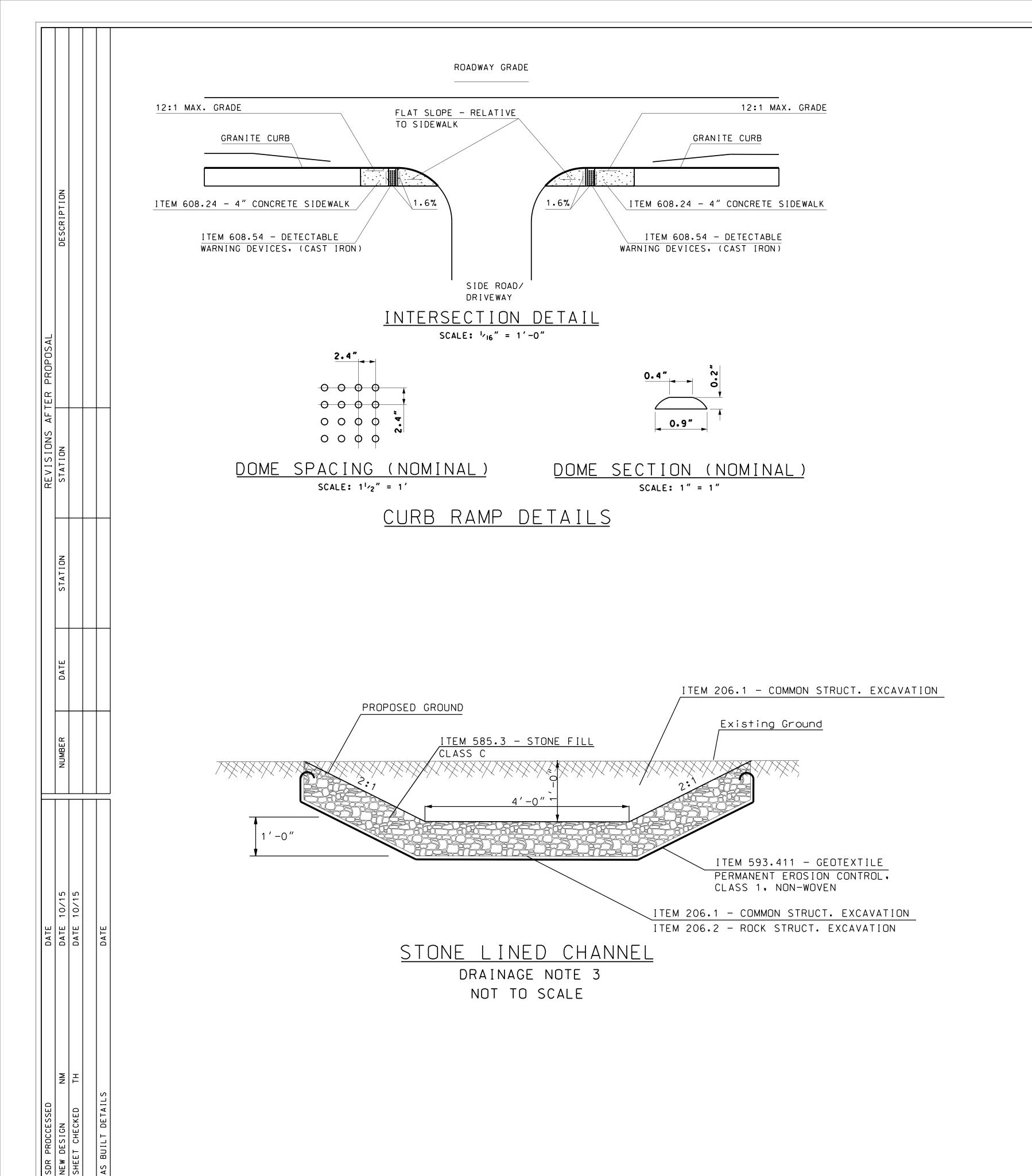
(6' MIN. DES.)

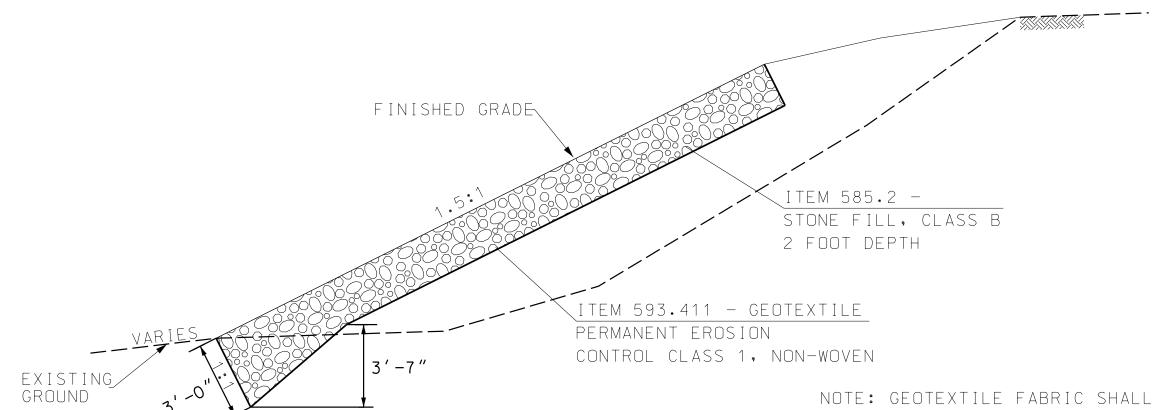
PROF ILE

VARIES

SHOULDER -EDGE

NOT TO SCALE





<u>ARMORED EMBANKMENT SLOPE</u>

FOR 1.5:1 SLOPES

STA. 6+92.5 LT TO STA. 7+60 LT STA. 6+92.5 RT TO STA. 7+50 RT NOT TO SCALE

CITY OF DOVER, NEW HAMPSHIRE

OVERLAP FABRIC PLACED UNDER

PER MANUFACTURER RECOMMENDATIONS.

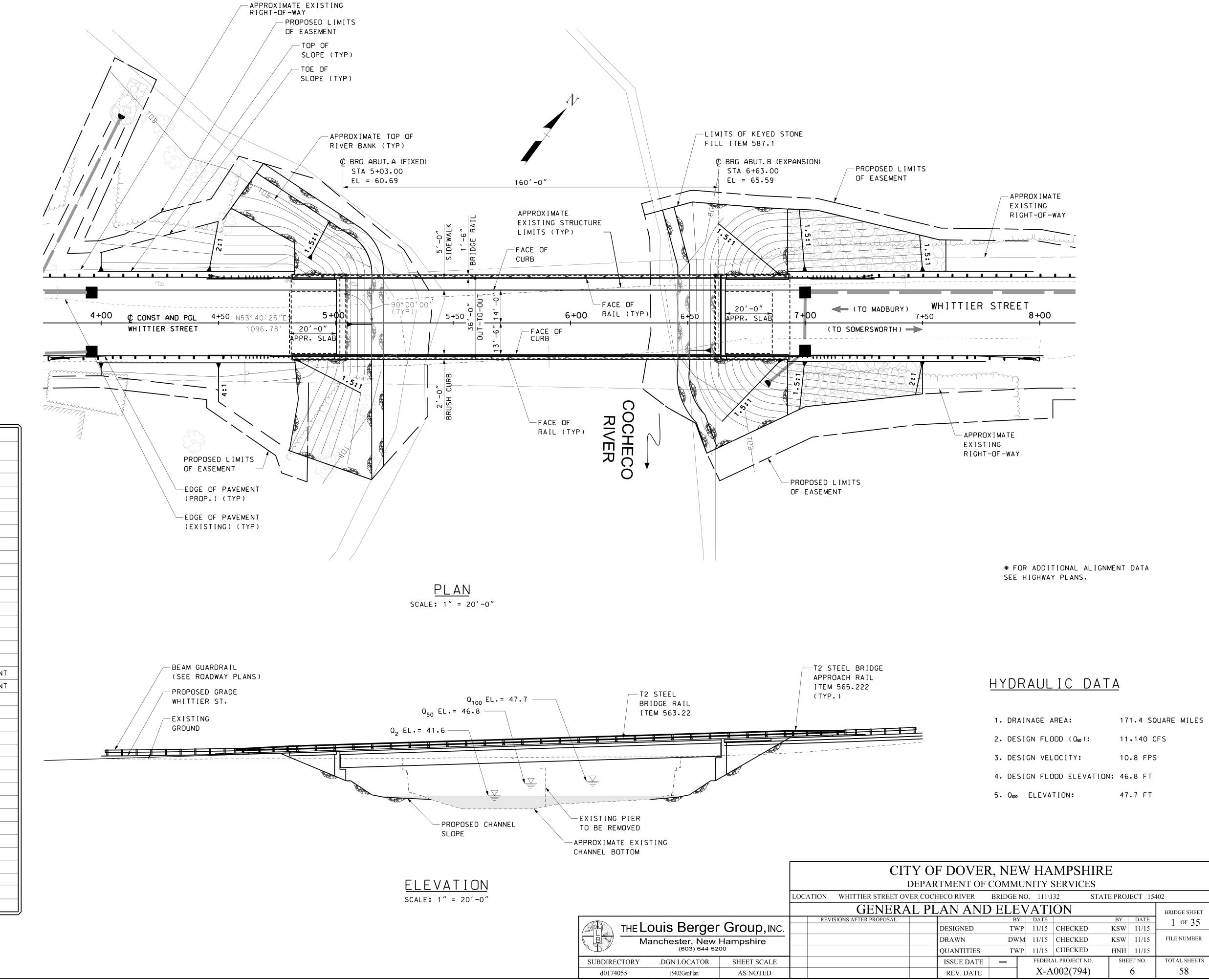
ADJACENT KEYED STONE FILL AS RECOMMENDED

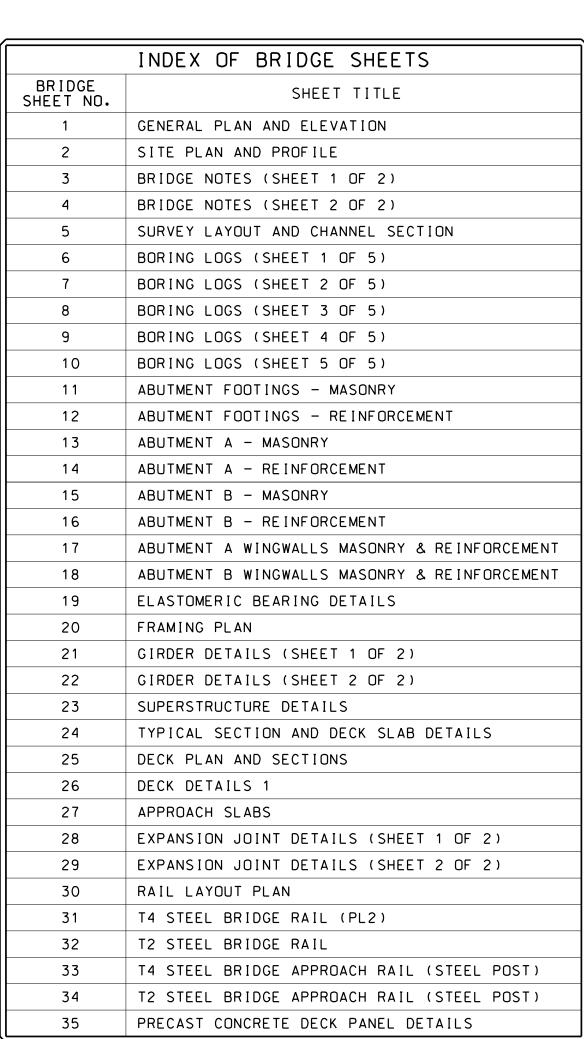
WHITTIER STREET OVER COCHECO RIVER

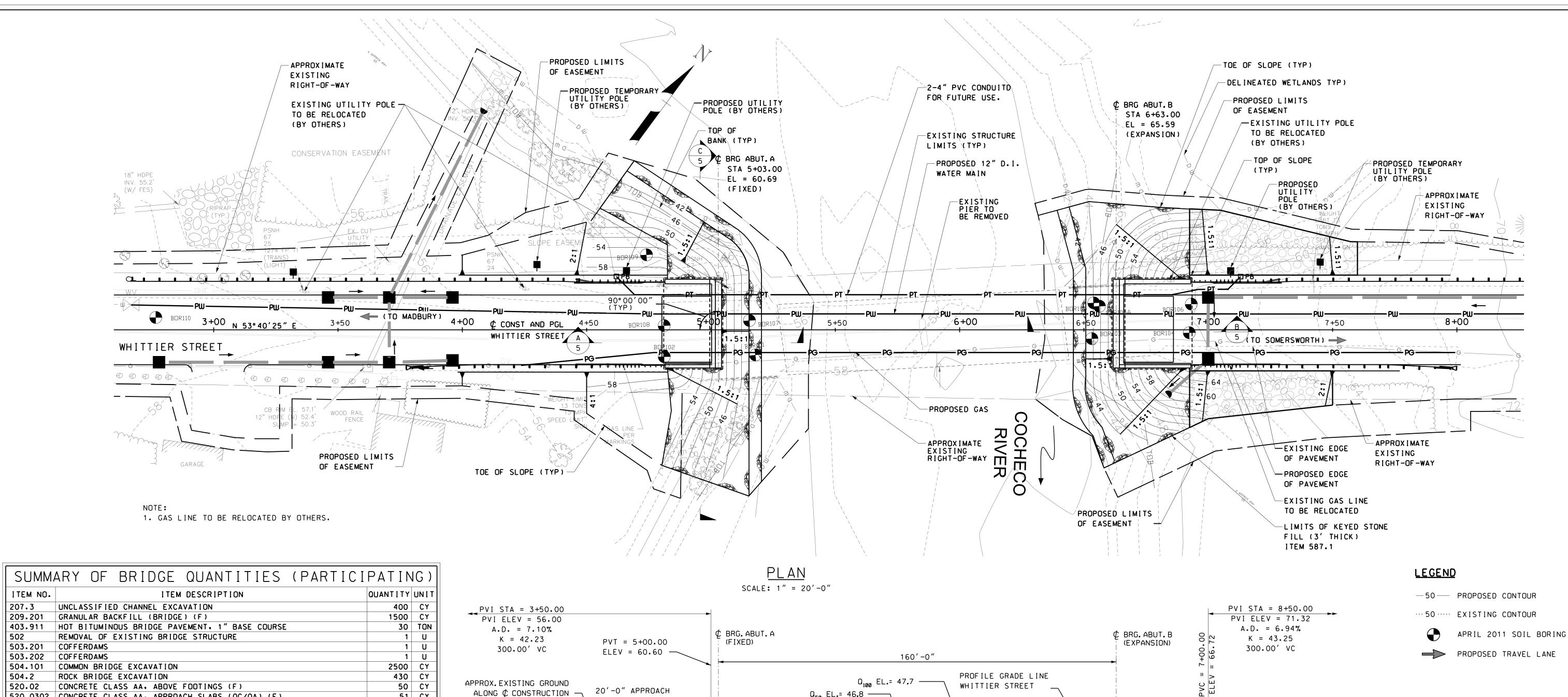
ROADWAY DETAILS



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	DGN	STATE PROJECT NO.	SHEET NO.	TOTAL SHEETS
	15402RD00.dgn	15402	5	58



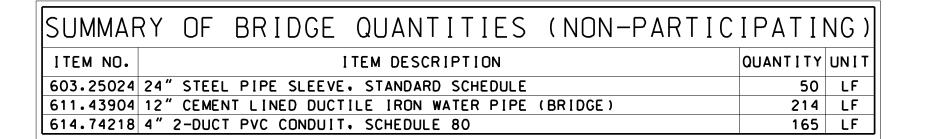


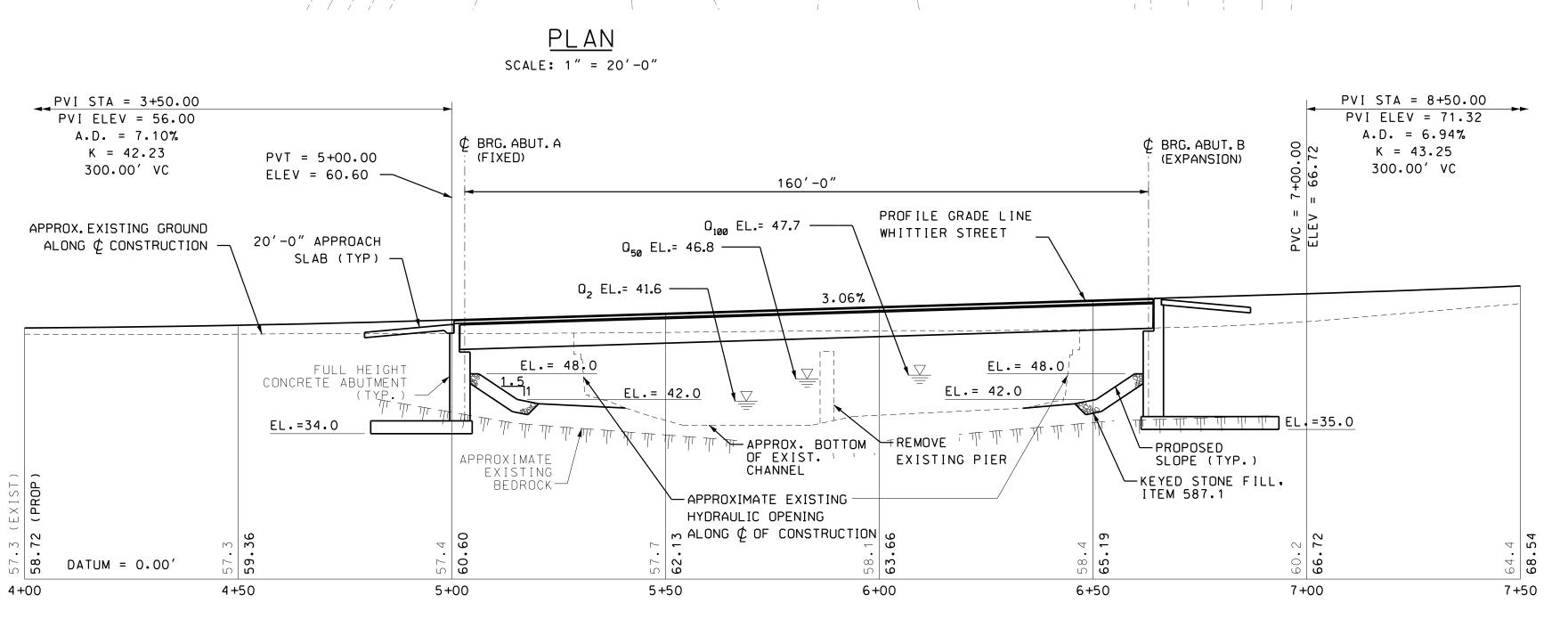


PROF ILE

SCALE: 1'' = 20' - 0''

ITEM NO.	ITEM DESCRIPTION	QUANTITY	UNI
207.3	UNCLASSIFIED CHANNEL EXCAVATION	400	CY
209.201	GRANULAR BACKFILL (BRIDGE) (F)	1500	CY
403.911	HOT BITUMINOUS BRIDGE PAVEMENT. 1" BASE COURSE	30	TO
502	REMOVAL OF EXISTING BRIDGE STRUCTURE	1	U
503.201	COFFERDAMS	1	U
503.202	COFFERDAMS	1	U
504.101	COMMON BRIDGE EXCAVATION	2500	CY
504.2	ROCK BRIDGE EXCAVATION	430	C,
520.02	CONCRETE CLASS AA. ABOVE FOOTINGS (F)	50	C
520.0302	CONCRETE CLASS AA. APPROACH SLABS (QC/QA) (F)	51	C,
520.12	CONCRETE CLASS A ABOVE FOOTINGS (F)	457	C.
520.211	CONCRETE CLASS B. FOOTINGS (ON ROCK)	305	C.
520.70026	CONCRETE BRIDGE DECK (QC/QA) (PANEL OPTION) (F)	220	C
534.3	WATER REPELLENT (SILANE-SILOXANE)	36	GA
538.2	BARRIER MEMBRANE, PEEL AND STICK - VERTICAL SURFACES (F)	48	S
538.6	BARRIER MEMBRANE, HEAT WELDED - MACHINE METHOD (F)	524	S
541.1	PVC WATERSTOPS. NH TYPE 1 (F)	27	LI
541.4	PVC WATERSTOPS. NH TYPE 4 (F)	144	L
541.5	PVC WATERSTOPS. NH TYPE 5 (F)	38	L
544.3	REINFORCING STEEL (CONTRACTOR DETAILED)	100000	L
544.31	REINFORCING STEEL. EPOXY COATED (CONTRACTOR DETAILED)	73000	L
544.7	SYNTHETIC FIBER REINFORCEMENT (F)	358	L
547	SHEAR CONNECTORS (F)	910	Ε
548.21	ELASTOMERIC BEARING ASSEMBLIES (F)	10	Ε
550.1	STRUCTURAL STEEL (F)	325000	L
559.41	ASPHALTIC PLUG FOR CRACK CONTROL (F)	28	L
561.1001	PREFABRICATED STRIP SEAL EXPANSION JOINT (F)	36	L
562.1	SILICONE JOINT SEALANT	165	L
563.22	BRIDGE RAIL T2	212	L
563.24	BRIDGE RAIL T4	212	L
565.222	BRIDGE APPROACH RAIL T2 (STEEL POSTS)	2	ι
565.242	BRIDGE APPROACH RAIL T4 (STEEL POSTS)	2	ι
587.1	KEYED STONE FILL	950	С
609.3	STRAIGHT GRANITE CURB (BRIDGE)	164	L
1010.42	QUALITY CONTROL / QUALITY ASSURANCE (QC/QA) FOR CONCRETE	1	4





CITY OF DOVER, NEW HAMPSHIRE

DEPARTMENT OF COMMUNITY SERVICES

TWP 11/15 CHECKED

DWM 11/15 CHECKED

TWP 11/15 CHECKED

FEDERAL PROJECT NO.

X-A002(794)

STATE PROJECT 15402

KSW 11/15

KSW 11/15

HNH 11/15

SHEET NO.

BRIDGE SHEET

2 of 35

FILE NUMBER

TOTAL SHEETS

58

LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132

REVISIONS AFTER PROPOSAL

THE Louis Berger Group, INC.

Manchester, New Hampshire (603) 644 5200

SHEET SCALE

AS NOTED

.DGN LOCATOR

15402SitePlan

SUBDIRECTORY

d0174055

SITE PLAN AND PROFILE

DESIGNED

QUANTITIES

ISSUE DATE

REV. DATE

DRAWN

PROJECT GENERAL NOTES

- 1. ALL WORK SHALL BE IN CONFORMANCE WITH CURRENT NHDOT STANDARD SPECIFICATIONS AND DETAILS OR AS OTHERWISE MODIFIED WITHIN THE CONTRACT DOCUMENTS.
- 2. ENGINEER SHALL BE DEFINED AS THE RESIDENT ENGINEER / OWNER'S REPRESENTATIVE, WHO IS RESPONSIBLE FOR ENGINEERING SUPERVISION OF THE CONSTRUCTION, ACTING DIRECTLY OR THROUGH HIS/HER DULY AUTHORIZED REPRESENTATIVES.
- 3. THE CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS, PUBLIC OR PRIVATE, SHOWN OR NOT SHOWN, ON THESE PLANS PRIOR TO CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IN WRITING OF ANY UTILITIES FOUND INTERFERING WITH THE PROPOSED CONSTRUCTION AND APPROPRIATE REMEDIAL ACTION SHALL BE TAKEN BEFORE PROCEEDING WITH THE WORK, THE CONTRACTOR SHALL NOTIFY DIG-SAFE PRIOR TO CONSTRUCTION (1-888-DIG-SAFE).
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HIRING A LICENSED SURVEYOR IN THE STATE OF NEW HAMPSHIRE TO ESTABLISH HORIZONTAL AND VERTICAL CONTROL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE HORIZONTAL AND VERTICAL CONTROL THROUGHOUT THE PROJECT.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE TO NOTIFY RESIDENTS OF ANY WORK RESTRICTING ACCESS TO ANY DRIVEWAY 24 HOURS IN ADVANCE.
- 6. FOR STANDARD PLANS, SEE CURRENT NHDOT "STANDARD PLANS FOR ROAD AND BRIDGE CONSTRUCTION".
- 7. OVERHEAD UTILITY LINES ARE LOCATED THROUGHOUT THE PROJECT WITH CROSSINGS AT VARIOUS LOCATIONS AND RUNNING ALONG THE ROAD. THE CONTRACTOR IS ADVISED THAT EXTREME CAUTION WILL BE REQUIRED IN THE OPERATION OF EQUIPMENT, ESPECIALLY CRANES.
- 8. REMOVE TOPSOIL FOR ITS TOTAL DEPTH WITHIN THE LIMITS OF THE SLOPE LINES. UNLESS OTHERWISE DIRECTED. STOCKPILE TOPSOIL AND USE IT ON THIS PROJECT AS NEEDED UNDER SECTION 641 LOAM AND/OR SECTION 646.31 TURF ESTABLISHMENT WITH MULCH AND TACKIFIERS. STOCKPILE LOCATION SHALL BE DETERMINED BY THE CONTRACTOR WITHIN THE RIGHT-OF-WAY OR OFF SITE. ALL COSTS FOR STOCKPILING OF TOPSOIL SHALL BE INCLUDED IN THE APPROPRIATE 641 OR 646 ITEMS OF THIS CONTRACT.
- 9. ALL WORK SHALL BE PERFORMED WITHIN THE EXISTING RIGHT-OF-WAY OR ACQUIRED TEMPORARY RIGHT OF ENTRY LIMITS.
- 10. SURVEY DATA FOR THIS PROJECT WAS COLLECTED BY PROMISED LAND SURVEY LLC. 25 NASHUA ROAD. SUITE B1. LONDONDERRY. NH 03053 IN JUNE 2011. COORDINATES ARE NEW HAMPSHIRE STATE PLANE COORDINATES OF N.A.D. 1983/1986 AND THE BEARINGS ARE GRID. ELEVATIONS ARE REFERENCED TO NGVD 29.
- 11. THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO ENSURE THAT DEBRIS DOES NOT FALL ON ANY ROADWAY, RAILROAD, OR WATERWAY BELOW THE EXISTING STRUCTURE. ALL COSTS INCLUDING ERECTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURES OR OTHER SUCH APPROVED METHODS, SHALL BE SUBSIDIARY TO THE APPROPRIATE ITEMS OF WORK BEING PERFORMED.
- 12. EXISTING BEDROCK WITHIN THE LIMITS OF THE PROPOSED KEYED STONE FILL AND UNCLASSIFIED CHANNEL EXCAVATION SHALL NOT BE REMOVED.

BRIDGE GENERAL NOTES

- 1. DESIGN LOADING: HL-93.
- 2. DESIGN METHOD: LOAD AND RESISTANCE FACTOR DESIGN METHOD (LRFD).
- 3. SPECIFICATIONS: AASHTO LRFD 2014. WITH 2015 INTERIMS NHDOT 2010 STANDARD SPECIFICATIONS.
- 4. FOUNDATION DATA: ABUTMENTS AND WINGWALLS SPREAD FOOTING ON PREPARED BEDROCK. NOMINAL BEARING RESISTANCE = 30.0 TSF IN
- 5. REINFORCING STEEL: AASHTO M 31 (ASTM A 615) GRADE 60.
 - BACKWALL, APPROACH SLABS, DECK, SIDEWALK, CURB AND WING WALL CAP, REINFORCING STEEL SHALL BE EPOXY

COMBINATION WITH A RESISTANCE FACTOR OF 0.45

- COATED, AS NOTED ON THE PLANS.
- 6. STRUCTURAL STEEL: AASHTO M270, GRADE 50W (ASTM A709, GRADE 50W),
- 7. CONCRETE: BRIDGE DECK, SIDEWALK, CURB, WING WALL COPINGS AND

UNPAINTED EXCEPT AS NOTED.

- APPROACH SLABS = 4,000 PSI (AT 28 DAYS)
 FOOTINGS, ABUTMENTS AND WING WALL STEMS = 3,000 PSI
- (AT 28 DAYS)
- 8. SEISMIC PEAK GROUND ACCELERATION (PGA) = 0.10
 - SITE CLASS C ZONE = 1.0
- 9. ALL EXISTING BRONZE DISCS REPRESENTING STATE BENCHMARKS OR SURVEY TRIANGULATION POINTS MUST NOT BE DISTURBED. WHEN THE WORK CALLED FOR INVOLVES DISTURBING A BRONZE DISC. THE CONTRACTOR SHALL NOTIFY THE ENGINEER SUFFICIENTLY IN ADVANCE OF THE WORK TO PERMIT THE STATE TO TEMPORARILY RELOCATE THE AFFECTED MARKER.
- 10. MAINTENANCE OF TRAFFIC: TEMPORARY DETOUR REFER TO HIGHWAY GENERAL NOTES, SHEET 3 OF 35.
- 11. FOR SURVEY LAYOUT SEE BRIDGE SHEET 5 OF 35.

BRIDGE REMOVAL NOTES

- 1. THE CONTRACTOR SHALL SUBMIT, FOR DOCUMENTATION IN ACCORDANCE WITH SECTION 105.02, A DETAILED OUTLINE OR PLAN OF THE PROPOSED METHOD FOR REMOVAL OF THE EXISTING BRIDGE PRIOR TO COMMENCEMENT OF ANY REMOVAL WORK. BRIDGE REMOVAL SUBMITTALS SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW HAMPSHIRE.
- 2. ITEM 502, REMOVAL OF EXISTING BRIDGE STRUCTURE, SHALL INCLUDE, BUT NOT LIMITED TO, REMOVAL OF THE EXISTING BRIDGE SUPERSTRUCTURE, STEEL PIER CAP, CONCRETE ABUTMENT AND SEATS, CONCRETE WINGWALL CAPS, STONE ABUTMENTS AND WINGWALLS. REMOVE THE CONCRETE PIER AND CONCRETE PIER FOOTINGS TO THE LEVEL OF EXISTING BEDROCK OR 3 FT BELOW EXISTING STREAM BED.
- 3. REMOVE THE EXISTING ABUTMENTS TO LEVEL OF PROPOSED STREAMBED OR LIMITS OF KEYED STONE FILL. REMOVE THE EXISTING WINGWALLS TO THE LIMITS OF THE UNCLASSIFIED CHANNEL EXCAVATION OR KEYED STONE FILL AND A VERTICAL LINE 1'-6" OUSIDE THE LIMITS OF THE PROPOSED FOOTINGS.
- 4. REMOVAL OR EXCAVATION OF FILL MATERIAL WITHIN THE PROPOSED CHANNEL WHICH IS NOT WITHIN THE LIMITS OF 502, 504,101 OR 504,2 SHALL BE REMOVED UNDER ITEM 207.3, UNCLASSIFIED CHANNEL EXCAVATION.
- 5. PLANS OF THE EXISTING BRIDGE STRUCTURE ARE AVAILABLE AT THE CITY OF DOVER.
- 6. ROCK BRIDGE EXCAVATION MAY USE EITHER DRILLING AND BLASTING METHODS OR MECHANICAL METHODS AND WILL BE PAID FOR UNDER ITEM 504.2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE OR REPAIRS TO THE COFFERDAM THAT RESULT FROM BLASTING.
- 7. TEMPORARY FILLS SHALL REMAIN WITHIN WETLAND IMPACT AREAS SHOWN IN THE WETLAND PERMIT AND WITHIN EASEMENTS SHOWN ON THE SITE PLANS. A GEOTEXTILE FABRIC SHALL BE PLACED UNDER ALL TEMPORARY FILLS TO MINIMIZE DISRUPTION OF NATIVE SOILS AND VEGETATION. ALL COSTS SUBSIDIARY TO ITEM 502.

BORING NOTES

- 1. BORINGS INDICATED THUS WERE MADE BY NEW HAMPSHIRE TEST BORING, INC.
 UNDER CONTRACT TO WARD GEOTECHNICAL CONSULTING IN APRIL/MAY 2011. BLOW
 COUNTS SHOWN ARE THE NUMBER OF BLOWS REQUIRED TO DRIVE A 2" O.D.
 STANDARD SPLIT SPOON SAMPLER 6", USING A 140 LB WEIGHT FALLING 30".
- 2. BORINGS ARE FOR DESIGN PURPOSES SHOWING CONDITIONS AT BORING POINTS ONLY, AND DO NOT NECESSARILY INDICATE MATERIAL TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. ROCK CORES WERE MADE USING A 2" I.D. CORE BARREL.
- 4. GROUNDWATER LEVELS NOTED, IF ANY, WERE MEASURED AT THE TIME OF EXPLORATION. THE WATER LEVELS ENCOUNTERED DURING CONSTRUCTION MAY VARY CONSIDERABLY DUE TO PREVAILING CLIMATE, RAINFALL, OR OTHER FACTORS.

COFFERDAM NOTES

- 1. ITEM 503.201, COFFERDAMS, SHALL BE REQUIRED TO CONSTRUCT THE ABUTMENTS. THE CONTRACTOR SHALL SUBMIT THE COFFERDAM DESIGN AND PROPOSED METHOD OF CONSTRUCTION TO THE ENGINEER IN ACCORDANCE WITH SECTION 105.02 OF THE NHDOT STANDARD SPECIFICATIONS. COFFERDAM SUBMITTALS SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW HAMPSHIRE. ALL COSTS FOR DESIGN, INSTALLATION AND REMOVAL OF COFFERDAMS SHALL BE INCLUDED IN ITEM 503.201.
- 2. THE CONTRACTOR SHALL DETERMINE THE COFFERDAM LIMITS REQUIRED TO SUPPORT THE PROPOSED EXCAVATION WHILE MINIMIZING IMPACTS TO THE RIVER.

FOUNDATION NOTES

- 1. ABUTMENT FOUNDATIONS WILL CONSIST OF REINFORCED CONCRETE PLACED DIRECTLY ON BEDROCK. AT THE CONTRACTOR'S OPTION, SOUND BUT IRREGULAR AREAS DUE TO OVERBREAKAGE MAY BE FILLED TO THEORETICAL BOTTOM OF FOOTING WITH ITEM 520.211, CONCRETE CLASS B, FOOTINGS (ON ROCK).
- 2. ALL ABUTMENT FOUNDATION SUBGRADES SHALL BE PREPARED IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT. USE OF THE ALTERNATIVE GEOTECHNICAL RECOMMENDATIONS FOR FOUNDATION ON GLACIAL TILL IS PRECLUDED FOR THIS PROJECT.
- 3. FOR ADDITIONAL INFORMATION, SEE GEOTECHNICAL REPORT, INCLUDED IN THE CONTRACT DOCUMENTS.
- 4. DEWATERING SHALL BE CONTINUOUS UNTIL THE STRUCTURE IS BACKFILLED TO THE ELEVATION OF THE SURROUNDING WATER TABLE, UNLESS DIRECTED OTHERWISE.
- 5. ALL MEANS AND METHODS ASSOCIATED WITH HANDLING WATER DURING CONSTRUCTION OF FOUNDATIONS SHALL BE LOCATED WITHIN THE LIMITS OF WORK SHOWN ON THE WETLANDS PERMIT APPROVED FOR THE PROJECT.
- 6. TO CONSTRUCT THE PROPOSED ABUTMENT AND WINGWALLS, THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS OF EXCAVATION FOR THE PROPOSED UTILITY POLES ADJACENT TO EACH PROPOSED ABUTMENT. THE POLES SHALL BE SET PRIOR TO THE CONSTRUCTION OF THE ABUTMENTS AT THE PROPOSED GRADE AND SHALL BE MAINTAINED WITHOUT UNDERCUTTING.

ABUTMENT AND WINGWALL NOTES

- 1. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4".
- 2. ABUTMENT FOOTING AND WINGWALL FOOTING CONCRETE SHALL BE PAID AS ITEM 520.211, CONCRETE CLASS B, FOOTINGS (ON ROCK). ALL OTHER CONCRETE IN THE ABUTMENTS (EXCEPT AS STATED IN NOTE 4) AND WINGWALLS BELOW THE COPINGS SHALL BE ITEM 520.12, CONCRETE CLASS A, ABOVE FOOTINGS (F).
- 3. THE CONTRACTOR SHALL POUR ALL CONCRETE IN THE DRY.
- 4. THE CONCRETE IN THE BACKWALLS ABOVE THE BEARING SEAT CONSTRUCTION JOINT AND ALL COPINGS SHALL BE CONCRETE CLASS AA. BACKWALL AND COPING CONCRETE SHALL BE THE SAME MIX AS THE DECK AND PAID UNDER ITEM 520.02. CONCRETE CLASS AA. ABOVE FOOTINGS (F).
- 5. ITEM 538.2. BARRIER MEMBRANE. PEEL AND STICK VERTICAL SURFACES (F). SHALL BE PLACED OVER THE BEARING SEAT CONSTRUCTION JOINT. 1'-O" ABOVE AND BELOW THE JOINT. OVER THE JOINTS BETWEEN THE WINGWALL AND ABUTMENT WALLS. AND OVER VERTICAL CONTRACTION JOINTS. SEE DETAILS ON BRIDGE SHEETS 13 & 15 OF 35.
- 6. ITEM 534.3, WATER REPELLENT (SILANE-SILOXANE) SHALL BE APPLIED TO THE FACE OF THE BACKWALL (EXCLUDING THE EXPANSION JOINT STEEL) AND THE ENTIRE BRIDGE SEAT, INCLUDING THE BEARING PEDESTAL SURFACES.ITEM 534.3 SHALL BE APPLIED TO ALL EXPOSED SURFACES OF ABUTMENTS AND WINGWALLS TO 1'-O" BELOW THE FILL LINE.
- 7. BLOCKOUTS AND SLEEVES SHALL BE PROVIDED IN THE ABUTMENT BACKWALLS
 BETWEEN EACH GIRDER FOR THE INDIVIDUAL UTILITIES AS SHOWN. REINFORCING
 STEEL SHALL BE ADJUSTED AS REQUIRED. ADDITIONAL BARS FOR UTILITY
 BLOCKOUTS SHALL BE PROVIDED AROUND EACH BLOCKOUT ON EACH FACE. ALL COSTS
 SUBSIDIARY TO ITEM 544.31.
- 8. ANCHOR RODS SHALL BE SET BY TEMPLATE PRIOR TO PLACING ABUTMENT CONCRETE. FOR ANCHOR BOLT LOCATION PLAN, SEE BRIDGE SHEET 19 OF 35.
- 9. ALL REINFORCING IN THE ABUTMENTS ABOVE THE BACKWALL CONSTRUCTION JOINT AND IN THE WINGWALL COPINGS SHALL BE EPOXY COATED AND SHALL BE PAID AS ITEM 544.31, REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED). ALL OTHER REINFORCING STEEL IN THE ABUTMENTS AND WINGWALLS SHALL BE PAID AS ITEM 544.3, REINFORCING STEEL (CONTRACTOR DETAILED).
- 10. ALL REINFORCING SHALL BE A MINIMUM OF 21/2" FROM CONCRETE SURFACES.

 UNLESS NOTED OTHERWISE.
- 11. ITEM 562.1 SILICONE JOINT SEALANT SHALL BE USED TO SEAL ALL CONCRETE ABUTMENT AND WINGWALL CONTRACTION AND EXPANSION JOINTS. SEALANT SHALL ALSO BE APPLIED BETWEEN THE GRANITE CURB AND THE BRIDGE BACKWALL AND THE WINGWALL COPING.

ELASTOMERIC BEARING NOTES

- 1. ELASTOMERIC BEARING PADS SHALL BE VIRGIN NATURAL RUBBER WITH A SHEAR MODULUS OF 165 KSI (±15%) AND A MINIMUM LOW TEMPERATURE GRADE 4.
- 2. STEEL LAMINATES FOR ELASTOMERIC BEARING PADS SHALL CONFORM TO ASTM A1011 WITH A MINIMUM GRADE OF 36.
- 3. BEARING ASSEMBLIES, INCLUDING ELASTOMERIC BEARING PADS, SOLE PLATES, MASONRY PLATES, ANCHOR RODS, NUTS, AND WASHERS SHALL BE PAID AS ITEM 548.21, ELASTOMERIC BEARING ASSEMBLIES (F).
- 4. DESIGN LOADS: (SERVICE LOADS DESIGN METHOD B):

 MAXIMUM NON-COMPOSITE DEAD LOAD: 111.0K

 MAXIMUM SUPERIMPOSED DEAD LOAD: 35.5K

 MAXIMUM LIVE LOAD: 112.7K
- 5. ANCHOR RODS SHALL BE FABRICATED IN ACCORDANCE WITH SECTION 550.2.5.1.
 ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 55. ANCHOR RODS, NUTS, AND
 WASHERS SHALL BE GALVANIZED AFTER FABRICATION AND CONFORM TO AASHTO M
 232 (ASTM A153).
- 6. APPLY AN APPROVED SEALANT ALONG THE TRANSVERSE EDGES OF THE SOLE PLATE, UP AND AROUND TO THE ENDS OF THE FILLET WELDS. COST SHALL BE INCLUDED IN ITEM 548.21.
- 7. ALL STEEL PLATES SHALL CONFORM TO AASHTO M 270 GRADE 50W (ASTM A 709 GRADE 50W). MASONRY AND SOLE PLATES SHALL BE PAINTED AFTER VULCANIZING.
- 8. THE CONTINUOUS WELD CONNECTING THE BOTTOM OF THE GIRDER FLANGE TO THE TOP OF THE SOLE PLATE SHALL BE ALLOWED TO COOL AFTER EACH PASS. HOWEVER, THE TEMPERATURE OF THE STEEL ADJACENT TO THE ELASTOMER SHALL NOT EXCEED 200°F (TEMPERATURE SHALL BE CONTROLLED BY WELDING PROCEDURES AND TEMPERATURE INDICATING CRAYON OR OTHER DEVICES APPROVED BY THE ENGINEER). BEARING SURFACES IN CONTACT TO BE WELDED, SHALL BE FINISHED IN ACCORDANCE WITH AASHTO DIVISION II, SECTION 11.4.6. ALL PLATES SHALL BE FLAT AND TRUE AFTER WELDING.
- 9. SOLE AND MASONRY PLATS SHALL BE BLAST CLEANED (SSPC-SP 10) AFTER THE VULCANIZING PROCEDURE PRIOR TO PAINTING. SHOP PAINT BEARING ASSEMBLIES PER SPECIAL PROVISION 550. AFTER WELDING TO THE GIRDER FLANGE, CLEAN AND APPLY FINISH COATS TO THE SOLE PLATES.
- 10. IF STEEL GIRDERS ARE ERECTED WITH BEARINGS PLUMB AT THE AMBIENT TEMPERATURE HIGHER THAN 70°F OR LOWER THAN 20°F, AND THE BEARING SHEAR DEFLECTION EXCEEDS ONE-SIXTH OF THE BEARING HEIGHT AT 60°F±10°F, THE GIRDERS SHALL BE JACKED AND THE BEARINGS RESET TO PLUM (UNDEFORMED SHAPE) AT 60°F±10°F AS DIRECTED BY THE ENGINEER, ALL COSTS SUBSIDIARY TO ITEM 548.21.
- 11. THE FABRICATOR SHALL CLEARLY MARK THE BEARING ASSEMBLIES TO ENSURE PROPER ORIENTATION IN THE FIELD.
- 12. THE GIRDER BOTTOM FLANGE SHALL NOT BE FIELD WELDED TO THE TOP OF THE STEEL SOLE PLATE UNTIL AFTER THE CONCRETE DECK IS POURED.
- 13. FOLLOWING THE MANUFACTURE OF ELASTOMERIC BEARINGS AND VERIFICATION OF THE INTERNAL STEEL LAMINATES. THE PIN GROOVE OPENING SHALL BE COATED WITH AN APPROVED ASPHALTIC SEALER AND THE SPACE FILLED WITH SILICONE CAULKING.

CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES

LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402

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STRUCTURAL STEEL NOTES

- 1. STRUCTURAL STEEL SHALL CONFORM TO AASHTO M 270, GRADE 50W (ASTM A709, GRADE 50W) UNPAINTED (EXCEPT AS NOTED). ALL STRUCTURAL STEEL SHALL BE PAID UNDER ITEM 550.1, STRUCTURAL STEEL (F), INCLUDING THE GIRDERS, CROSS FRAMES, GUSSET PLATES, FILL PLATES, CONNECTION PLATES, SPLICE PLATES. STIFFENERS. AND FASTENERS.
- 2. THE NOTCH TOUGHNESS REQUIREMENTS OF THE NHDOT STANDARD SPECIFICATION 550.2.2 SHALL APPLY TO THE WEB. FLANGES AND SPLICE PLATES OF THE GIRDERS.
- 3. ALL WELDING AND THE PREPARATION AND ASSEMBLY OF MATERIAL FOR WELDING SHALL CONFORM TO THE NHDOT STANDARD SPECIFICATIONS. THE BRIDGE WELDING CODE (AASHTO/AWS D1.5) AND ALL INTERIM REVISIONS.
- 4. ALL STRUCTURAL STEEL, INCLUDING BRIDGE SHOES, ADJACENT TO THE DECK EXPANSION JOINTS SHALL BE PAINTED WITHIN 10' OF THE CENTERLINE OF BEARING. EXCEPT THE FASCIA SURFACES OF THE EXTERIOR GIRDER (THE BOTTOM OF THE TOP FLANGE, WEB, AND THE TOP AND EDGE OF THE BOTTOM FLANGE) SHALL NOT BE PAINTED.
- 5. THE LOCATION OF SHOP SPLICES SHALL BE APPROVED BY THE ENGINEER. WEB SPLICES SHALL BE LOCATED A MINIMUM OF 9" FROM WELDED FLANGE SPLICES AND A MINIMUM OF 6" FROM TRANSVERSE STIFFENERS OR CONNECTION PLATES.
- 6. ALL BOLTED FIELD CONNECTIONS SHALL BE MADE WITH $\frac{7}{8}$ " ϕ HIGH STRENGTH BOLTS AASHTO M164 (ASTM A325) TYPE 3 PLACED IN $^{15}/_{16}$ " ϕ HOLES. BOLTS IN PAINTED AREAS SHALL BE ASTM A325 TYPE 1 GALVANIZED.
- 7. DIRECT TENSION INDICATOR WASHERS SHALL BE INSTALLED WITH HIGH STRENGTH BOLTS.
- 8. HOLES FOR FIELD SPLICES SHALL BE SHOP DRILLED WHILE GIRDERS ARE ASSEMBLED TO FIT BEARING ELEVATIONS.
- 9. GIRDERS SHALL BE CAMBERED FOR THE FULL DEAD LOAD DEFLECTION. SEE BRIDGE SHEET 21 OF 35 FOR CAMBER TABLE. CAMBER TOLERANCE IS +15/16".-0".
- 10. SHOP OR FIELD WELDING OF ATTACHMENTS TO, OR PLACEMENT OF HOLES IN ANY EXPOSED PORTION OF THE PLATE GIRDERS FOR CONSTRUCTION PURPOSES, IS NOT PERMITTED. SHOP OR FIELD ATTACHMENTS TO THE TOP FLANGE FOR CONSTRUCTION PURPOSES MUST BE APPROVED BY THE ENGINEER.
- 11. CROSS FRAMES SHALL BE SHOP WELDED WITH 1/4" FILLET WELDS, UNLESS NOTED OTHERWISE. THE GRAVITY AXES OF CROSS FRAME MEMBERS SHOULD INTERSECT AS NEARLY AS PRACTICAL AT THE CENTERLINE OF THE GIRDER.
- 12. BEARING STIFFENERS AND GIRDER ENDS SHALL BE VERTICAL UNDER FULL DEAD LOAD DEFLECTION.
- 13. GIRDERS AND CROSS FRAMES SHALL BE FABRICATED SO THAT GIRDER WEBS ARE PLUMB UNDER FULL DEAD LOAD DEFLECTION.
- 14. THE BOLTED SPLICE CONNECTIONS SHALL BE PREPARED AS SLIP CRITICAL CLASS В.
- 15. SCREED RAIL SUPPORTS REQUIRED FOR THE PLACEMENT OF THE DECK CONCRETE SHALL BE LOCATED AT THE CENTERLINE OF THE GIRDER.
- 16. ALL SHEAR CONNECTORS SHALL BE FIELD WELDED TO THE TOP FLANGE WITH AUTOMATICALLY TIMED STUD WELDING EQUIPMENT. SHEAR CONNECTORS AT FIELD SPLICE LOCATIONS SHALL BE ARRANGED TO CLEAR FASTENERS AND SHALL BE WELDED TO THE SPLICE PLATES. THE TOTAL NUMBER OF SHEAR CONNECTORS IN A GIVEN LENGTH SHALL NOT BE REDUCED. SEE BRIDGE SHEET 20 OF 35 FOR SHEAR CONNECTOR DETAILS.
- 17. STEEL ERECTION SHALL NOT BE PERMITTED UNTIL THE ABUTMENTS HAVE BEEN BACKFILLED TO THE LEVEL OF THE APPROACH SLAB.
- 18. PRIOR TO HANDLING THE STRUCTURAL STEEL, THE CONTRACTOR SHALL SUBMIT DETAILED HANDLING AND ERECTION PLANS IN ACCORDANCE WITH SECTION 550.
- 19. TEMPORARY SHORING TOWERS SHALL NOT BE REMOVED UNTIL ALL STRUCTURAL STEEL IS ERECTED AND ALL SPLICES AND CROSS FRAME CONNECTIONS ARE FULLY TIGHTENED. ALL TEMPORARY SHORING TOWERS SHALL BE REMOVED PRIOR TO CONSTRUCTING THE DECK.
- 20. ALL STEEL ERECTION COSTS ARE INCLUDED IN ITEM 550.1.
- 21. THE ENGINEER WILL INSPECT THE SHOP FABRICATION OF THE STRUCTURAL STEEL.
- 22. ALL WELDS SHALL HAVE CORROSION RESISTANCE AND WEATHERING APPEARANCE AS SPECIFIED FOR WEATHERING STRUCTURAL STEEL.
- 23. THE STRUCTURAL STEEL FABRICATOR SHALL ARRANGE FOR NON-DESTRUCTIVE TESTING OF THE WELDS. ALL COSTS TO BE INCLUDED IN ITEM 550.1.
- 24. SHOP DRAWINGS SHALL INDICATE THE METHOD AND SEQUENCE TO BE FOLLOWED IN WELDING THE GIRDER COMPONENTS.
- 25. ALL PLAN DIMENSIONS ARE MEASURED HORIZONTAL AT 45°F WITHOUT ACCOUNT FOR PROFILE GRADE, UNLESS OTHERWISE NOTED.

DECK SLAB NOTES

- 1. AFTER THE STEEL GIRDERS ARE ERECTED, BUT BEFORE THE DECK FORMS ARE BUILT, ELEVATIONS ON THE TOP FLANGE OF THE GIRDERS ARE TO BE OBTAINED AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATIONS OBTAINED AND THOSE SHOWN IN THE TABLE IS THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF THE GIRDER TO THE BOTTOM OF THE DECK SLAB AT THE CENTERLINE OF THE GIRDER. SEE ELEVATION TABLE AND HAUNCH DETAIL ON BRIDGE SHEET 24 OF 35.
- 2. ELEVATIONS SHOWN IN THE TABLE ARE BOTTOM OF DECK SLAB ELEVATIONS ADJUSTED FOR TOTAL DEAD LOAD DEFLECTION, LESS THE DEFLECTION DUE TO GIRDER WEIGHT.
- 3. CONCRETE FOR THE BRIDGE DECK, SIDEWALK AND BRUSH CURB SHALL BE ITEM 520.70026, CONCRETE BRIDGE DECK (QA/QC)(PANEL OPTION)(F)
- 4. THE BRIDGE DECK CONCRETE SHALL BE PLACED IN ONE CONTINUOUS POUR AND REMAIN PLASTIC THROUGHOUT THE ENTIRE POUR.
- 5. DECK SLAB THICKNESS GIVEN IS FOR CAST-IN-PLACE DECK. FOR THICKNESS WITH DECK PANEL OPTION, SEE BRIDGE SHEET 35 OF 35.
- 6. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4".

DECK REINFORCEMENT NOTES

- 1. ALL REINFORCING IN THE BRIDGE DECK, SIDEWALK AND BRUSH CURB SHALL BE EPOXY COATED AND SHALL BE PAID AS ITEM 544.31. REINFORCING STEEL. EPOXY COATED (CONTRACTOR DETAILED).
- 2. ALL REINFORCING SHALL BE 21/2" FROM CONCRETE SURFACES, UNLESS OTHERWISE NOTED.

APPROACH SLAB NOTES

- 1. CONCRETE FOR THE APPROACH SLABS SHALL BE ITEM 520.0302. CONCRETE CLASS AA. APPROACH SLABS (QC/QA) (F).
- 2. ALL REINFORCING STEEL SHALL BE 21/2" CLEAR FROM CONCRETE SURFACES EXCEPT AS NOTED.
- 3. REINFORCEMENT IN THE APPROACH SLABS SHALL BE EPOXY COATED, AND PAID UNDER ITEM 544.31, REINFORCING STEEL EPOXY COATED (CONTRACTOR DETAILED).
- 4. FILL SPACE BETWEEN TIPPED DOWN APPROACH SLAB AND ROADWAY CURB OR WINGWALLS WITH ITEM 520.0302 CONCRETE CLASS AA, APPROACH SLABS (QC/QA) (F) (6" MIN DEPTH), EXTEND CONCRETE FROM ABUTMENT END OF APPROACH SLAB 6' ALONG SLAB, OR AS DIRECTED BY ENGINEER (QC/QA TESTING REQUIREMENTS WAIVED).
- 5. ITEM 544.7, SYNTHETIC FIBER REINFORCEMENT (F), SHALL BE ADDED TO THE CONCRETE USED FOR THE APPROACH SLABS.

REINFORCEMENT NOTES

- 1. REINFORCEMENT IN THE FOOTING, APPROACH SLABS, AND FACE OF CONCRETE CURB SHALL HAVE 3" CLEAR COVER. ALL OTHER REINFORCEMENT SHALL HAVE 21/2" CLEAR COVER, UNLESS OTHERWISE NOTED.
- 2. PLACE REINFORCING STEEL TO AVOID WEEPERS, RAIL POST ANCHOR ASSEMBLIES, PILES AND EXPANSION JOINT STEEL.
- 3. REINFORCING IN THE TOP OF ABUTMENTS SHALL BE ADJUSTED TO CLEAR ANCHOR
- 4. ANY EPOXY COATED REBARS CUT TO FIT SHALL BE TOUCHED UP WITH AN APPROVED EPOXY COATING MATERIAL. ALL COSTS SHALL BE INCLUDED IN ITEM 544.3 OR 544.31.
- 5. REINFORCING LEGEND:

SS = STAINLESS STEEL

ALT = ALTERNATE BOT = BOTTOM BRG = BEARING CLR = CLEARDOW = DOWEL EQ = EQUALFF = FAR FACE MAX = MAXIMUM MC = MECHANICAL CONNECTOR MIN = MINIMUM NF = NEAR FACE MID = MIDDLESECT= SECTION SP = SPACE SPL = SPLICE SYM = SYMMETRICAL TYP = TYPICAL E = EPOXY COATED

EXPANSION JOINT NOTES

- 1. ALL EXPANSION JOINT STEEL, INCLUDING ANCHORS, SHALL BE GALVANIZED. STEEL ANGLES SHALL BE AASHTO M223 (ASTM A572) GRADE 50. MINOR STEEL PLATES MAY CONFORM TO AASHTO M183 (ASTM A36). THE ENTIRE ASSEMBLY, INCLUDING STRIP SEAL, SHALL BE PAID FOR AS ITEM 561.1001, PREFABRICATED STRIP SEAL EXPANSION JOINT (F).
- 2. SPLICES FOR EXPANSION JOINT STEEL SHALL DEVELOP FULL STRENGTH.
- 3. EXPANSION JOINT OPENING SHALL BE ADJSTED TO TEMPERATURE ANTICIPATED JUST PRIOR TO POURING DECK BLOCKOUT, FINAL SETTING IN THE FIELD SHALL BE DETWERMINED BY THE ENGINEER. SEE TEMPERATURE ADJSTMENT TABLE AND NOTES.
- 4. STRIP SEAL SHALL BE FURNISHED IN ONE CONTINUOUS LENGTH. NO SPLICES WILL BE ALLOWED. SEAL SHALL BE INSTALLED IN THE FIELD BY THE CONTRACTOR, IN ACCORDANCE WITH THE MANUFACTURER OF THE SEAL, USING AN APPROVED TOOL THAT WILL NOT DAMAGE THE SEAL.
- 5. JOINT SUPPORT PLATES AND CURB PLATES SHALL BE SHOP WELDED TO EXPANSION JOINT STEEL AND SHALL BE NORMAL TO GRADE AFTER JOINT ASSEMBLY HAS BEEN ADJUSTED FOR ROADWAY CROSS-SLOPE AND GRADE. STEEL ANGLES AND EXTRUSIONS SHALL BE ASSEMBLED WITH A CONSTANT JOINT OPENING TO ENSURE PROPER PERFORMANCE AND WATER TIGHTNESS.
- 6. THE EXPANSION JOINT ASSEMBLY SHALL BE INSTALLED ONLY AFTER THE ABUTMENT HAS BEEN BACKFILLED TO WITHIN 3'-0" OF FINISHED GRADE.
- 7. IMMEDIATELY AFTER THE JOINT HAS BEEN SECURED TO THE STRUCTURAL STEEL AND BACKWALL, REMOVE SHIPPING DEVICES AND GRIND SMOOTH ANY WELDS ON EXPOSED SURFACES. REPAIR ANY DAMAGE TO GALVANIZED SURFACES IN ACCORDANCE WITH SECTION 550.
- 8. PROTECT TOP OF EXPANSION JOINT DURING PLACEMENT OF CONCRETE AND BITUMINOUS PAVEMENT.
- 9. THE STRIP SEAL HAS BEEN DESIGNED FOR A TOTAL FACTORED MOVEMENT OF 1.87 INCHES. DESIGN INCLUDES MOVEMENT DUE TO TEMPERATURE, SKEW, SHRINKAGE AND MINIMUM INSTALLATION WIDTH. THE CONTRACTOR SHALL USE AN SE-400 SEAL BY WATSON BOWMAN OR A2R-400 BY D.S. BROWN.
- 10. ELEVATIONS SHOWN AT TOP OF ANGLES ARE 1/8" LOWER THAN PROPOSED FINISHED ROADWAY GRADE.
- 11. ANGLES 6"x4"x³/₄" SHALL BE UTILIZED FOR SEALS LESS THAN 5" (HEIGHT).
- 12. STEEL ANGLES AND STOP BARS SHALL BE MAINTAINED FREE FROM DIRT, WATER AND ANY OTHER LOOSE DEBRIS, WITH THE USE OF COMPRESSED AIR, TO ENSURE PROPER FIT OF THE SEAL. CARE SHALL BE TAKEN NOT TO DAMAGE GALVANIZED SURFACES.
- 13. IF JOINT ASSEMBLY IS IN PLACE OVER A WINTER WITHOUT A SEAL, THE JOINT OPENING AND ABUTMENT SHALL BE WASHED PRIOR TO INSTALLATION OF THE SEAL. ALTERNATELY, A TEMPORARY SEAL CAN BE PLACED AND REMOVED FOR INSTALLATION OF THE FINAL SEAL, ALL COSTS SHALL BE SUBSIDIARY TO THE EXPANSION JOINT.

TEMPERATURE ADJUSTMENT NOTES

- 1. "T" DIMENSIONS ARE PERPENDICULAR TO FACE OF BACKWALL.
- 2. MINIMUM "T" WIDTH FOR SEAL INSTALLATION = 1.5" FOR THE SE-400 OR 1.75" FOR THE A2R-400 (APPROXIMATELY 65°F OR LESS).
- 3. VALUES IN THE TEMPERATURE ADJUSTMENT TABLE ARE FOR SETTING THE EXPANSION JOINT ASSEMBLY IMMEDIATELY PRIOR TO POURING THE DECK BLOCKOUT.

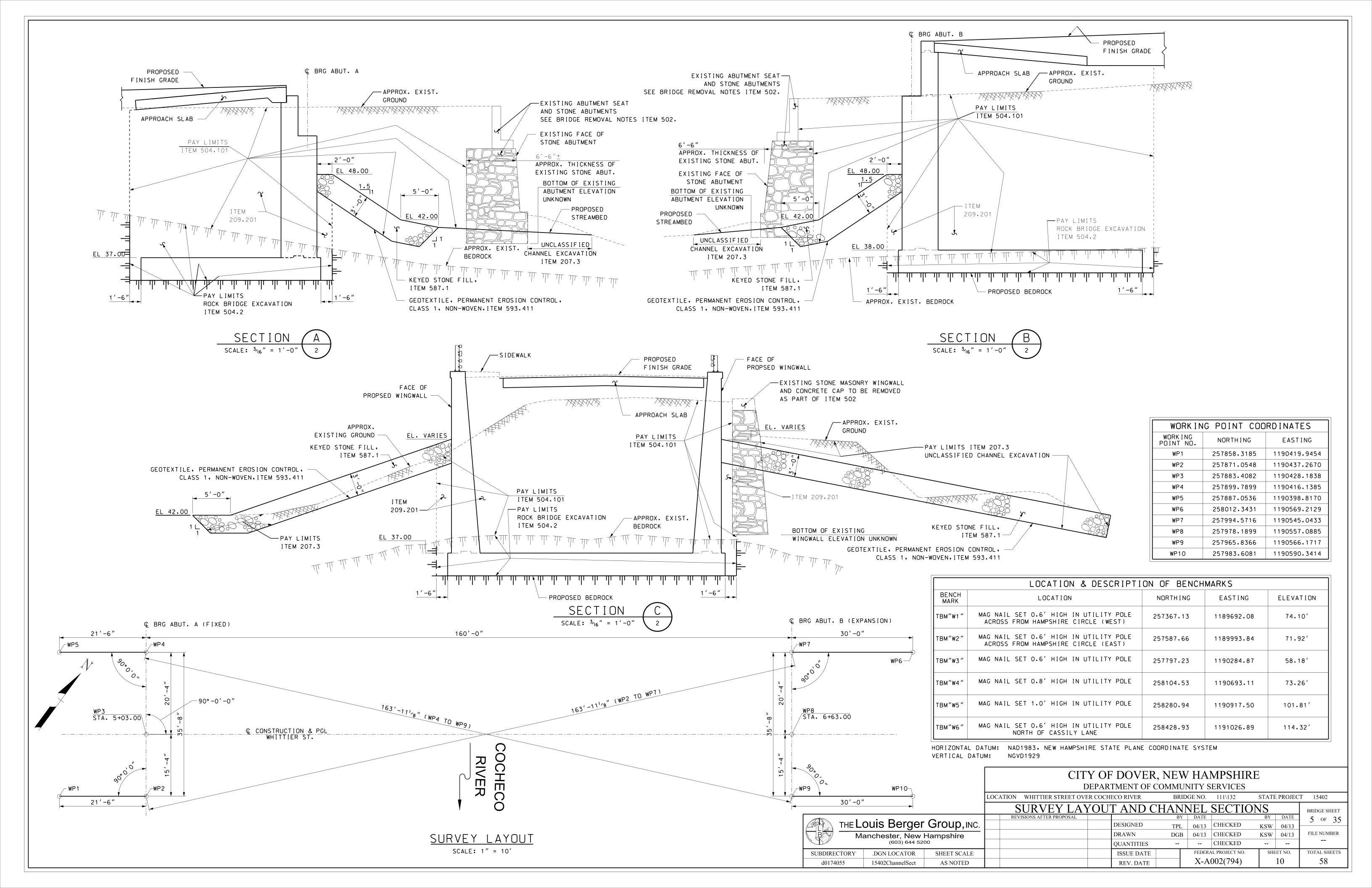
UTILITY NOTES

- 1. ITEM 611.43904, 12" CEMENT LINED DUCTILE IRON WATER PIPE (BRIDGE), SHALL INCLUDE ALL NECESSARY MATERIALS, INCLUDING PIPE, INSULATION, STRAPS, PROTECTIVE JACKETING, EXPANSION JOINTS, SADDLES, ROLLER SUPPORTS, ASSOCIATED HARDWARE AND LABOR TO INSTALL THE UTILITY. FOR LIMITS, SEE BRIDGE SHEET 23 OF 35.
- 2. ITEM 614.74218, 4 INCH 2-DUCT PVC CONDUIT, SCHEDULE 80, SHALL INCLUDE ALL NECESSARY MATERIALS, INCLUDING CONDUIT, HANGER SYSTEMS, COUPLINGS, EXPANSION COUPLINGS, PULL WIRES, AND LABOR TO INSTALL THE COMMUNICATIONS CONDUITS. LIMITS OF ITEM 614.74218 SHALL BE BETWEEN THE FRONT FACES OF ABUTMENT BACKWALLS.
- 3. CONDUIT EXPANSION COUPLINGS SHALL PROVIDE MINIMUM MOVEMENT RANGE OF 4".
- 4. UTILITY SUPPORTS FOR COMMUNICATIONS CONDUIT SHALL BE FIBERGLASS 1H X 2W BASE SUPPORTS BY METRA INDUSTRIAL CORPORATION OF COLUMBUS, OHIO OR APPROVED EQUAL. SUPPORTS SHALL BE CAPABLE OF SUPPORTING A TOTAL CONDUIT REACTION OF 0.5 KIPS.
- 5. ALL THREADED RODS, NUTS, WASHERS, STRAPS, AND PIPE ROLL SUPPORTS SHALL BE GALVANIZED.
- 6. ALL HOLES IN STRUCTURAL STEEL SHALL BE SHOP DRILLED. FIELD DRILLING WILL NOT BE PERMITTED. HOLE DIMENSIONS AND SPACING SHALL BE VERIFIED BY CONTRACTOR FOR ACTUAL UTILITY SUPPORTS UTILIZED.
- 7. ALL WORK RELATED TO THE INSTALLATION OF THE TEMPORARY PIPE BRIDGE. REMOVAL OF THE EXISTING GAS LINE ON THE BRIDGE AND APPROACHES, INSTALLATION OF THE PROPOSED GAS LINE UTILITY SUPPORTS AND PIPE ROLLER SUPPORTS, AND REMOVAL OF TEMPORARY GAS MAIN AND TEMPORARY PIPE BRIDGE SHALL BE SUBSIDIARY TO ITEM 612.99.

CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES

LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402

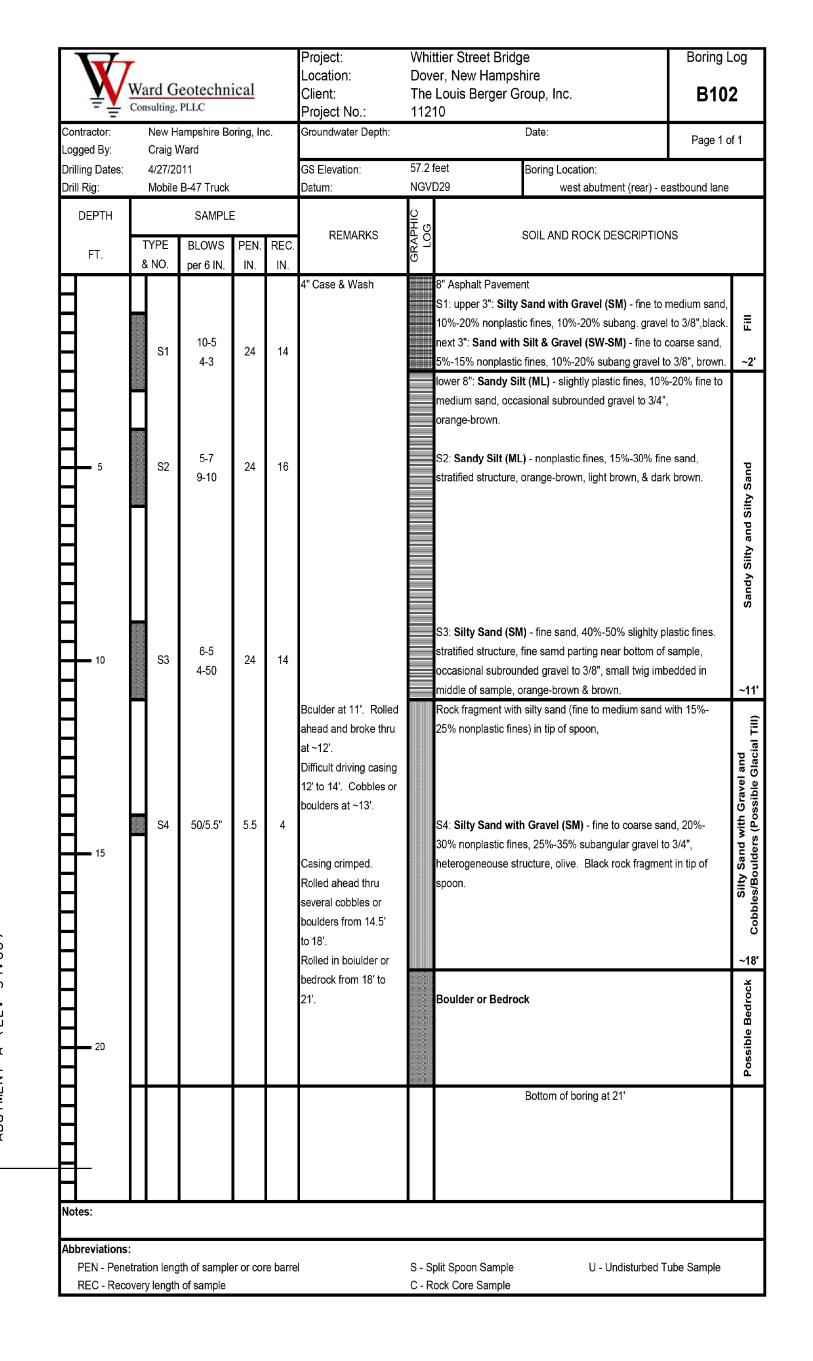
			BRIDG	E NO	OTES (SH	EET 2	OF	2)			BRIDGE SHEET
			REVISIONS AFTER PROPOSAL			BY	DATE		BY	DATE	4 of 35
THE LO	ouis Berger	Group, INC.			DESIGNED	TWP	11/15	CHECKED	KSW	11/15	1 3 33
Ma Ma	anchester, New	Hampshire			DRAWN	DWM	11/15	CHECKED	KSW	11/15	FILE NUMBER
	(603) 644 52	00			QUANTITIES	TWP	11/15	CHECKED	HNH	11/15	
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE			ISSUE DATE	_ [FEDER	AL PROJECT NO.	SHE	ET NO.	TOTAL SHEETS
d0174055	15402BrNotes02	AS NOTED			REV. DATE		X-A	A 002(794)		9	58

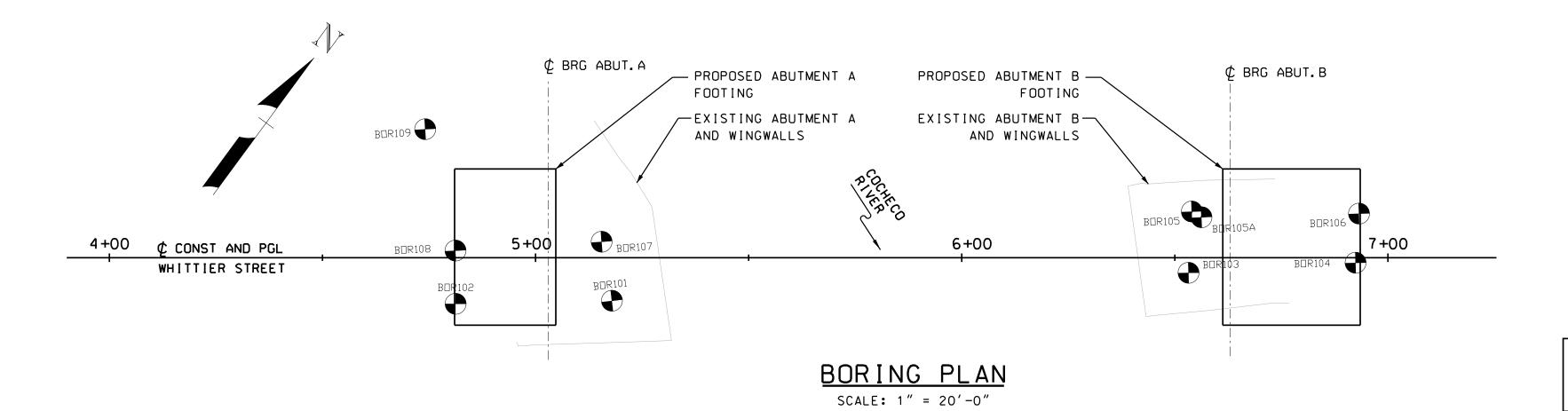


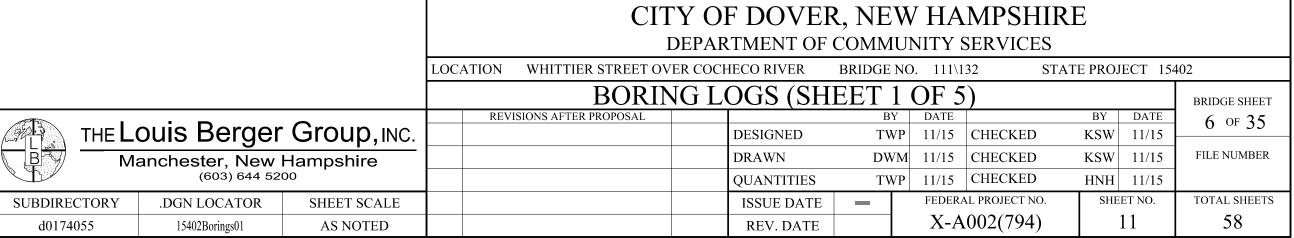
STA. 4+81.2. 10.8' RT.

<u> </u>		Geotechn	ical		Project: Location: Client: Project No.:	Whittier Street Bridge Dover, New Hampshire The Louis Berger Group, Inc. 11210 Boring Boring B10				
Contractor: Logged By:		Hampshire B g Ward	oring, Ir	IC.	Groundwater Depth:		Date:	Page 1 of 2		
Drilling Dates: Drill Rig:		- /2011 ile B-47 Truck			GS Elevation: Datum:	57.4 feet NGVD29	Boring Location: west abutment - ea	ethound lane		
DEPTH	IVIOL	SAMPL			Datam.		west abutifiert - ear	Stodend lane		
	TYPE		PEN.	REC.	REMARKS	GRAPHIC LOG	SOIL AND ROCK DESCRIPTION	ONS		
FT.	& NO	per 6 IN.	IN.	IN.	4" Case & Wash		A 1 11 D			
5	S ²	10-8	24	12	4 Case a wasii	S1: coar lowe nont	Asphalt Pavement upper 10": Sand with Gravel (SW) - fine to mease) sand, 20%-30% subangular gravel to 1", b r 2": Silty Sand (SM) - fine to medium sand, 1 elastic fines, brown. Silty Sand (SM) - fine to medium sand, 40%-5 ic fines, occasional subangular gravel to 3/8",	own. 5%-25% 0% slightly		
10	S	6-3 2-10	24	16	Little casing driving resistance 4' to 9'. Rolled through gravel or cobble at 8.8'.	S3:	Sandy Silt (ML) - slightly plastic fines, 10%-20 um sand, occasional subangular gravel to 1/4 ighout, moist, redish brown. Rock fragment in	", fine roots		
15	S4	30-50 11-22	24	9	Little casing driving resistance 9' to 14'. Split-spoon bent. Casing refusal at 16.4'. Rolled ahead and broke	30% rock	Silty Sand with Gravel (SM) - fine to coarse s nonplastic fines, 30%-40% angular gravel to a fragments), olive-brown.	and, 20%-		
20	St	25-27 35-16	24	10	through boulder at 17'. Drove casing to 19'. End of casing crimped. Rolled ahead to 24', then drove casing to 24'.	S5::	Silty Sand with Gravel (SM) - fine to medium nonplastic fines, 30%-40% subangular gravel crock fragments), olive & rust.	sand, 20%-		
24.0	S6	50/0"	0	0		98.	Spoon Refusal - no penetration/no recovery			
Notes:	30	30/0	U	ı ^U	<u> </u>	30:	ppoori merupai - no penetration/no recovery			

	Vard G	eotechn	ical		Project: Location: Client: Project No.:	Dov			Boring I	
Contractor: _ogged By:	New H Craig \	ampshire Bo Nard	oring, In	IC.	Groundwater Depth:			Date:	Page 2 o	of 2
Orilling Dates: Orill Rig:	4/27/20				GS Elevation: Datum:	57.4 t		Boring Location: west abutme	nt - eastbound lane	
DEPTH		SAMPLE				HIC G				
FT.	TYPE & NO.	BLOWS per 6 IN.	PEN. IN.	REC. IN.	REMARKS	GRAPHIC LOG		SOIL AND ROCK DESC	RIPTIONS	
25	C1		60	60	Rolled to 25' to core. Slowly lost water while coring. Coring rates varied from 6.5 to 11.5 min/foot.	1	to hard, fresh to several fine qua 3.1', most joints	fine grained gray meta-sedin o slightly weathered, steep fo artz veins and quartz intrusion s near horizontal & dipping 10 or to 70° at 28.6', joint spacing	liation (60° - 90°), n from 27.8' to 0° to 30°, one	Ē
40										







¥ <u>¥</u>		Geotechr	<u>iical</u>		Project: Location: Client: Project No.:	Dove	tier Street Bridge er, New Hampshire Louis Berger Group, Inc. IO	Boring B1 (
Contractor: Logged By: Drilling Dates: Drill Rig:	Crai 4/28	/ Hampshire B g Ward 5/2011 ille B-47 Truck	-	nc.	Groundwater Depth: GS Elevation: Datum:	~58.4 NGVI	Date: feet Boring Location:	Page 1
DEPTH		SAMPL			DEMARKS	PHIC G		
FT.	TYPE & NO		PEN. IN.	REC. IN.	REMARKS	GRAPHIC LOG	SOIL AND ROCK DESCRIPTION	S
5	S ²	3-2 3-4	24	13	4" Case & Wash Pushed cobble ahead with spoon. Casing drove easily 4' to 9'. Rolled through gravel or cobbles at ~8.7' to 9'.		10" Asphalt Pavement S1: upper 2": Asphalt and Gravel next 3": Sand with Gravel (SW) - fine to medium (solisand, 15%-25% subangular gravel to 1/2", light brown lower 8": Silty Sand with Gravel (SM) - fine to mediuc coarse) sand, 15%-25% nonplastic fines, 10%-20% sigravel to 3/4", brown. S2: Silty Sand (SM) - fine to medium sand, 15%-25% fines, 10%-20% subangular gravel to 3/4", brown. S3: Silty Sand with Gravel (SM) - fine to medium sand, 15%-25% fines, 10%-20% subangular gravel to 3/4", occurrence fines, 20%-30% angular gravel to 3/4", occ	n. im (some ubangular 6 nonplastic
15	S ²	3-2 2-3	24	0	from 9' to 14'. Rolled through gravel or cobbles. Pushed cobble ahead with spoon. Casing drove easily to ~17'. Rolled through cobbles		S4: No Recovery Redrove with 3" split-spoon: 12" recovery: Sandy Silt (ML) - slightly plastic fines, 10%-30% fine sand, occasional gravel to 3/4", 2 small pockets of orgolive-brown.	
20	SS	5 25-26 50/5"	17	10	or boulders below 18'. Casing refusal at 19.6'. Rolled ahead. Rolled on dense till or weathered bedrock at 21'. Rolled to 23' to		S5: Silty Sand with Gravel (SM) - fine to medium (so sand, 15%-25% nonplastic fines, 25%-35% subangulato 1" (some weathered), heterogeneous structure, oliv	ar gravel

Contractor:	Consulting,	ampshire Bo		c.	Project: Location: Client: Project No.: Groundwater Depth:	Dov The 112		hire	Boring L B103	3
Drilling Dates: Drill Rig:	4/28/20 Mobile)11 B-47 Truck			GS Elevation: Datum:	~58.4 NGV		Boring Location: east abutment - eastb	ound lane	
DEPTH FT.	TYPE & NO.	SAMPLE BLOWS per 6 IN.	PEN. IN.	REC.	REMARKS	GRAPHIC LOG		SOIL AND ROCK DESCRIPTION		
25	C2 C3	per o in.	60	18	Coring rate varied from 2 to 13 min/foot. Only 18" recovery - attempt to retreive failed. Coring rates of 1.5 & 5 min/foot. Core barrel jamed at 29.3'. Rolled to 30' to core C3. Coring rate varied from 4 to 6 min/foot.		to hard, moderately fine quartz veins, jo foliations), joint spa some fracture faces RQD ~ 0% C2: Bedrock - fine to hard, highly weat fine quartz veins, jo spacings range fro C3: Bedrock - fine to hard, fresh to slig 3" thick quartz intru dip 70° to 80° with s	grained gray meta-sedimentary rothered, steep foliation (~80°), seven oints near vertical and near horizon m 0.5" to ~3". RQD ~ 0. grained gray meta-sedimentary roghtly weathered, steep foliation (70 sion at 32.7', joints above quartz is spacings of 0.5" to 5.5", joints belowerizontal and near vertical at space	several along gouge on ock, soft eral htal, joint ock, soft	Bedrock
35								Bottom of boring at 35'.		
Abbreviations: PEN - Pene REC - Reco	tration leng		er or cor	e barre	1		plit Spoon Sample ock Core Sample	U - Undisturbed Ti	ube Sample	

V					Project: Location:	Whittier Stre Dover, New	•	Boring L	.og
<u> </u>	Ward G Consulting,	eotechn	ical		Client:	The Louis B	erger Group, Inc.	B104	4
Contractor:	0.733	ampshire Bo	oring In	r	Project No.: Groundwater Depth:	11210	Date:		
ogged By:	Craig \	-	omig, m	0.	Groundwater Beptin.		bate.	Page 1 of	ıf 1
rilling Dates:	4/28/20				GS Elevation:	59.7 feet	Boring Location:		
orill Rig:	Mobile	B-47 Truck			Datum:	NGVD29	east abutment - east	bound lane (rear)	
DEPTH	TYPE	SAMPLI BLOWS	PEN.	REC.	REMARKS	GRAPHIC LOG	SOIL AND ROCK DESCRIPT	TIONS	
FT.	& NO.	per 6 IN.	IN.	IN.		Q R			
5 10	S1 S2	12-10 8-8 4-3 5-5	24	15	4" Case & Wash Rolled through cobble at ~3.5'.	S1: upper 10%-20% lower 6": nonplastic chunk of a sand (occorasional lower 5": 20% nonp	r 9": Sand with Gravel (SW) - fine to consubangular gravel to 1/2", orange-brown Silty Sand (SM) - fine to medium sand, or fines, occasional subrounded gravel to asphalt, olive-brown. r 10": Sandy Silt (ML) - nonplastic finest easional medium to coarse sand), vague all fine roots, orange-brown. Silty Fine Sand (SM) - fine (some mediolastic fines, light brown-orange.	nn. 20%-30% 3/4", 3/8" , 10%-20% fine estratification, fium) sand, 10%-	~
— 15	\$4 \$5	10-31 43-28 41-33 36-44	24	6	Rolled through several cobbles from 16' to 19'.	sand, 20% 3/4", olive	Sand with Gravel (SM) - fine to mediur %-30% nonplastic fines, 30%-40% subarbrown. Rock fragment in tip of spoon. Sand with Gravel (SM) - similar to S4.		2
otes:					Rolling on boulder or bedrock from 21.5' to 24.1'.		Bottom of Boring at 24.1'		30 30 70 70 70 70 70 70 70 70 70 70 70 70 70

				F DOVER			MPSHIRI SERVICES	E	
	LOCAT	TION WHITTIER STREET OV	ER COCH	HECO RIVER	BRIDGE	NO. 111\1	32 STAT	E PROJECT 154	102
		BORIN	IG L	OGS (SH)	EET 2	2 OF 5	(i)		BRIDGE SHEET
Tuel Quie Perger Croup No		REVISIONS AFTER PROPOSAL		DEGLES VED	B		GUID GUID D	BY DATE	7 of 35
THE Louis Berger Group, INC.				DESIGNED	TW		CHECKED	KSW 11/15	
Manchester, New Hampshire				DRAWN	DW	M 11/15	CHECKED	KSW 11/15	FILE NUMBER
(603) 644 5200				QUANTITIES	TW	/P 11/15	CHECKED	HNH 11/15	
SUBDIRECTORY .DGN LOCATOR SHEET SCALE				ISSUE DATE	=	FEDERA	AL PROJECT NO.	SHEET NO.	TOTAL SHEETS
d0174055 15402Borings02 AS NOTED				REV. DATE		X-A	.002(794)	12	58

BORING NO. B105A STA. 6+56.2, 9.5' LT.

BORING NO. B105A

V /					Project: Location:		tier Street Bridge er, New Hampshire	Boring L	2
<u> </u>		eotechn	ical		Client:	The I	Louis Berger Group, Inc.	B105	5
:=:	Consulting,				Project No.:	1121			
ontractor: ogged By:	New H Craig \	lampshire Bo Ward	oring, In	IC.	Groundwater Depth:		Date:	Page 1 of	f 1
rilling Dates:	4/29/2				GS Elevation:	58.3 fe	eet Boring Location:	<u> </u>	
rill Rig:	Mobile	B-47 Truck	_	_	Datum:	NGVE		tbound lane	
DEPTH		SAMPLI	E			⊇ - ⊒			
	TYPE	BLOWS	PEN.	REC.	REMARKS	GRAPHIC LOG	SOIL AND ROCK DESCRIPTION	NS	
FT.	& NO.	per 6 IN.	IN.	IN.		GF			
5 10 20	\$1 \$2 \$3	16-7 5-3 2-3 6-12 24-18 19-10	24 24 24 25	10	Split-spoon kicked away from river. Spoon bent. Casing refusal at 10'. Rolled thru boulder at 11.5'. Casing refusal at 12.8'. Rolled ahead (lost water) to 13.1'. Seated casing at 13'. Rolled to 14' to core. Core barrel dropped suddenly from 15' to 15.4'. Core barrel dropped suddenly again from 16.5' to 17'. Pulled core barrel to check. Took S4 at 17'. Tried to roll ahead to 19', but hole collapsed. Coud not advance casing through boulders or masonry (refusal at 13'). Abandoned boring and moved		11" Asphalt Pavement S1: upper 7": Sand with Silt & Gravel (SW-SM) - fisand, 5%-15% nonplastic fines, 25%-35% subround, 3/4", brown & black. Iower 6": Sandy Silt with Gravel (ML) - nonplastic 20% fine sand, light brown-orange. S2: Sandy Silt with Gravel (ML) - nonplastic fines fine to medium sand, 10%-20% subrounded gravel light brown-olive. S3: Silty Sand with Gravel (SM) - fine to medium 30% nonplastic fines, 30%-40% subangular gravel olive-brown. C1: Boulders or Stone Masonry - gray meta-sedirock similar to local bedrock but with horizontal folliprobably stone masonry near bottom of wing wall owall. S4: Silty Sand with Gravel (SM) - fine (some med 20%-30% nonplastic fines, 20%-30% subangular golive-brown & gray. Possibly fill or till that was distused by Bottom of Boring at 19'	ded gravel to fines, 10%- 10%-30% to 3/4", sand, 20%- to 1', mentary ation - r abutment ium) sand, ravel to 3/4",	

<u></u>	Ward G	eotechn	ical		Project: Location: Client: Project No.:	Dov	ttier Street Bridge ver, New Hampshire Louis Berger Group, Inc. 10	Boring B10
Contractor: Logged By: Drilling Dates:	Craig \ 4/29/2	011		IC.	Groundwater Depth: GS Elevation:	58.5		Page 1
Drill Rig: DEPTH	Mobile	B-47 Truck SAMPL BLOWS		REC.	Datum: REMARKS	GRAPHIC SA LOG	<u></u>	
FT.	& NO.	per 6 IN.	IN.	IN.	4" Case & Wash	Ö		
10					Little resistance to driving casing from 4' to 9'. Little resistance to driving casing from 9' to 14'.		Moved to B5A from B5, which was abandoned due difficulties in advancing casing through boulders or blocks. B5A drilled to determine conditions below B5 was abandoned. No samples obtained from B5 Refer to log for B5 for descriptions of subsurface coabove 19'.	masonry 19', where 5A above 19'.
20	S1	29-31 56-73	24	13	Increased casing resistance at ~17'. Rolled ahead to 22'. Drove casing to refusal at 22.3'. Rolled in bedrock from		S1: Silty Sand with Gravel (SM) - fine to medium sand, 15%-25% nonplastic fines, 25%-35% suband to 1", olive-brown.	

				Project: Whittier Street Bridge Boring Location: Dover, New Hampshire						
<u> </u>	Ward G Consulting,	eotechni	ical		Client:	The	Louis Berger Group, I	nc.	B105	Α
contractor: ogged By:	0.533	ampshire Bo	oring, In	C.	Project No.: Groundwater Depth:	112	Date:		Page 2 o	of 2
Orilling Dates: Orill Rig:	4/29/20				GS Elevation: Datum:	58.5 f	3	Location: east abutment - wes	though long	
DEPTH	Mobile	SAMPLE			Datum.		528	east abutment - wes	ibourid lane	
FT.	TYPE & NO.	BLOWS	PEN.		REMARKS	GRAPHIC LOG	SOIL A	ND ROCK DESCRIPTIO	NS	
25	C1	per 6 IN.	IN. 60	1N. 59	Coring rate varied from 4 to 5 min/foot.		C1: Bedrock - fine grained hard, fresh to slightly weath joints dipping 0° to 20° and ranging from 1.5" to 9". RQD = 45"/60" = 75%	ered, steep foliation (60°	' to 80°),	
30							Botto o	f Boring at 28'		
35										
40										
45 										

		CIT	Y O	F DOVER	R, NEV	W HA	MPSHIR	Е		
		I	DEPAR	TMENT OF	COMMU	JNITY S	SERVICES			
	LOCA	ATION WHITTIER STREET OV	ER COCH	IECO RIVER	BRIDGE N	NO. 111\1:	32 STAT	E PROJ	ECT 154	02
		BORING LOGS (SHEET 3 OF 5)								BRIDGE SHEET
		REVISIONS AFTER PROPOSAL		,	BY	DATE		BY	DATE	8 of 35
THE Louis Berger Group, INC.				DESIGNED	TW	P 11/15	CHECKED	KSW	11/15	
Manchester, New Hampshire				DRAWN	DWI	M 11/15	CHECKED	KSW	11/15	FILE NUMBER
(603) 644 5200				QUANTITIES	TW	P 11/15	CHECKED	HNH	11/15	
SUBDIRECTORY .DGN LOCATOR SHEET SCALE				ISSUE DATE	_	FEDERA	L PROJECT NO.	SHE	ET NO.	TOTAL SHEETS
d0174055 15402Borings03 AS NOTED				REV. DATE		X-A	002(794)	-	13	58

STA. 5+15.5. 3.8' LT.

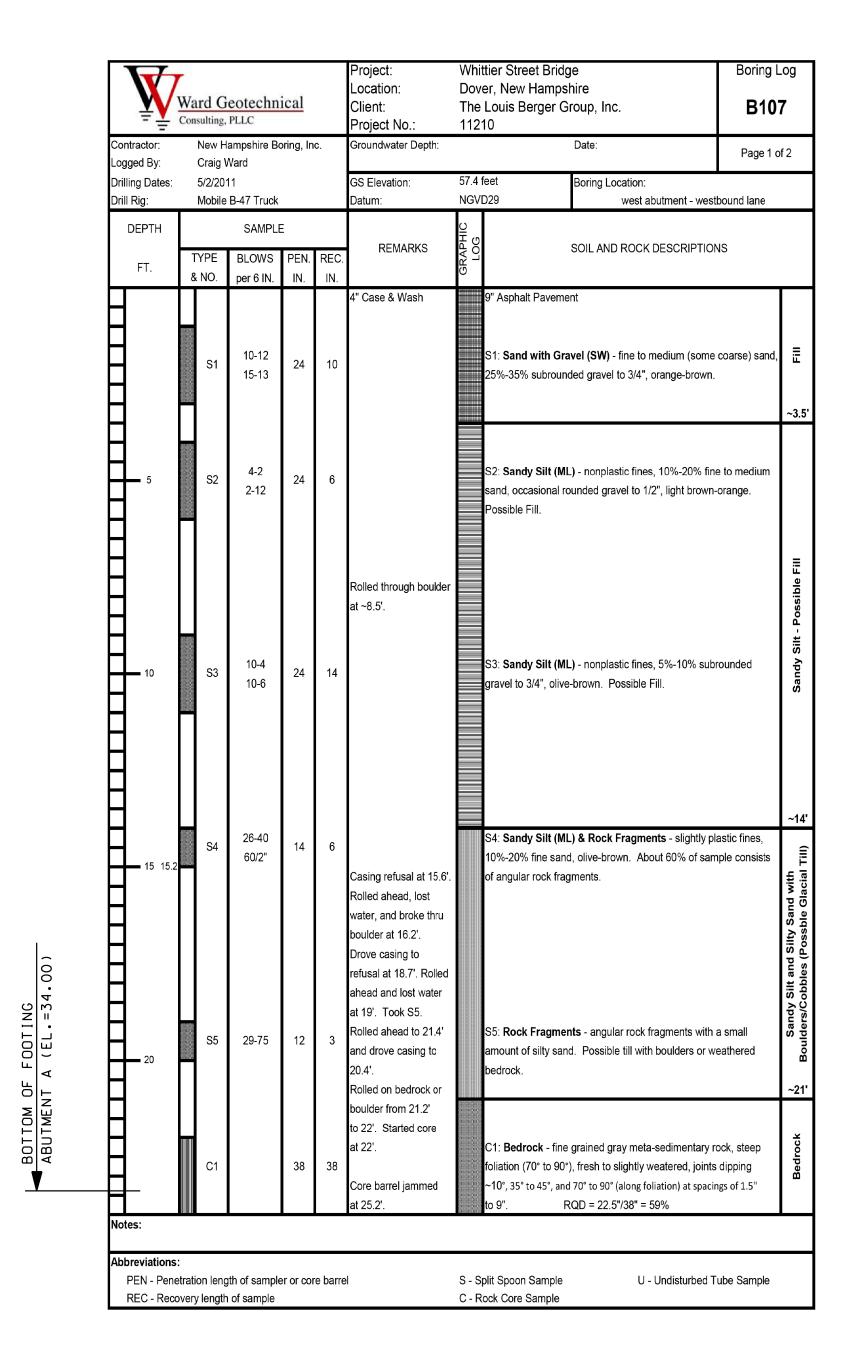
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<u>V</u>	Ward C	Geotechn	ical		Project: Location: Client:	Dove The	tier Street Bridge Borin er, New Hampshire Louis Berger Group, Inc. B1	
Contractor: Logged By:	New I Craig	Hampshire B	oring, In	IC.	Project No.: Groundwater Depth:	1121 59.9 f	Date: Page	1 of 1
Drilling Dates: Drill Rig:	4/29/2 Mobile	:011 e B-47 Truck			GS Elevation: Datum:	NGV[9	ar)
DEPTH	TVDE	SAMPL		DEO	REMARKS	GRAPHIC LOG	SOIL AND ROCK DESCRIPTIONS	
FT.	TYPE & NO.	BLOWS per 6 IN.	PEN. IN.	REC. IN.		GR. L		
	S1	6-4 5-6	24	22	4" Case & Wash		9" Asphalt Pavement S1: Silty Fine Sand (SM) - fine sand, 10%-30% (variable) nonplastic fines, occasional angular gravel to 3/4", light brown-orange.	-
5 - -	S2	5-4 5-5	24	20			S2: Silty Fine Sand (SM) - fine sand, 10%-30% (increasing witl depth) nonplastic fines, light brown-orange.	T
10	S3	4-3 2-2	24	21			S3: Silty Fine Sand (SM) - similar to S2.	-1
15	S4	26-22 15-21	24	7	Rolled through boulder		S4: Silty Sand with Gravel (SM) - fine to medium sand, 15%-2 nonplastic fines, 25%-35% subangular gravel to 3/4", olive-brow	
20	S5	24-25 26-35	24	17	from 16.8' to ~19'.		S5: Silty Sand with Gravel (SM) - fine to medium (some coars sand, 10%-20% nonplastic fines, 30%-40% subangular gravel to 1", olive-brown.	23
Notes:					Rolling on boulder or bedrock from 23.4' to 27.4'.		Bottom of Boring at 27.4'	Bedrock or
.5.05.							Bottom of Botting at 21.4	_

C - Rock Core Sample

BOTTOM OF FOOTING ANUTMENT B (EL.=35

REC - Recovery length of sample



	Ward G	eotechn PLLC	<u>ical</u>		Project: Location: Client: Project No.:	Whi Dov The 112	Boring L			
Contractor: Logged By:	New H Craig V	ampshire Bo Vard	oring, In	C.	Groundwater Depth:			Date:	Page 2 o	f 2
Drilling Dates: Drill Rig:	5/2/201				GS Elevation: Datum:	57.4 NGV		Boring Location: west abutment - westb	ound lane	
DEPTH		SAMPLE								
FT.	TYPE	BLOWS	PEN.	REC.	REMARKS	GRAPHIC LOG		SOIL AND ROCK DESCRIPTION	S	
25 25.2	& NO.	per 6 IN.	IN. 22	1N. 21	Coring rate varied from 5.5 to 7.5 min/foot. Coring rate of 7.5 min/foot.		slightly weathered,	grained gray meta-sedimentary ro ~45° foliation, quartz intrusion fron vizontal and dipping ~45° (along fo 0 6.5". RQD = 11.5"/22" = 52 Bottom of Boring at 27'.	n 25.8' to oliation) at s	Bedrock
	tration leng	th of sample	er or con	e barre			plit Spoon Sample	U - Undisturbed Tu	ibe Sample	
REC - Reco	very length	of sample				C-F	ock Core Sample			

				CIT	Y O	F DOVER	R, NE	W HA	MPSHIR	E		
				I	DEPAR	TMENT OF	COMM	UNITY S	SERVICES			
			LOCA	ATION WHITTIER STREET OV	ER COCH	IECO RIVER	BRIDGE	NO. 111\1	32 STA	TE PROJ	ECT 154	102
				BORING LOGS (SHEET 4 OF 5)								
				REVISIONS AFTER PROPOSAL			BY	DATE		BY	DATE	9 of 35
THE LO	ouis Berger	Group, INC.				DESIGNED	TW	P 11/15	CHECKED	KSW	11/15	7 - 33
Ma Ma	anchester, New	Hampshire				DRAWN	DW	M 11/15	CHECKED	KSW	11/15	FILE NUMBER
	(603) 644 520	00				QUANTITIES	TW	P 11/15	CHECKED	HNH	11/15	
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE				ISSUE DATE	=		AL PROJECT NO.	SHE	ET NO.	TOTAL SHEETS
d0174055	15402Borings04	AS NOTED				REV. DATE		X-A	.002(794)		14	58

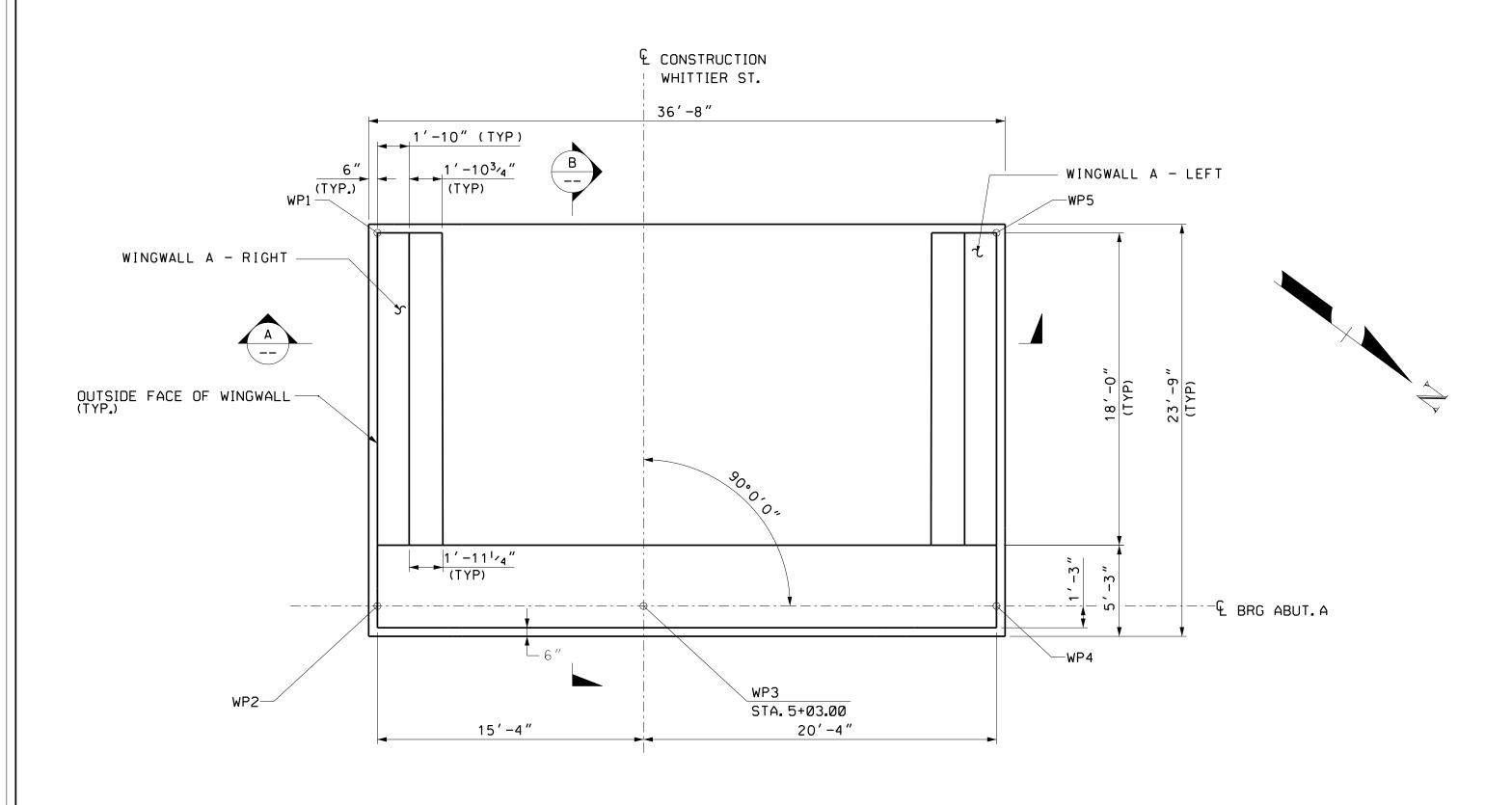
BORING NO. B108 STA. 4+81.2. 1.7' LT.

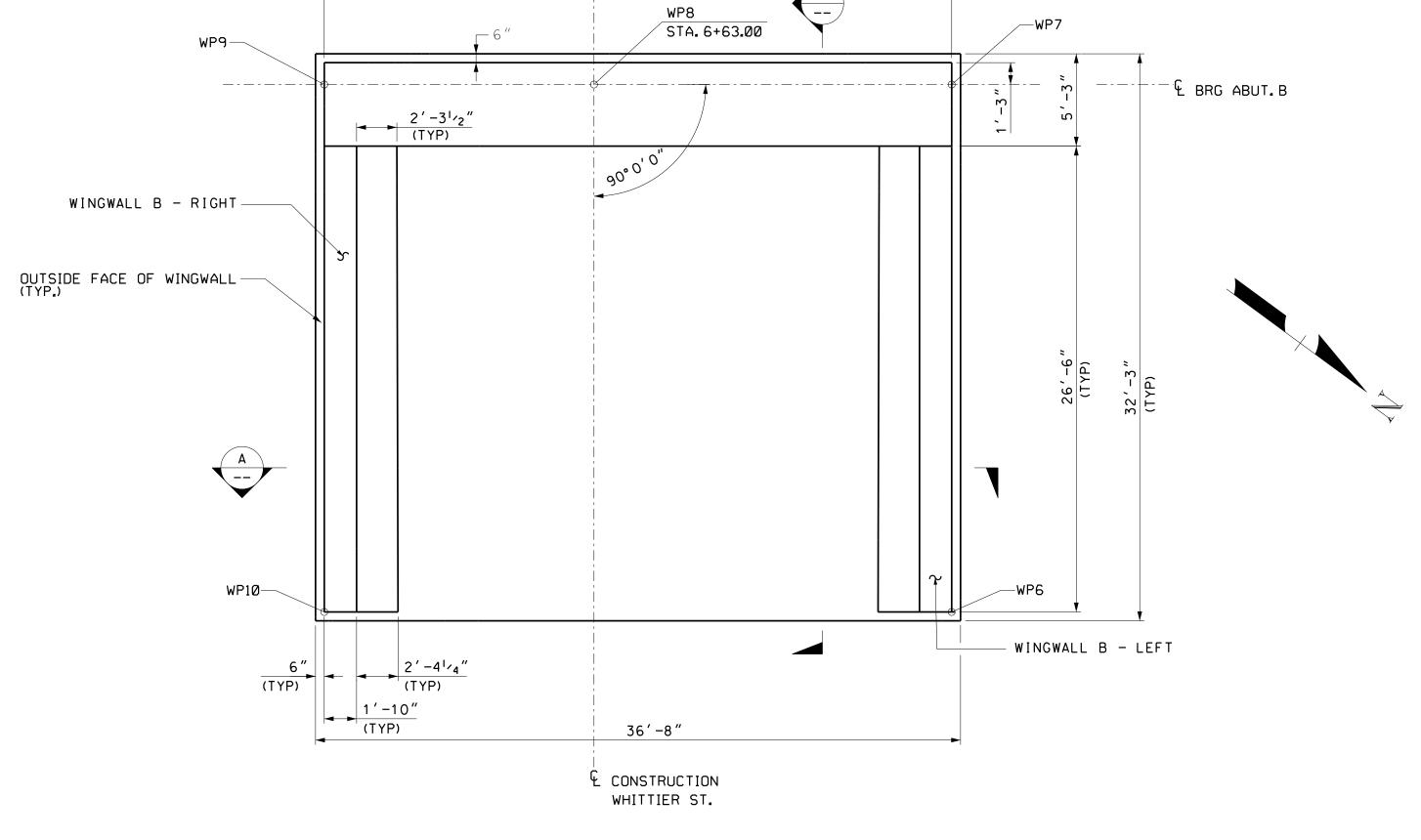
BORING NO. B109 STA. 4+74.1, 30.2' LT.

	Ward G	eotechn PLLC	ical		Project: Location: Client: Project No.:	Dov	ttier Street Bridge er, New Hampshire Louis Berger Group, Inc. 10	Boring L	
Contractor: Logged By:	New H Craig V	ampshire Bo Vard	oring, In	c.	Groundwater Depth:		Date:	Page 1 c	of 1
Drilling Dates: Drill Rig:	5/2/201 Mobile	I1 B-47 Truck			GS Elevation: Datum:	57.3 NGV	209 2004	ound lane (rear)	
DEPTH	TYPE	SAMPLE		l DE O	REMARKS	GRAPHIC LOG	SOIL AND ROCK DESCRIPTION	ONS	
FT.	& NO.	BLOWS per 6 IN.	PEN. IN.	REC. IN.		GR, L			
5	S1 S2	18-13 10-6 4-3 5-13	24	13	4" Case & Wash		10" Asphalt Pavement S1: Sand with Silt & Gravel (SP-SM) - fine to mee 5%-15% nonplastic fines, 20%-30% gravel, brown, fragment in tip of spoon. Due to poor recovery, overdrove 3" spoon: 20" recovery preserved to 2". Sand with Gravel (SW) - fine to coarse 35% subrounded & subangular gravel to 2", 5%-10 lower 8": Sandy Silt (ML) - nonplastic fines, 10%-2 medium sand, occasional fine roots, olive-brown. F S2: Sandy Silt (ML) - similar to lower 8" of S1 over Possible Fill.	Rock overy: sand, 25%- % fines, brown. 20% fine to Possible Fill.	Sandy Silt - Possible Fill
10 9.7	S3	22-50/2"	8	0	Increased casing resistance below 8'. Lost water at 9'. Rolled ahead and drove casing to 14'.		S3: No Recovery - probably pushed boulder with s	spoon.	Silty Sand with Gravel & Cobbles/
14.3	S 4	50/3"	3	1	Rolled ahead. Cuttings in wash appear to be weathered rock. Roller bit cut rapidly from 15' to 16', then slowed. Lost water at 16.4'. Rolled to 18' in boulder or bedrock.		S4: Silty Sand with Gravel (SM) - fine to coarse s nonplastic fines, 40%-50% angular gravel and rock olive-brown.		Boulder or Bedrock
20					in boulder of bedrock.		Bottom of Boring at 18'		8
Notes: Abbreviations PEN - Pene	etration leng		er or co	re barre	<u>-</u>		plit Spoon Sample U - Undisturbed ock Core Sample	Tube Sample	

= ₹ (Consulting,				Project: Location: Client: Project No.:	Dov					
Contractor: .ogged By:	Craig \		oring, In	C.	Groundwater Depth:		Date		Page 1 o	f 1	
Orilling Dates: Orill Rig:	5/3/20 Remot	11 e ATV Rig			GS Elevation: Datum:	47.5 NGV	20	ng Location: toe of embankment at no	orthwest quadrant	t	
DEPTH FT.	TYPE	SAMPLE BLOWS	PEN.		REMARKS	GRAPHIC LOG	SOIL	. AND ROCK DESCRIPTI	ONS		
1.8 - 10 - 15 - 20 - 20	& NO.	2-4 12-100/4"	IN. 22	1N. 16	4" Case & Wash Rolled in bedrock to 3' to core. Coring rate varied from 3 to 4 min/foot. Core barrel dropped ~1" at 4.9'.		roots, brown. lower 5": Sand with Grasand, 30%-40% subrounto 3/4", dry, light brown-c C1: Bedrock - fine grainto slightly weathered, value of the signification of the signification of the signification of the signification of the significant of the significa	- nonplastic fines, 20%-3 avel (SW) - fine to medium aded gravel and angular ro	ock fragments y rock, fresh to 80° (variable), z veins, joints	Also Company of the State of th	

				CITY OF DOVER, NEW HAMPSHIRE											
				DEPARTMENT OF COMMUNITY SERVICES											
			LOCA	_											
				BORIN	VG L	OGS (SH	EET:	5 OF 5)		BRIDGE SHEET					
<u> </u>	ouio Dargar (C 14 C 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		REVISIONS AFTER PROPOSAL			В		BY DATE	10 of 35					
	ouis Berger (roup,INC.				DESIGNED	TV	VP 11/15 CHECKED	KSW 11/15						
Ma Ma	anchester, New H	ampshire				DRAWN	DW	VM 11/15 CHECKED	KSW 11/15	FILE NUMBER					
	(603) 644 5200	•				QUANTITIES	TV	VP 11/15 CHECKED	HNH 11/15						
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE		ISSUE DATE = FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS											
d0174055	15402Borings05	AS NOTED				REV. DATE		X-A002(794)	15	58					



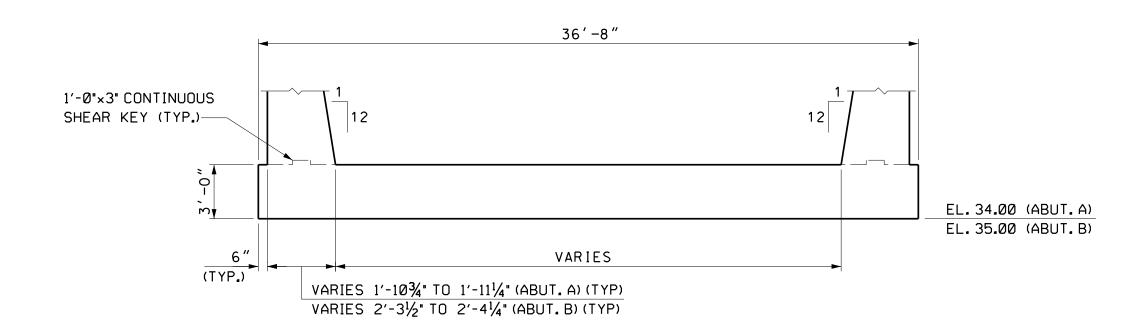


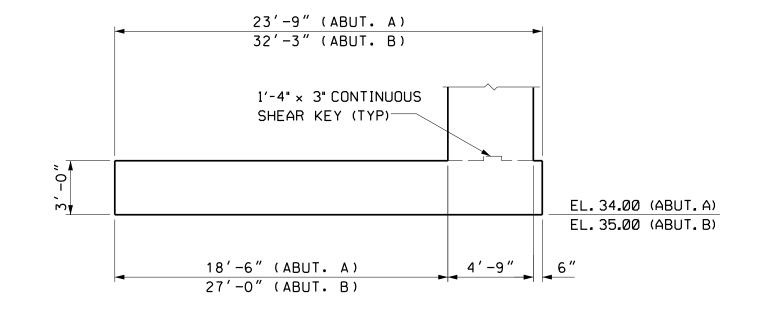
20'-4"

FOOTING LAYOUT PLAN - ABUTMENT A SCALE: 3/16" = 1'-0"

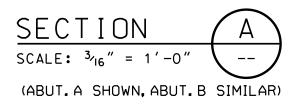
FOOTING LAYOUT PLAN - ABUTMENT B

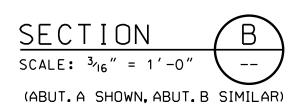
SCALE: 3/16" = 1'-0"



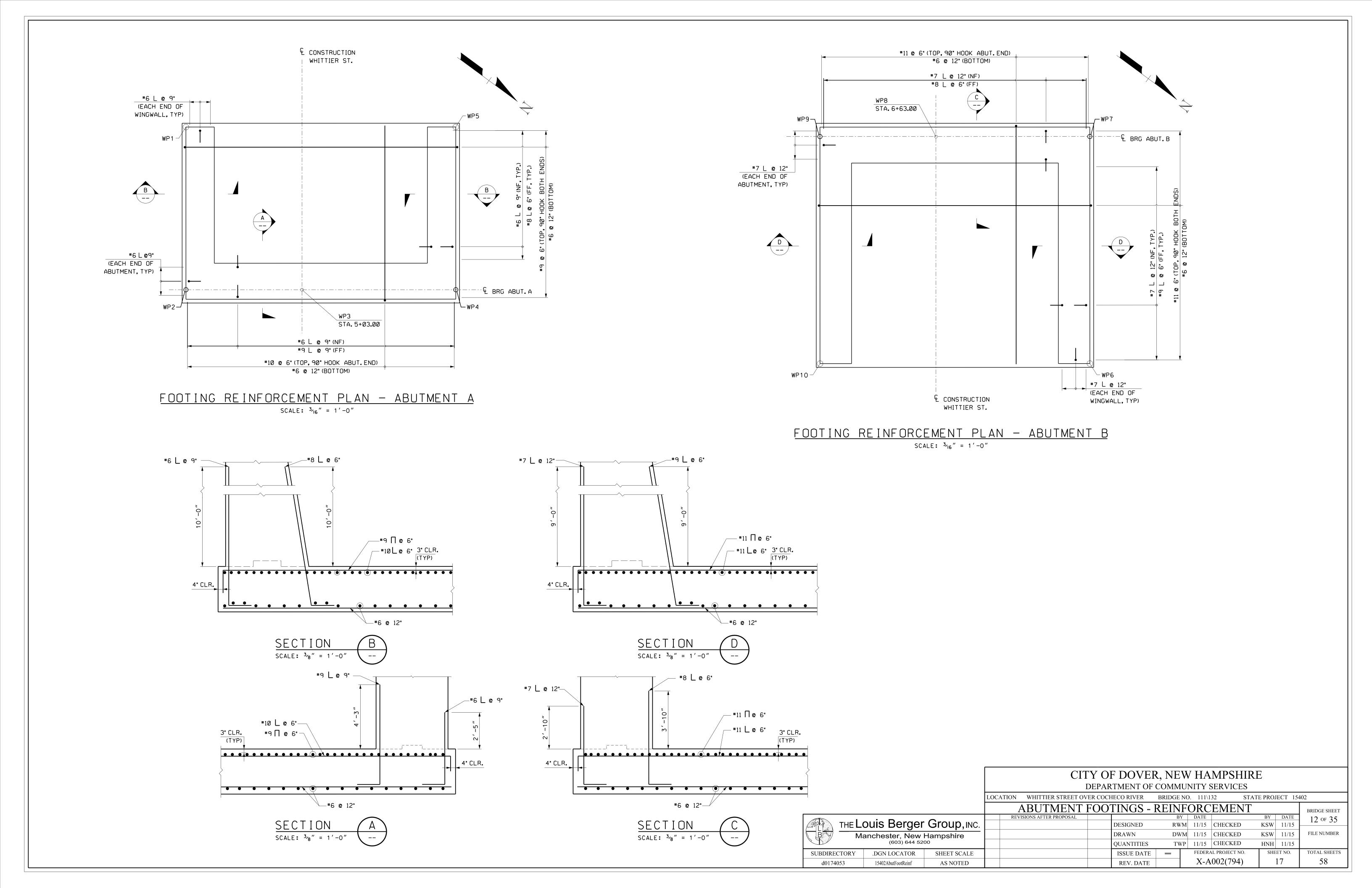


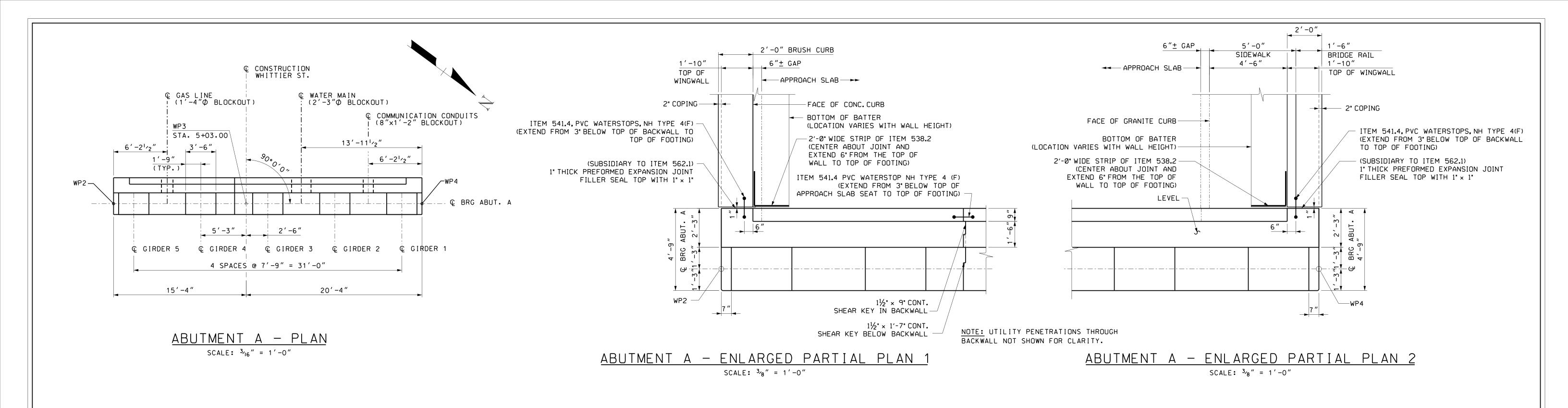
15′-4″

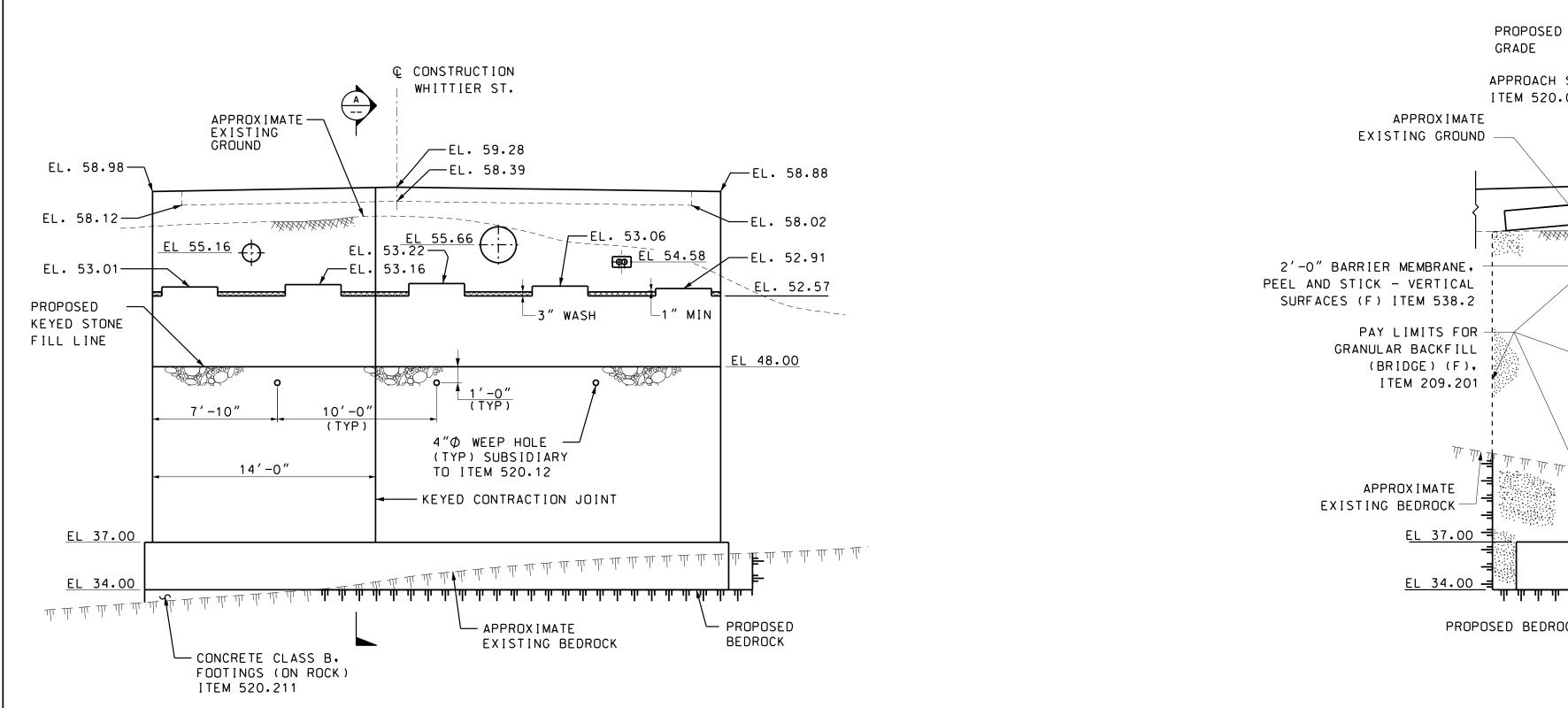




	CITY OF DOVER, NEW HAMPSHIRE								
	DEPARTMENT OF COMMUNITY SERVICES								
	LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402								
	ABUTMENT FOOTINGS - MASONRY B	BRIDGE SHEET							
TUEL QUIE BORGOT GROUP INC	REVISIONS AFTER PROPOSAL BY DATE BY DATE DEGICALED BY DATE 1/15 CHECKED WOW 11/15	11 of 35							
THE Louis Berger Group, INC.									
Manchester, New Hampshire	DRAWN DWM 11/15 CHECKED KSW 11/15 F	FILE NUMBER							
(603) 644 5200	QUANTITIES TWP 11/15 CHECKED HNH 11/15								
SUBDIRECTORY .DGN LOCATOR SHEET SCALE	ISSUE DATE = FEDERAL PROJECT NO. SHEET NO. T	TOTAL SHEETS							
d0174053 15402AbutFoot AS NOTED	REV. DATE X-A002(794) 16	58							







<u>ABUTMENT A - ELEVATION</u>

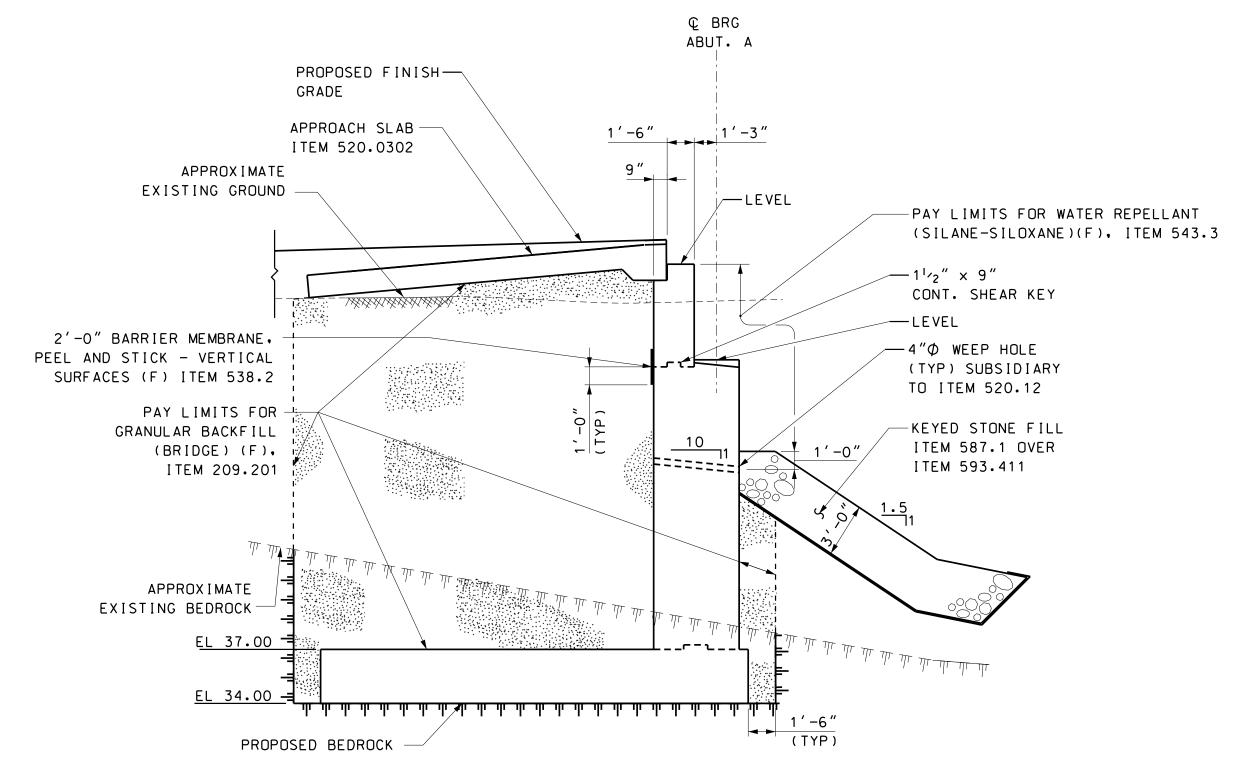
SCALE: $\frac{3}{16}'' = 1' - 0''$

1. ELEVATIONS AT TOP OF BACKWALL TAKEN AT BACK FACE.

APPROXIMATE AND TO BE CONFIRMED IN FIELD BY OTHERS.

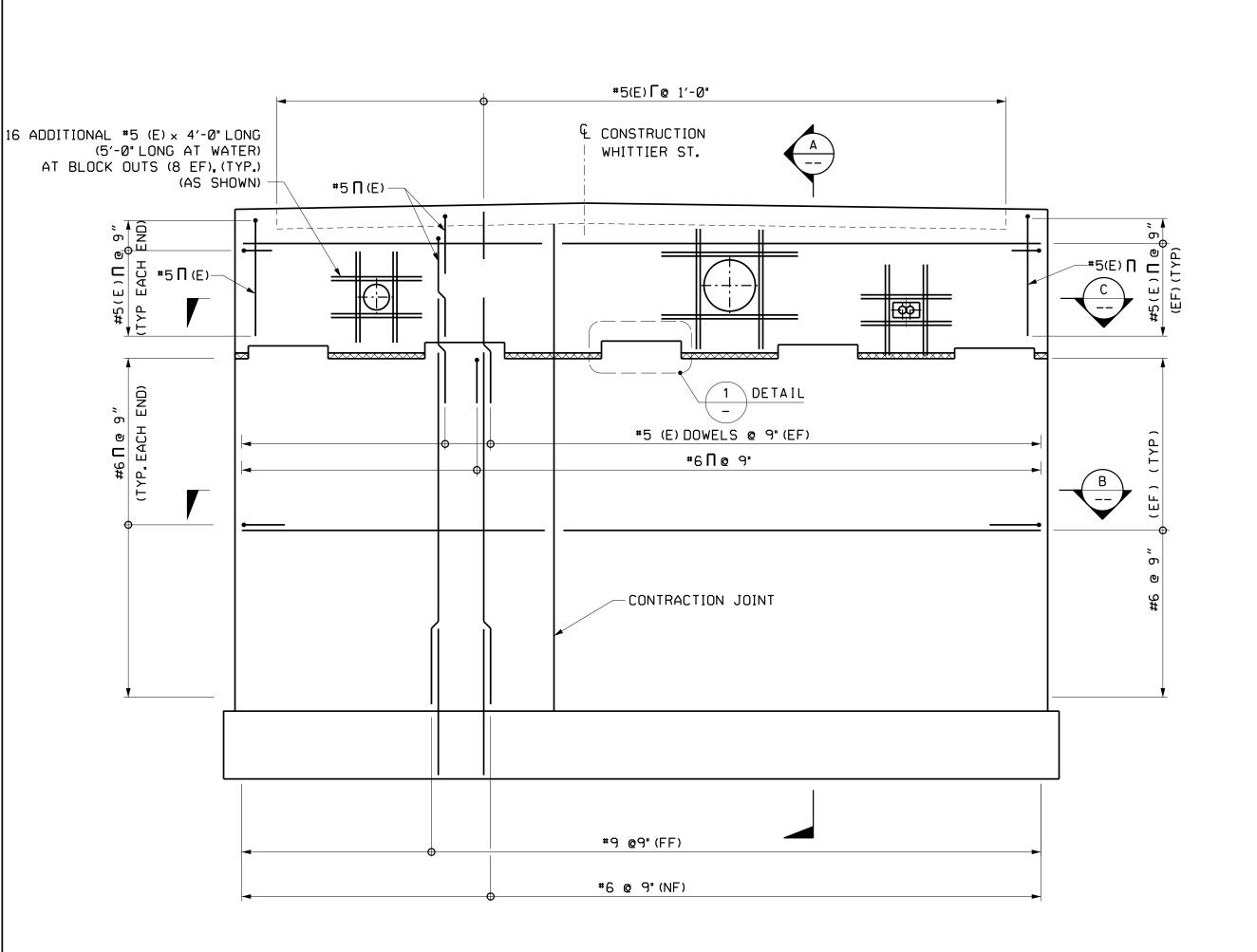
2. ELEVATIONS OF UTILITIES THROUGH BACKWALL ARE

3. BEAM SEATS SHALL BE LEVEL.



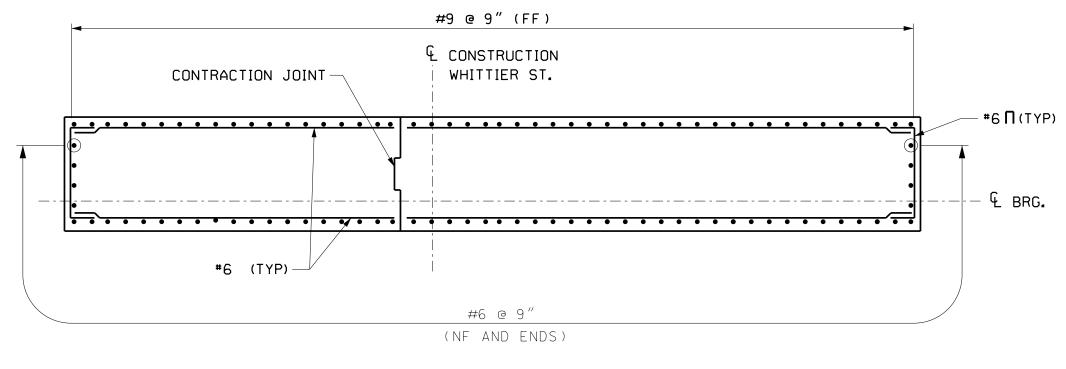
<u>SECTION A</u> SCALE: 3/16" = 1'-0"

> CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402 ABUTMENT A - MASONRY BRIDGE SHEET REVISIONS AFTER PROPOSAL 13 of 35 THE Louis Berger Group, INC DESIGNED RWM 11/15 CHECKED KSW 11/15 FILE NUMBER KSW | 11/15 Manchester, New Hampshire (603) 644 5200 **DRAWN** DWM 11/15 CHECKED **QUANTITIES** TWP 11/15 CHECKED HNH 11/15 FEDERAL PROJECT NO. SHEET NO. TOTAL SHEETS .DGN LOCATOR SHEET SCALE ISSUE DATE SUBDIRECTORY X-A002(794) 58 15402Abut-Plan-Elev AS NOTED REV. DATE d0174053

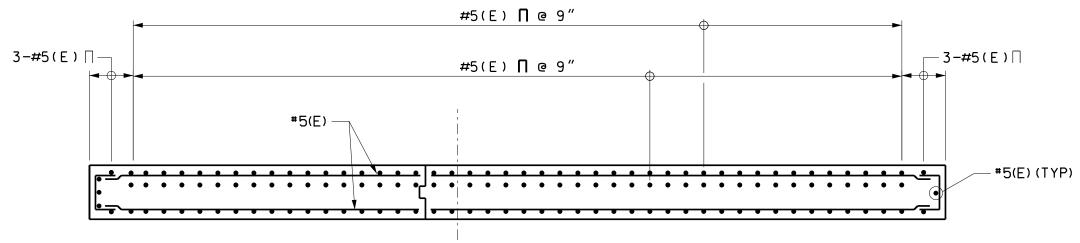


ABUTMENT A ELEVATION

SCALE: 1/4" = 1'-0"

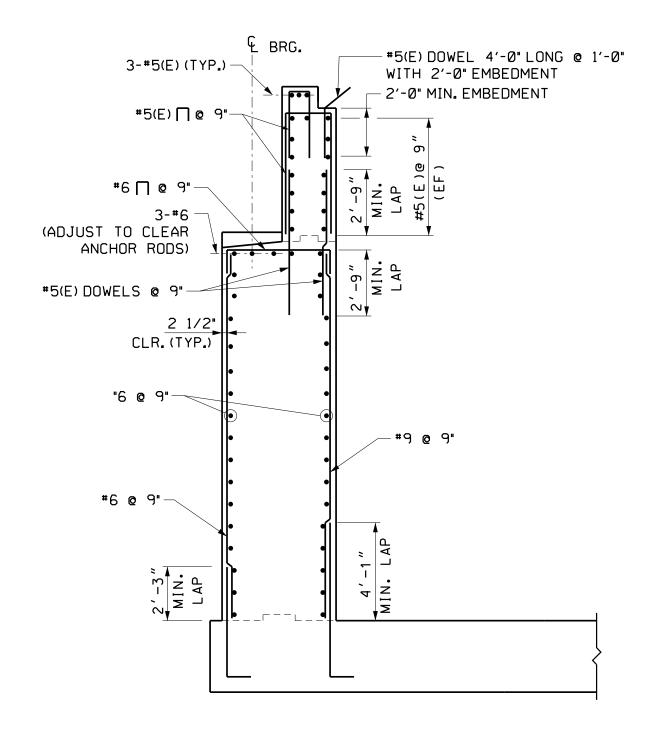


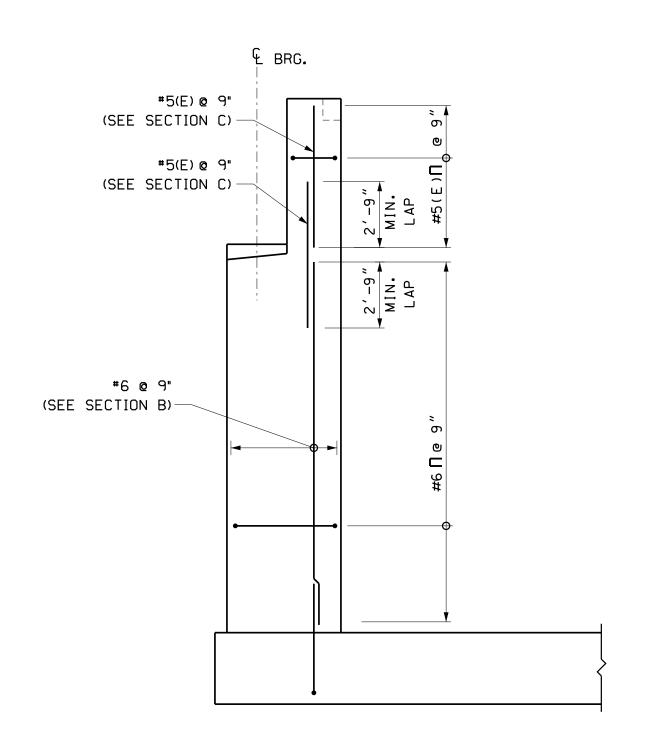
SECTION B SCALE: 1/4" = 1'-0"



& CONSTRUCTION WHITTIER ST.

SECTION C SCALE: 1/4" = 1'-0"



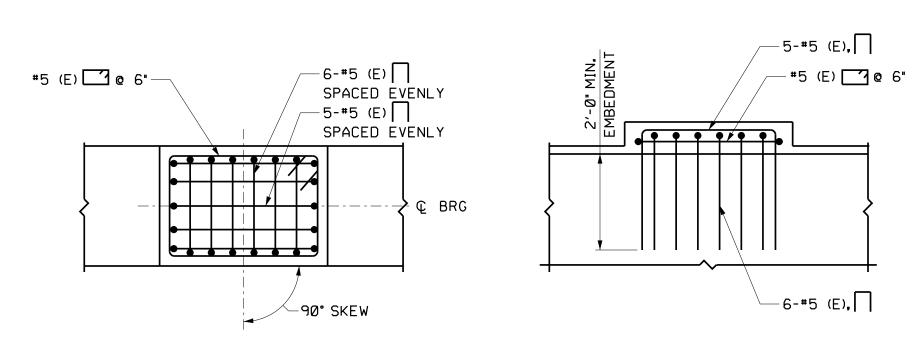


SECTION A SCALE: 1/4" = 1'-0" LEFT END ELEVATION

(RIGHT END ELEVATION SIMILAR)

SCALE: 1/4" = 1'-0"

X-A002(794)



AS NOTED

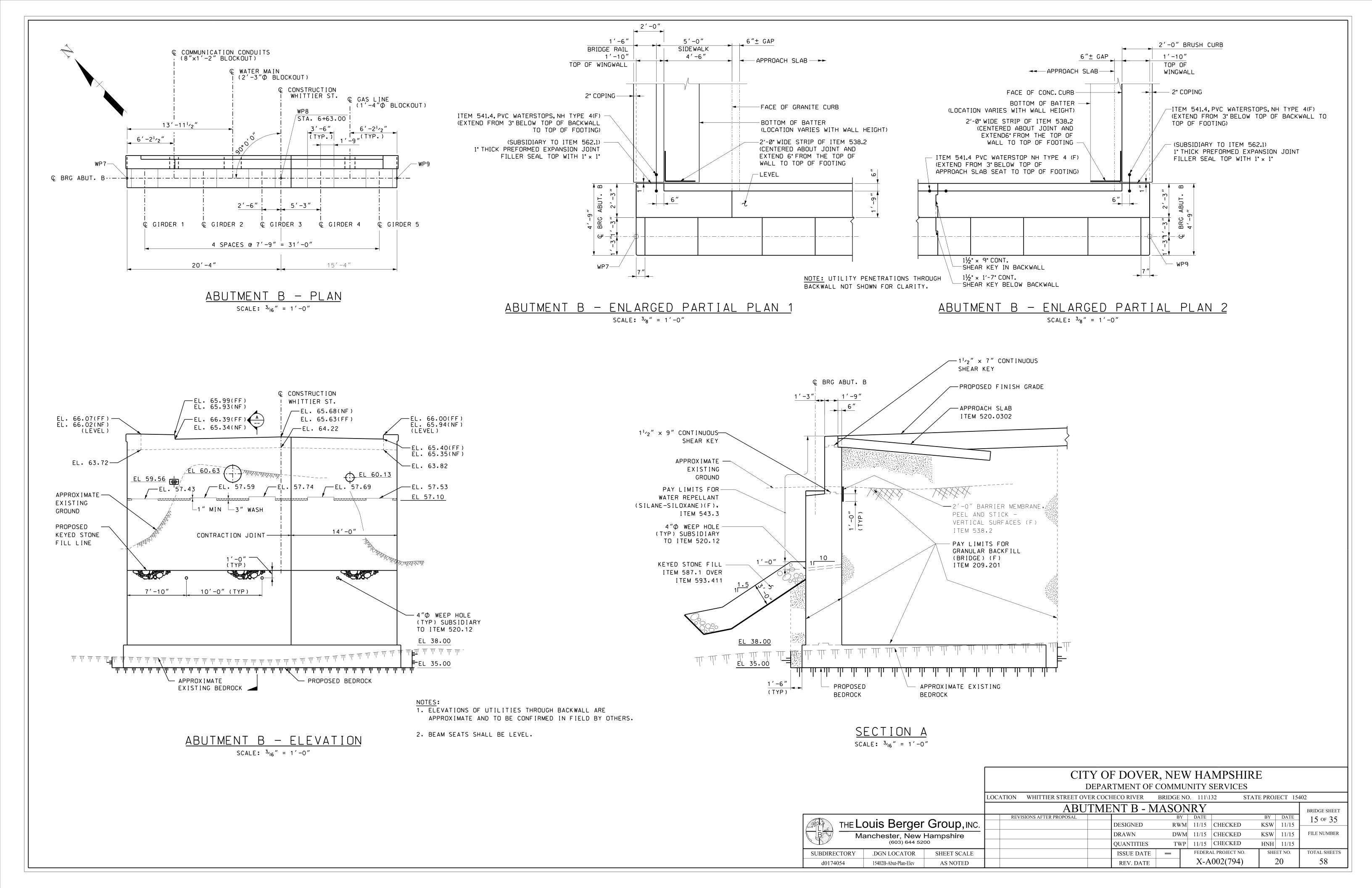
15402AbutDetls01

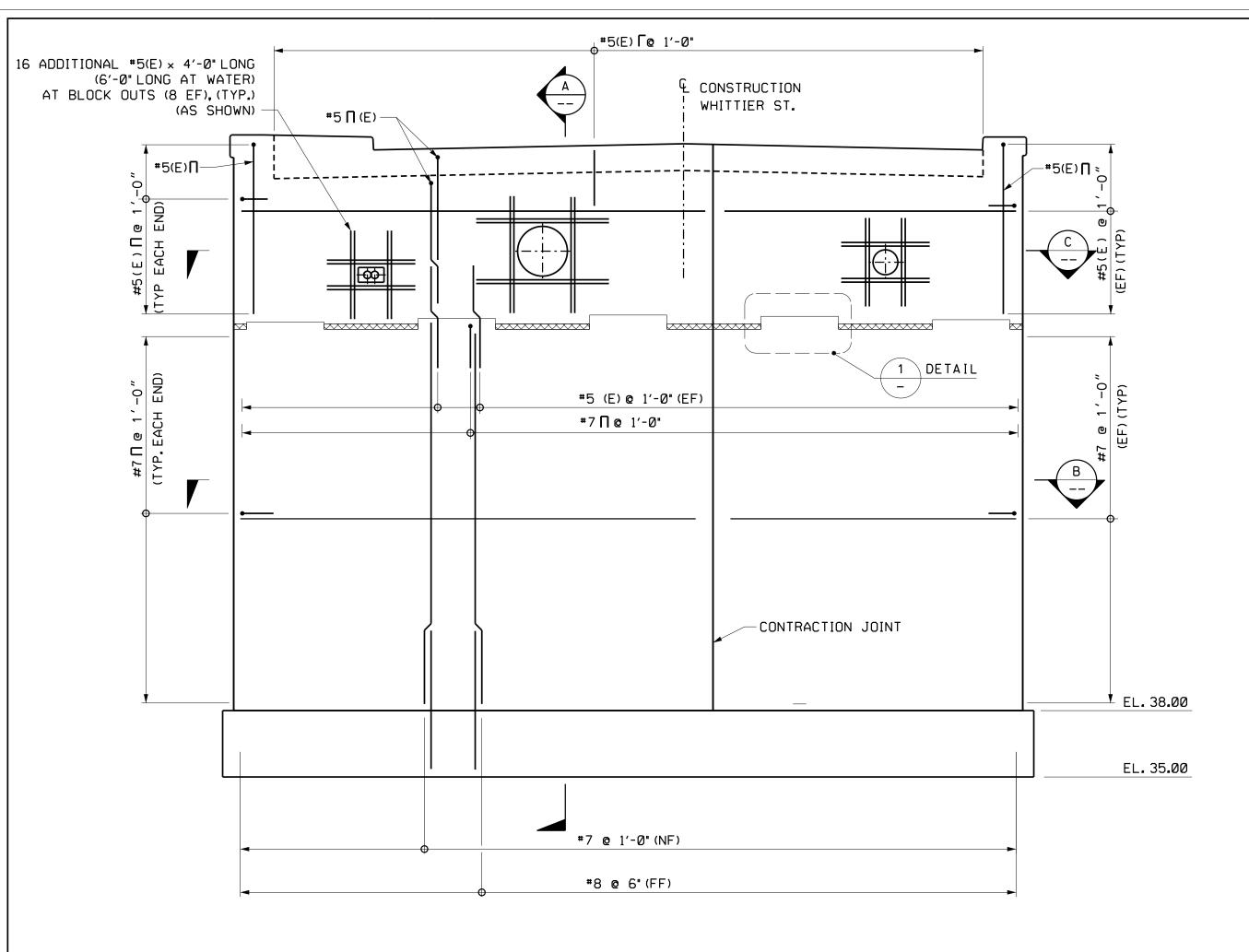
d0174053



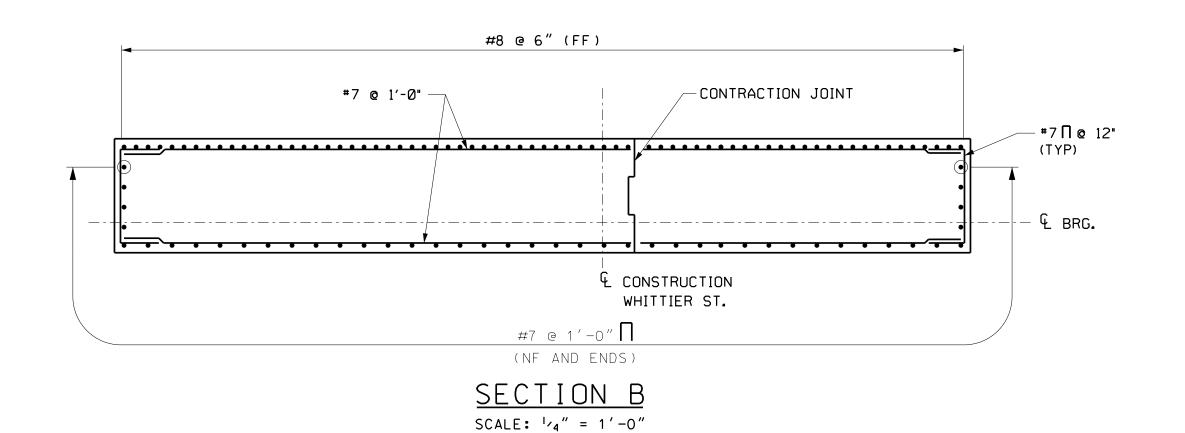
		CIT	YO	F DOVER	R, NE	W HA	MPSHIR	Ε		
		I	DEPAR	RTMENT OF	COMM	UNITY	SERVICES			
	LOCA	ATION WHITTIER STREET OV	ER COCI	HECO RIVER	BRIDGE 1	NO. 111\	132 STA	TE PROJ	ECT 154	102
		ABUTMI	ENT	A - REIN	FORG	CEME	ENT			BRIDGE SHEET
Louis Dorger Croup		REVISIONS AFTER PROPOSAL			BY			BY	DATE	14 of 35
THE Louis Berger Group, INC.				DESIGNED	RW	M 11/15	CHECKED	KSW	11/15	
Manchester, New Hampshire				DRAWN	DW	M 11/15	CHECKED	KSW	11/15	FILE NUMBER
(603) 644 5200				QUANTITIES	TW	P 11/15	CHECKED	HNH	11/15	
SUBDIRECTORY .DGN LOCATOR SHEET SCALE				ISSUE DATE	_	FEDER	AL PROJECT NO.	SHE	ET NO.	TOTAL SHEETS

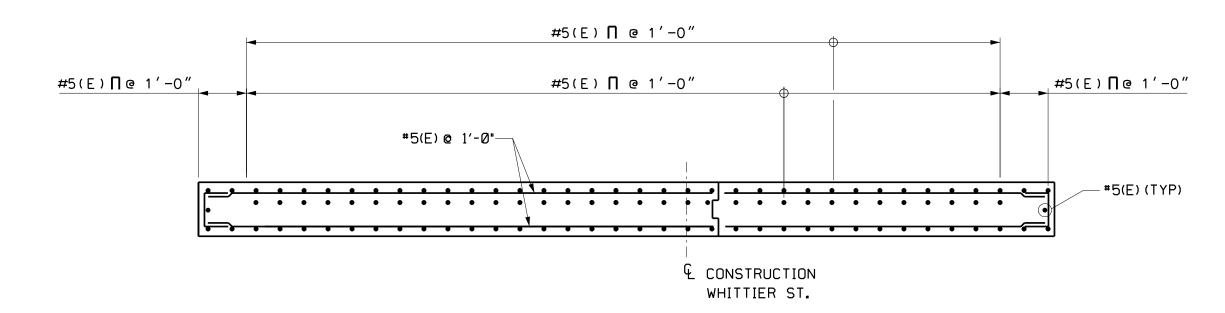
REV. DATE



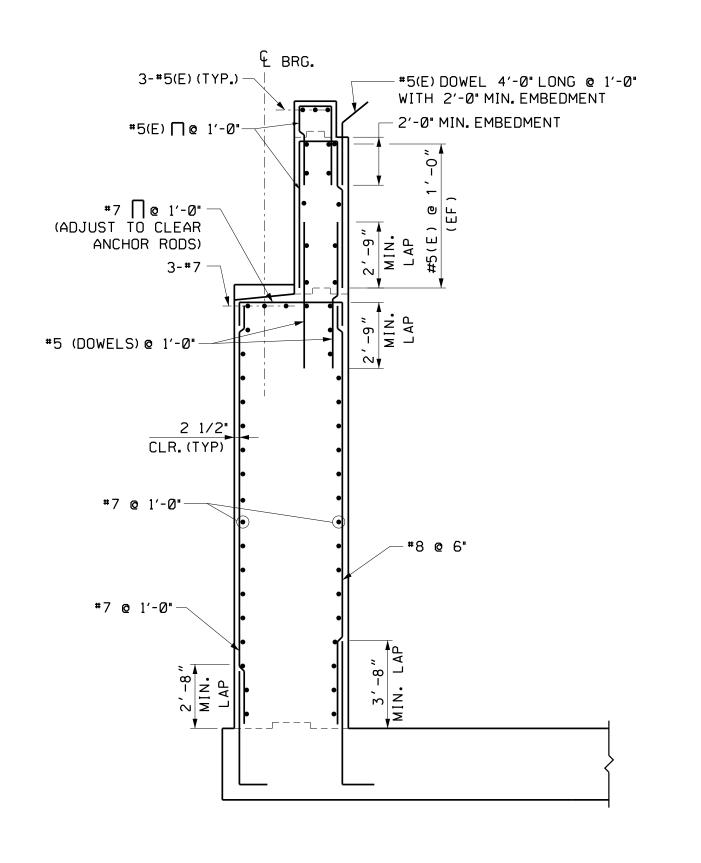


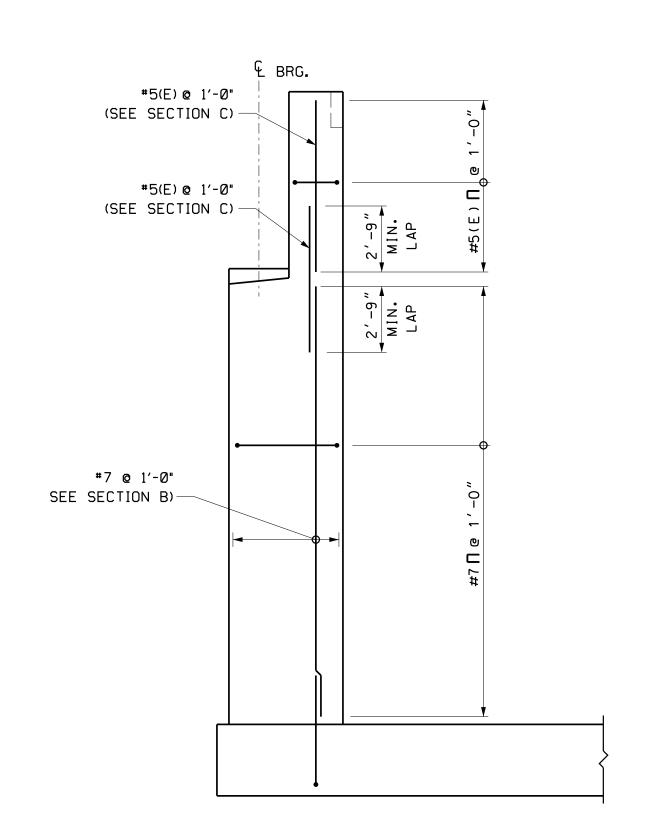
ABUTMENT B ELEVATION SCALE: 1/4" = 1'-0"





SECTION C SCALE: 1/4" = 1'-0"





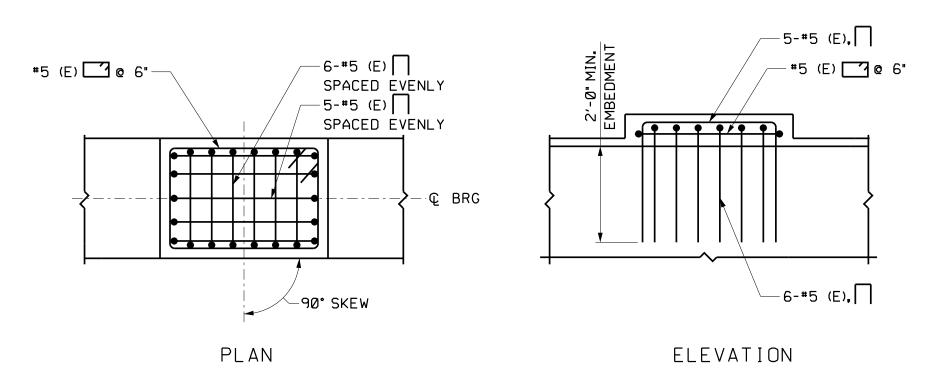
SECTION A SCALE: 1/4" = 1'-0"

d0174054

RIGHT END ELEVATION
(LEFT END ELEVATION, SIMILAR)

X-A002(794)

21



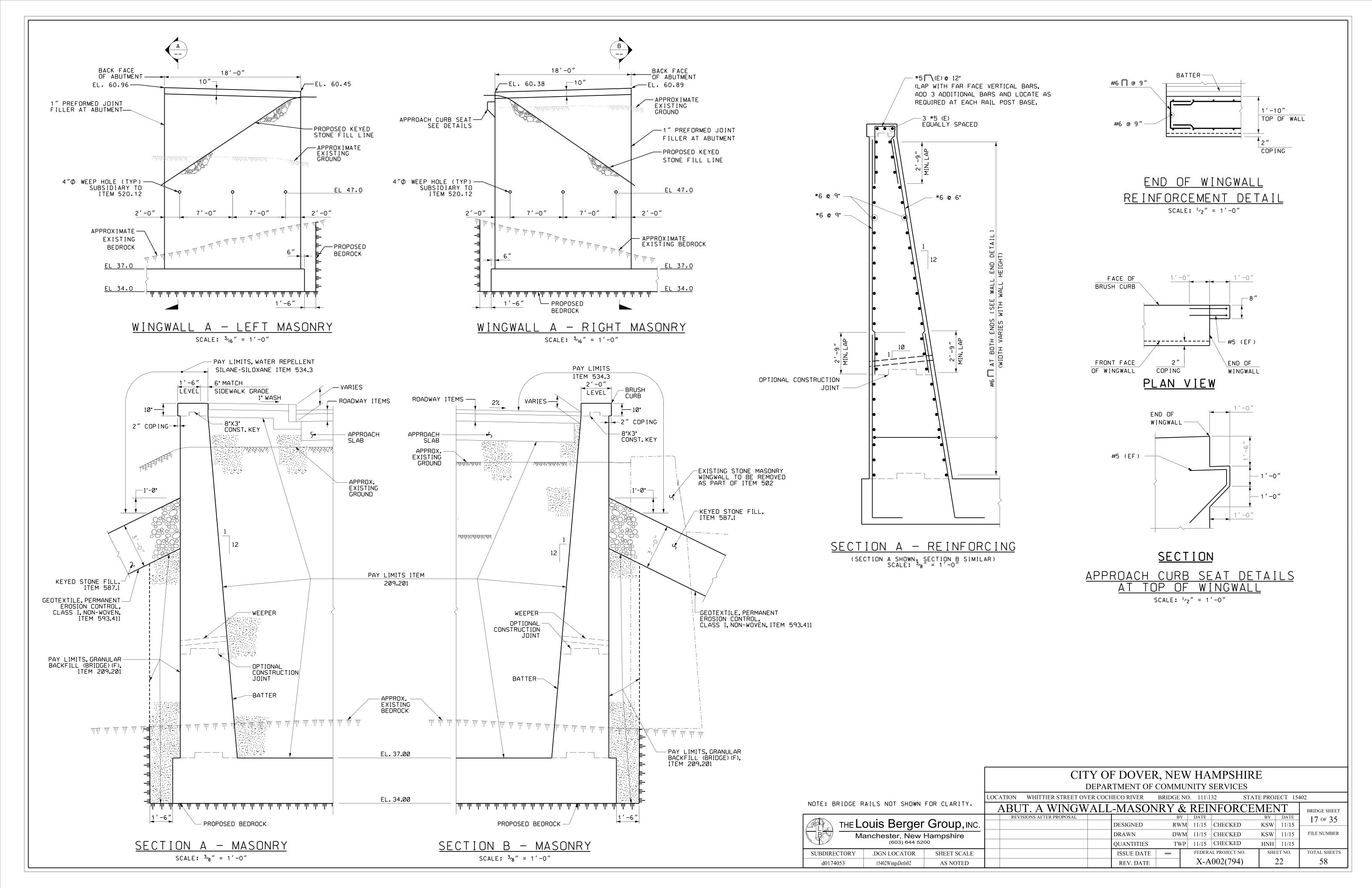


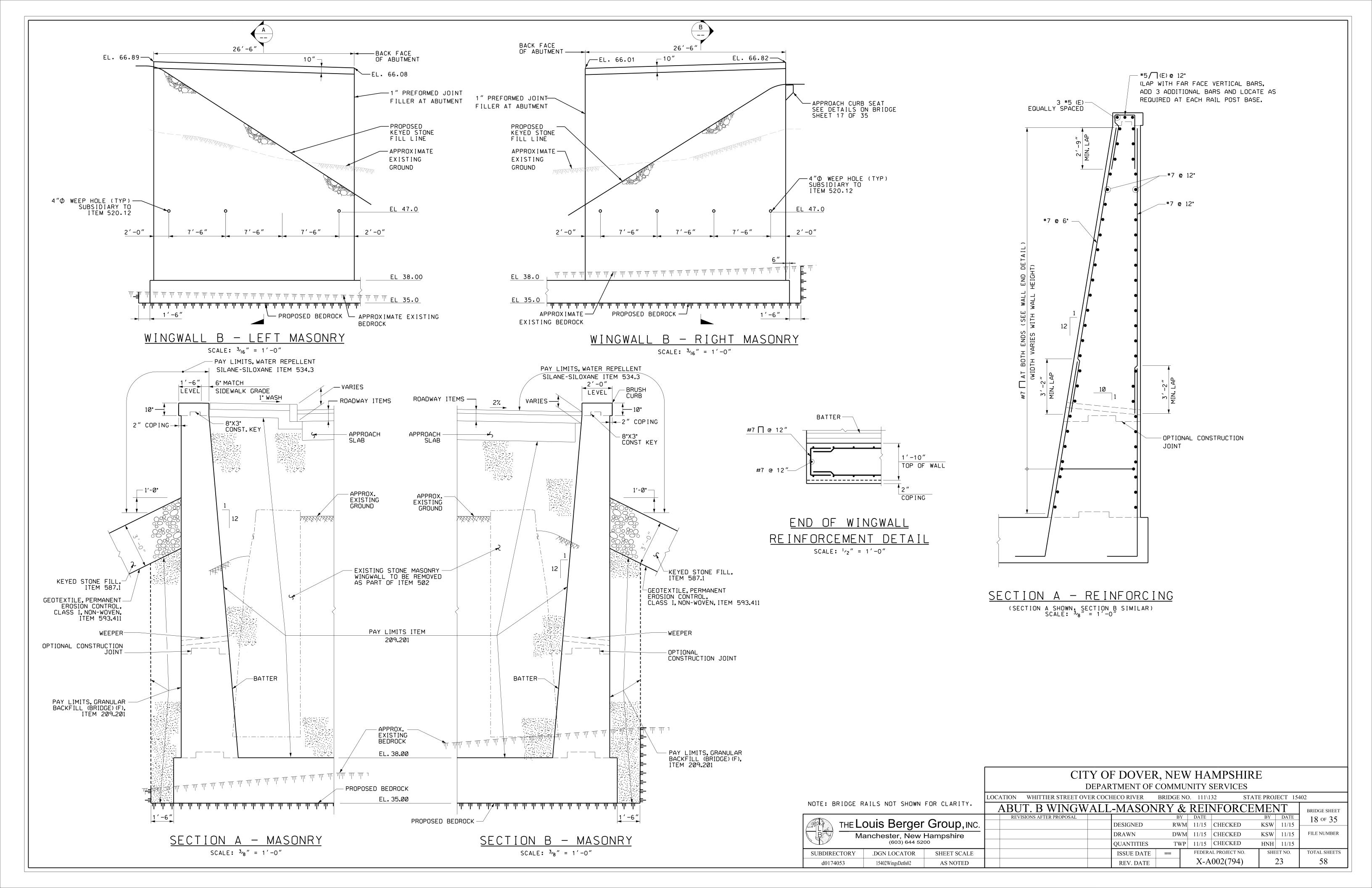
AS NOTED

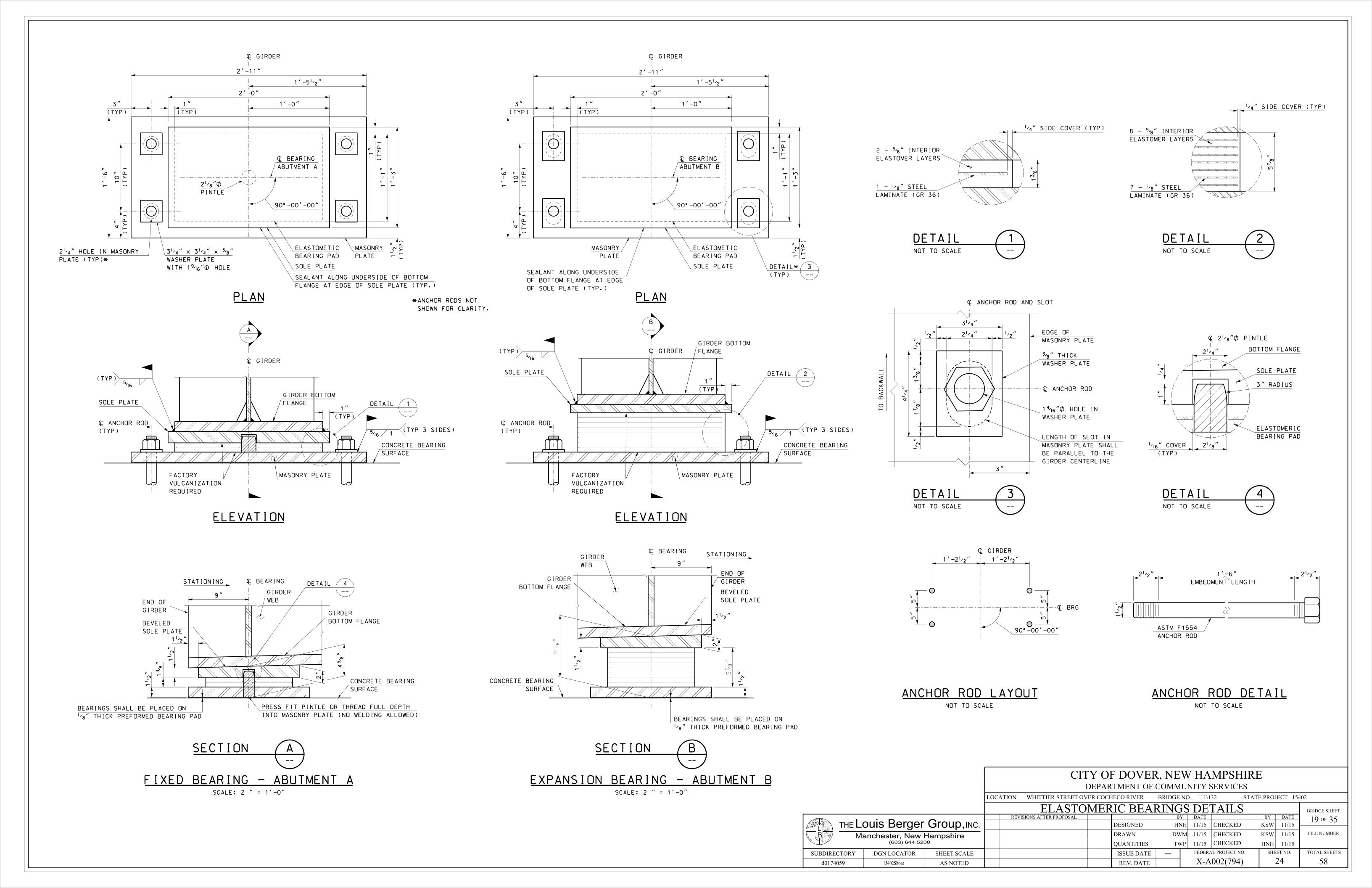
15402B-AbutDetls01

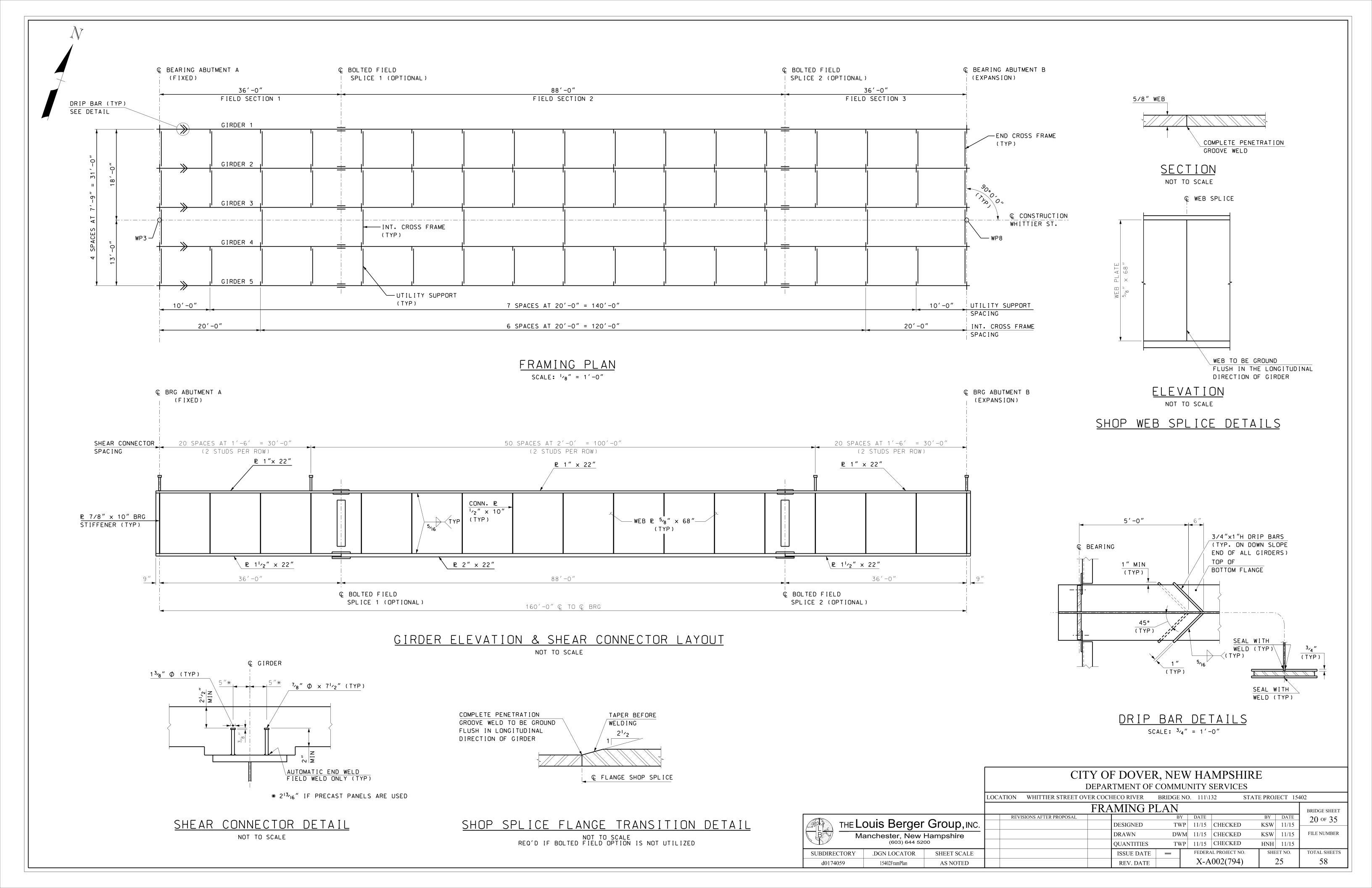
				CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES												
			LOCA	ATION WHITTIER STREET OV	ER COCI	HECO RIVER	BRIDGE	NO. 111\132	STAT	E PROJ	ECT 154	102				
				ABUTMENT B - REINFORCEMENT BRIDGE SHEET												
	'- D	0		REVISIONS AFTER PROPOSAL			В	Y DATE		BY	DATE	16 of 35				
THE LO	ouis Berger	Group, INC.				DESIGNED	RW	/M 11/15 CHE	CKED	KSW	11/15					
Ma Ma	anchester, New	Hampshire				DRAWN	DW	VM 11/15 CHE	CKED	KSW	11/15	FILE NUMBER				
	(603) 644 520	00				QUANTITIES	TV	VP 11/15 CHE	CKED	HNH	11/15					
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE				ISSUE DATE	=	FEDERAL PRO	IECT NO.	SHE	ET NO.	TOTAL SHEETS				

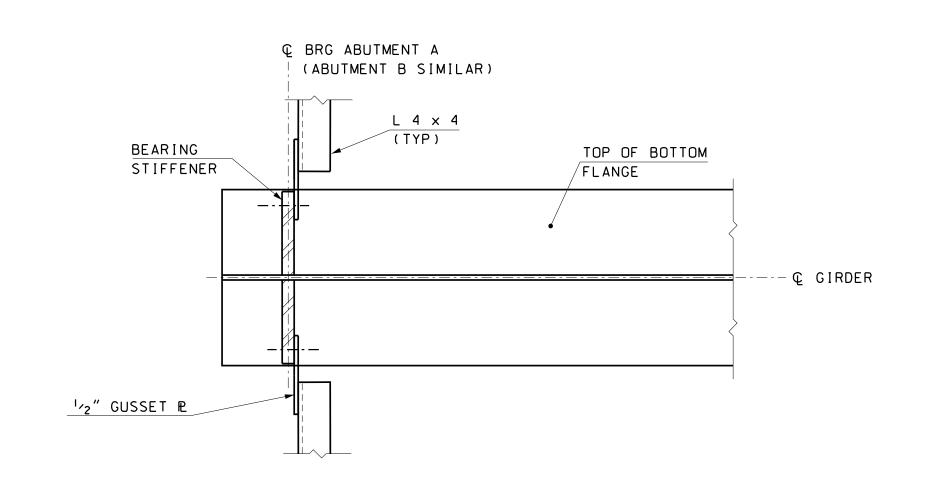
REV. DATE

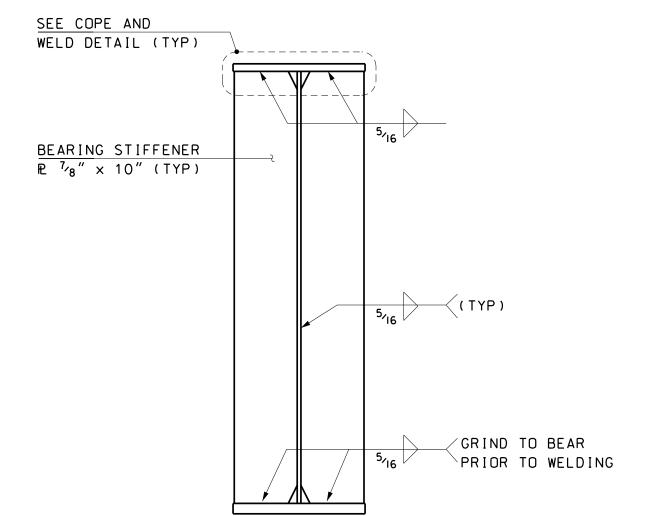


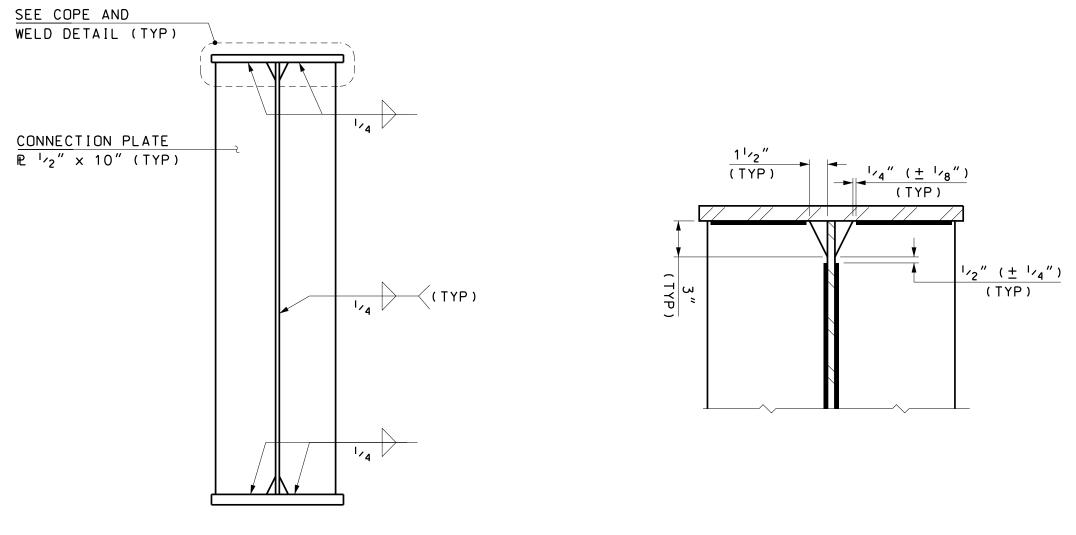












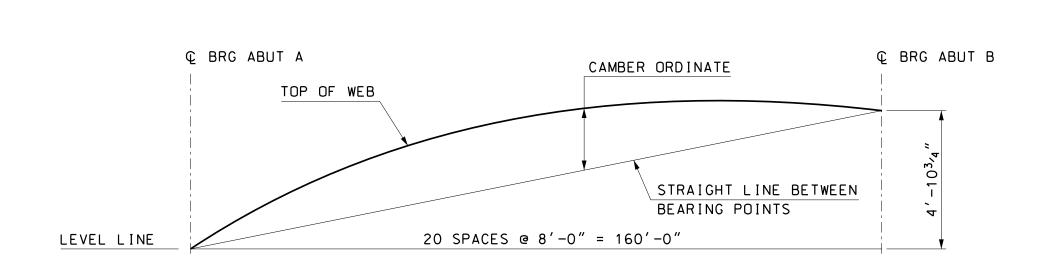
END CROSS FRAME CONNECTION DETAIL SCALE: 1" = 1'-0"

BEARING STIFFENER SCALE: $\frac{3}{4}$ " = 1'-0"

CONNECTION PLATE SCALE: $\frac{3}{4}$ " = 1'-0"

COPE AND WELD DETAIL SCALE: 11/2" = 1'-0"

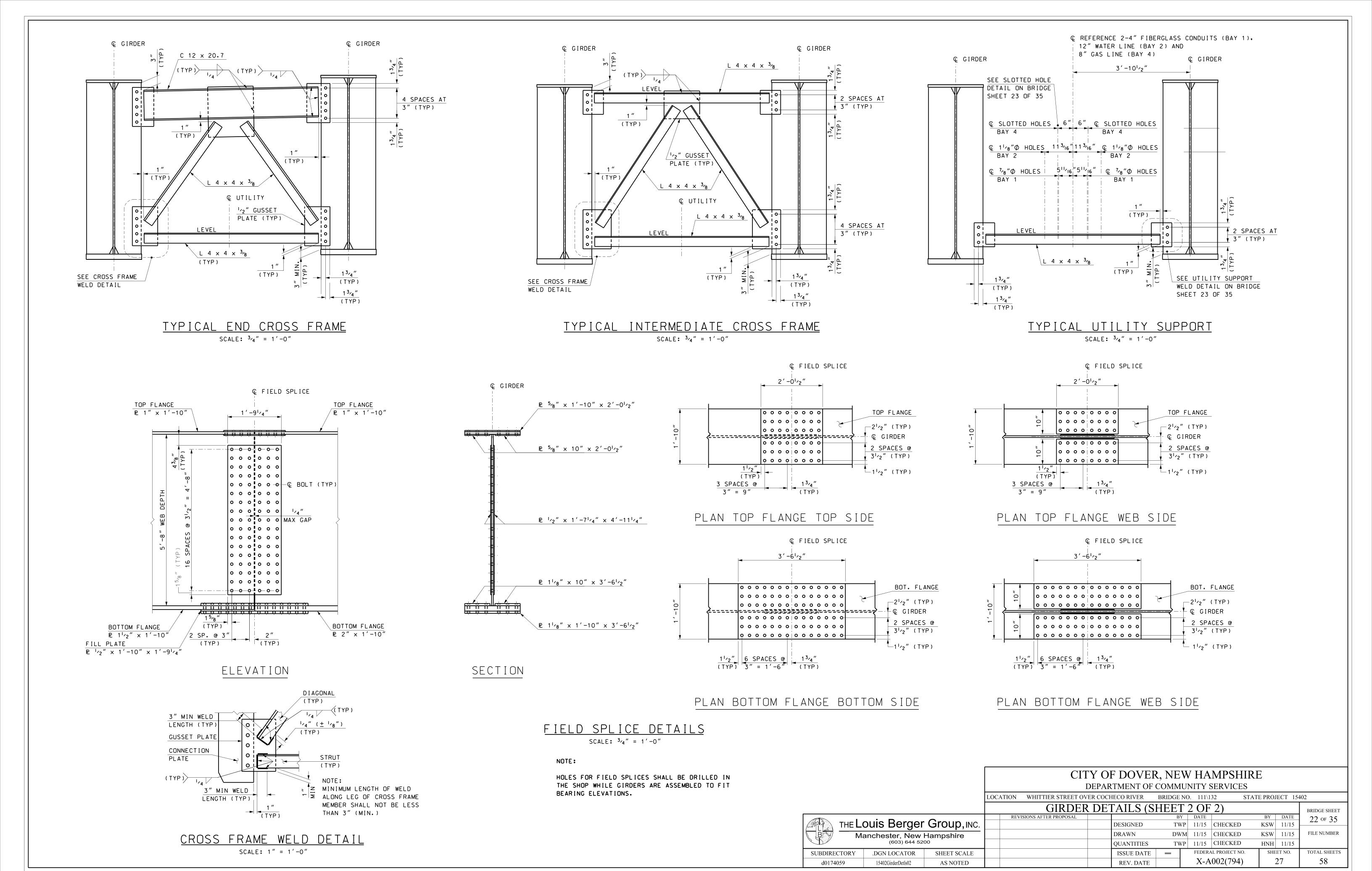
								-	TABLE OF C	AMBER AT	20TH POIN	NTS (INCHE	:S)									
	POINT ALONG SPAN	CL BRG ABUT A	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	CL BRG ABUT B
	STEEL DL DEFL	0.00	0.38	0.75	1.10	1.42	1.69	1.93	2.12	2.25	2.33	2.36	2.33	2.25	2.11	1.92	1.69	1.41	1.09	0.75	0.38	0.00
-	CONC SLAB DEFL	0.00	0.67	1.32	1.92	2.48	2.96	3.37	3.70	3.93	4.08	4.12	4.08	3.93	3.70	3.37	2.96	2.48	1.92	1.32	0.67	0.00
)ER	SUPERIMPOSED DL DEFL	0.00	0.47	0.92	1.34	1.73	2.06	2.35	2.58	2.74	2.84	2.87	2.84	2.74	2.58	2.35	2.06	1.73	1.34	0.92	0.47	0.00
GIRDI	TOTAL DEFLECTION	0.00	1.52	2.99	4.37	5.62	6.72	7.65	8.39	8.92	9.25	9.35	9.24	8.92	8.38	7.64	6.71	5.61	4.36	2.98	1.52	0.00
	VC ORDINATE	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	TOTAL CAMBER	0.00	1.52	2.99	4.37	5.62	6.72	7.65	8.39	8.92	9.25	9.35	9.24	8.92	8.38	7.64	6.71	5.61	4.36	2.98	1.52	0.00
	STEEL DL DEFL	0.00	0.45	0.89	1.30	1.67	2.00	2.28	2.50	2.66	2.75	2.78	2.75	2.65	2.49	2.27	1.99	1.66	1.29	0.88	0.45	0.00
7	CONC SLAB DEFL	0.00	0.79	1.56	2.29	2.94	3.52	4.01	4.39	4.67	4.84	4.90	4.84	4.67	4.39	4.01	3.52	2.94	2.29	1.56	0.79	0.00
DER	SUPERIMPOSED DL DEFL	0.00	0.37	0.72	1.05	1.35	1.62	1.84	2.02	2.15	2.22	2.25	2.22	2.15	2.02	1.84	1.62	1.35	1.05	0.72	0.37	0.00
GIRI	TOTAL DEFLECTION	0.00	1.61	3.17	4.64	5.97	7.14	8.12	8.91	9.48	9.82	9.94	9.82	9.47	8.90	8.11	7.13	5.96	4.63	3.17	1.61	0.00
	VC ORDINATE	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	TOTAL CAMBER	0.00	1.61	3.17	4.64	5.97	7.14	8.12	8.91	9.48	9.82	9.94	9.82	9.47	8.90	8.11	7.13	5.96	4.63	3.17	1.61	0.00
	STEEL DL DEFL	0.00	0.43	0.84	1.23	1.58	1.89	2.15	2.36	2.51	2.60	2.63	2.60	2.51	2.35	2.14	1.88	1.57	1.22	0.83	0.42	0.00
m	CONC SLAB DEFL	0.00	0.79	1.56	2.29	2.94	3.52	4.01	4.39	4.67	4.84	4.90	4.84	4.67	4.39	4.01	3.52	2.94	2.29	1.56	0.79	0.00
DER	SUPERIMPOSED DL DEFL	0.00	0.25	0.50	0.73	0.93	1.12	1.27	1.39	1.48	1.54	1.55	1.54	1.48	1.39	1.27	1.12	0.93	0.73	0.50	0.25	0.00
GIRI	TOTAL DEFLECTION	0.00	1.48	2.90	4.24	5.46	6.53	7.43	8.15	8.67	8.98	9.09	8.98	8.66	8.14	7.42	6.52	5.45	4.23	2.90	1.47	0.00
	VC ORDINATE	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	TOTAL CAMBER	0.00	1.48	2.90	4.24	5.46	6.53	7.43	8.15	8.67	8.98	9.09	8.98	8.66	8.14	7.42	6.52	5.45	4.23	2.90	1.47	0.00
	STEEL DL DEFL	0.00	0.40	0.78	1.14	1.47	1.76	2.00	2.19	2.33	2.42	2.44	2.41	2.33	2.18	1.99	1.75	1.46	1.13	0.78	0.39	0.00
4	CONC SLAB DEFL	0.00	0.79	1.56	2.29	2.94	3.52	4.01	4.39	4.67	4.84	4.90	4.84	4.67	4.39	4.01	3.52	2.94	2.29	1.56	0.79	0.00
GIRDER	SUPERIMPOSED DL DEFL	0.00	0.29	0.57	0.84	1.08	1.29	1.47	1.61	1.71	1.77	1.79	1.77	1.71	1.61	1.47	1.29	1.08	0.84	0.57	0.29	0.00
SIRI	TOTAL DEFLECTION	0.00	1.48	2.92	4.26	5.49	6.57	7.47	8.19	8.72	9.03	9.14	9.03	8.71	8.19	7.46	6.56	5.48	4.26	2.91	1.48	0.00
	VC ORDINATE	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	TOTAL CAMBER	0.00	1.48	2.92	4.26	5.49	6.57	7.47	8.19	8.72	9.03	9.14	9.03	8.71	8.19	7.46	6.56	5.48	4.26	2.91	1.48	0.00
	STEEL DL DEFL	0.00	0.39	0.77	1.12	1.44	1.72	1.96	2.15	2.29	2.37	2.40	2.37	2.28	2.14	1.95	1.71	1.43	1.11	0.76	0.39	0.00
5	CONC SLAB DEFL	0.00	0.67	1.32	1.92	2.48	2.96	3.37	3.70	3.93	4.08	4.12	4.08	3.93	3.70	3.37	2.96	2.48	1.92	1.32	0.67	0.00
)ER	SUPERIMPOSED DL DEFL	0.00	0.23	0.45	0.65	0.84	1.00	1.14	1.25	1.33	1.38	1.39	1.38	1.33	1.25	1.14	1.00	0.84	0.65	0.45	0.23	0.00
GIRE	TOTAL DEFLECTION	0.00	1.28	2.53	3.69	4.75	5.68	6.47	7.09	7.55	7.82	7.91	7.82	7.54	7.09	6.46	5.67	4.74	3.68	2.52	1.28	0.00
	VC ORDINATE	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	TOTAL CAMBER	0.00	1.28	2.53	3.69	4.75	5.68	6.47	7.09	7.55	7.82	7.91	7.82	7.54	7.09	6.46	5.67	4.74	3.68	2.52	1.28	0.00

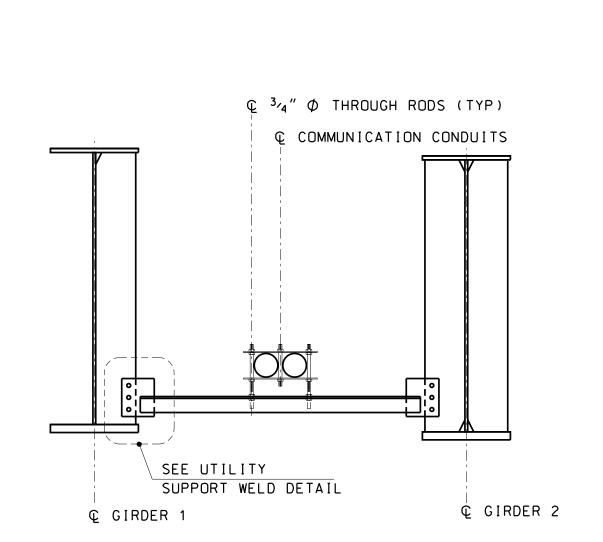


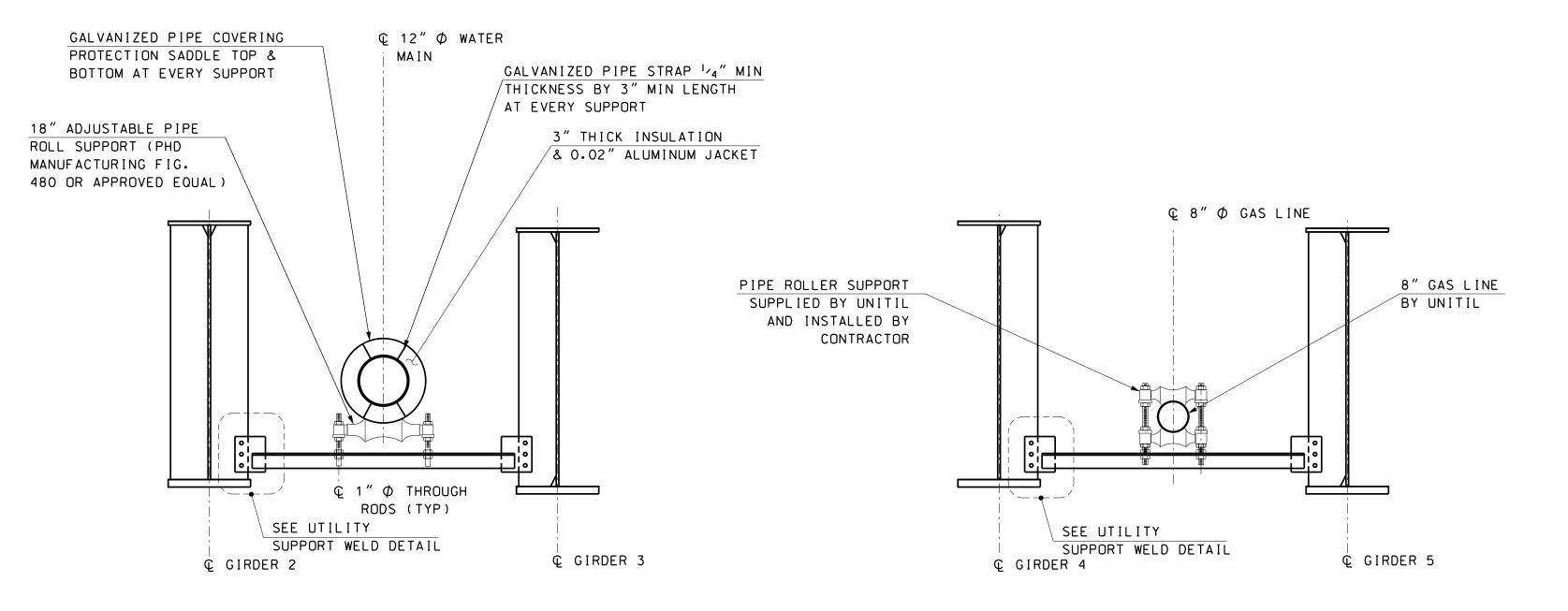
<u>CAMBER DIAGRAM</u> NOT TO SCALE

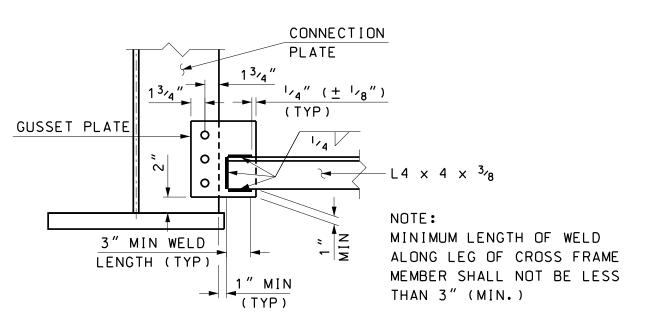
	CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES											
	LOCA	ATION	WHITTIER STREET OV	ER COCI	HECO RIVER	BRIDGE NO	D. 111\1	32 S	STATE PRO	ECT 154	402	
			GIRDER	R DE	TAILS (S	SHEET	1 OF	(2)			BRIDGE SHEET	
		REVI	SIONS AFTER PROPOSAL			BY	DATE		BY	DATE	21 of 35	
					DESIGNED	TWP	11/15	CHECKED	KSW	11/15	21 33	
•					DRAWN	DWM	11/15	CHECKED	KSW	11/15	FILE NUMBER	
		1						1			I	

	<u> </u>		LOCATION	WHITTIER STREET OV	ER COCH	IECO RIVER	BRIDGE N	O. 111\1	32 STA	TE PROJ	IECT 154	102
				GIRDER	DE	TAILS (S	HEET	1 OF	2)			BRIDGE SHEET
			RE	EVISIONS AFTER PROPOSAL			BY	DATE		BY	DATE	21 of 35
THE LO	ouis Berger	Group, INC.				DESIGNED	TWF	11/15	CHECKED	KSW	11/15	21 % 33
Ma Ma	anchester, New	Hampshire				DRAWN	DWN	1 11/15	CHECKED	KSW	11/15	FILE NUMBER
	(603) 644 520					QUANTITIES	TWF	11/15	CHECKED	HNH	11/15	
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE				ISSUE DATE	_		L PROJECT NO.		ET NO.	TOTAL SHEETS
d0174059	15402GirderDetls01	AS NOTED				REV. DATE		X-A	002(794)		26	58









COMMUNICATION CONDUIT DETAILS

SCALE: 1/2" = 1'-0"

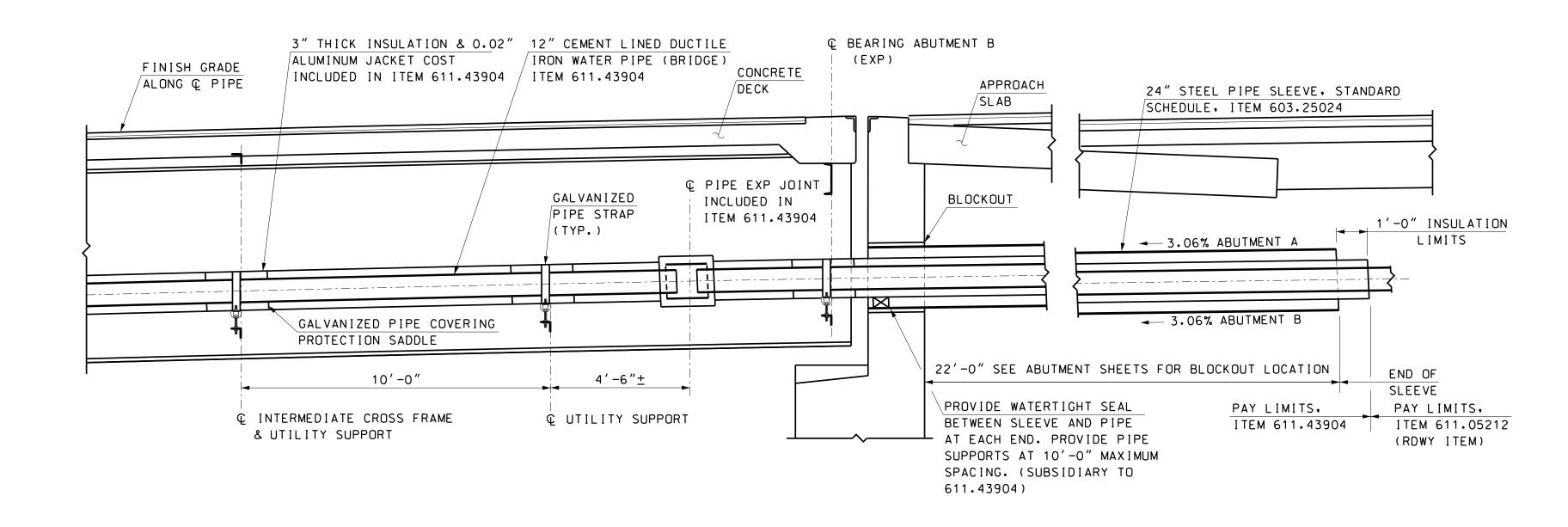
WATER MAIN DETAILS

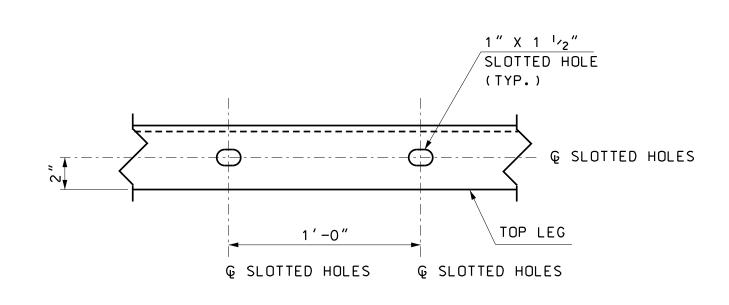
SCALE: 1/2" = 1'-0"

GAS LINE DETAILS

SCALE: 1/2" = 1'-0"

UTILITY SUPPORT
WELD DETAIL
SCALE: 1" = 1'-0"





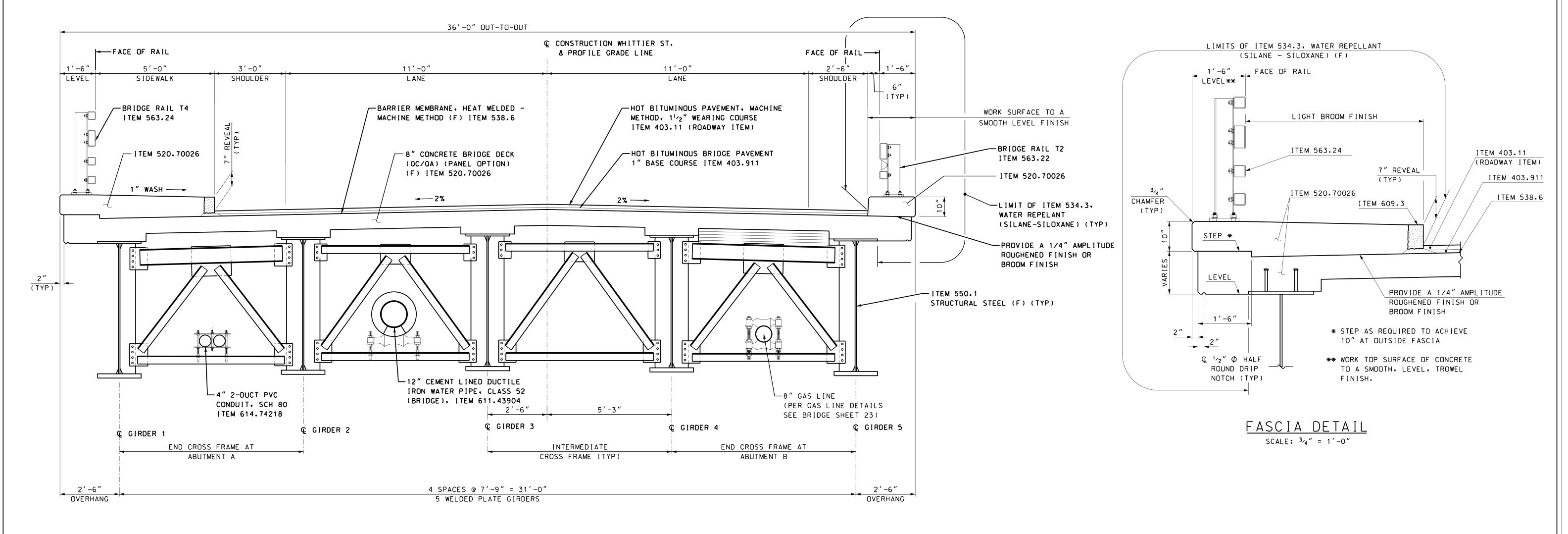
SLOTTED HOLE DETAIL

NOT TO SCALE

WATER MAIN DETAIL

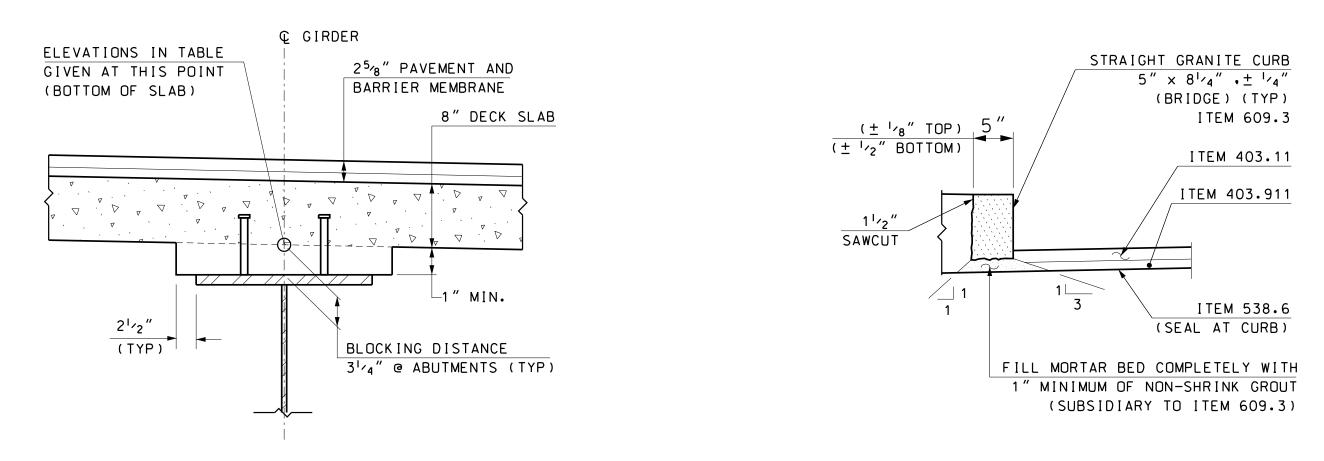
SCALE: 3/8" = 1'-0"

					CIT	YO	F DOVER	R, NE'	W HAMPSHIR	E						
					I	DEPAR	RTMENT OF	COMM	UNITY SERVICES							
				LOC	CATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402											
			SUPERSTRUCTURE DETAILS BRIDGE SHEET													
Г		'- D	0		REVISIONS AFTER PROPOSAL			BY		BY DATE	23 of 35					
- [/	THE L	ouis Berger	Group, INC.				DESIGNED	TW	P 11/15 CHECKED	KSW 11/15	20 00					
Į		anchester, New I	<u> </u>				DRAWN	DW	M 11/15 CHECKED	KSW 11/15	FILE NUMBER					
		(603) 644 520	00				QUANTITIES	TW	P 11/15 CHECKED	HNH 11/15						
Г	SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE				ISSUE DATE	_	FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS					
	d0174059	15402SuperDetIs01	AS NOTED				REV DATE		X-A002(794)	28	58					



TYPICAL DECK SECTION

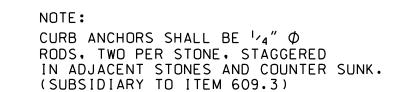
SCALE: 1/2'' = 1'-0''

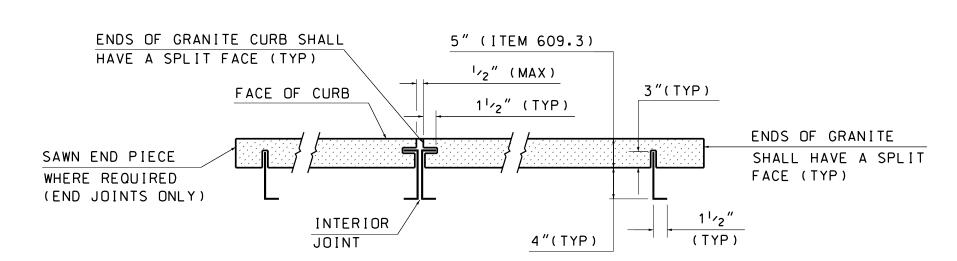


HAUNCH DETAIL NOT TO SCALE

CURB DETAIL NOT TO SCALE

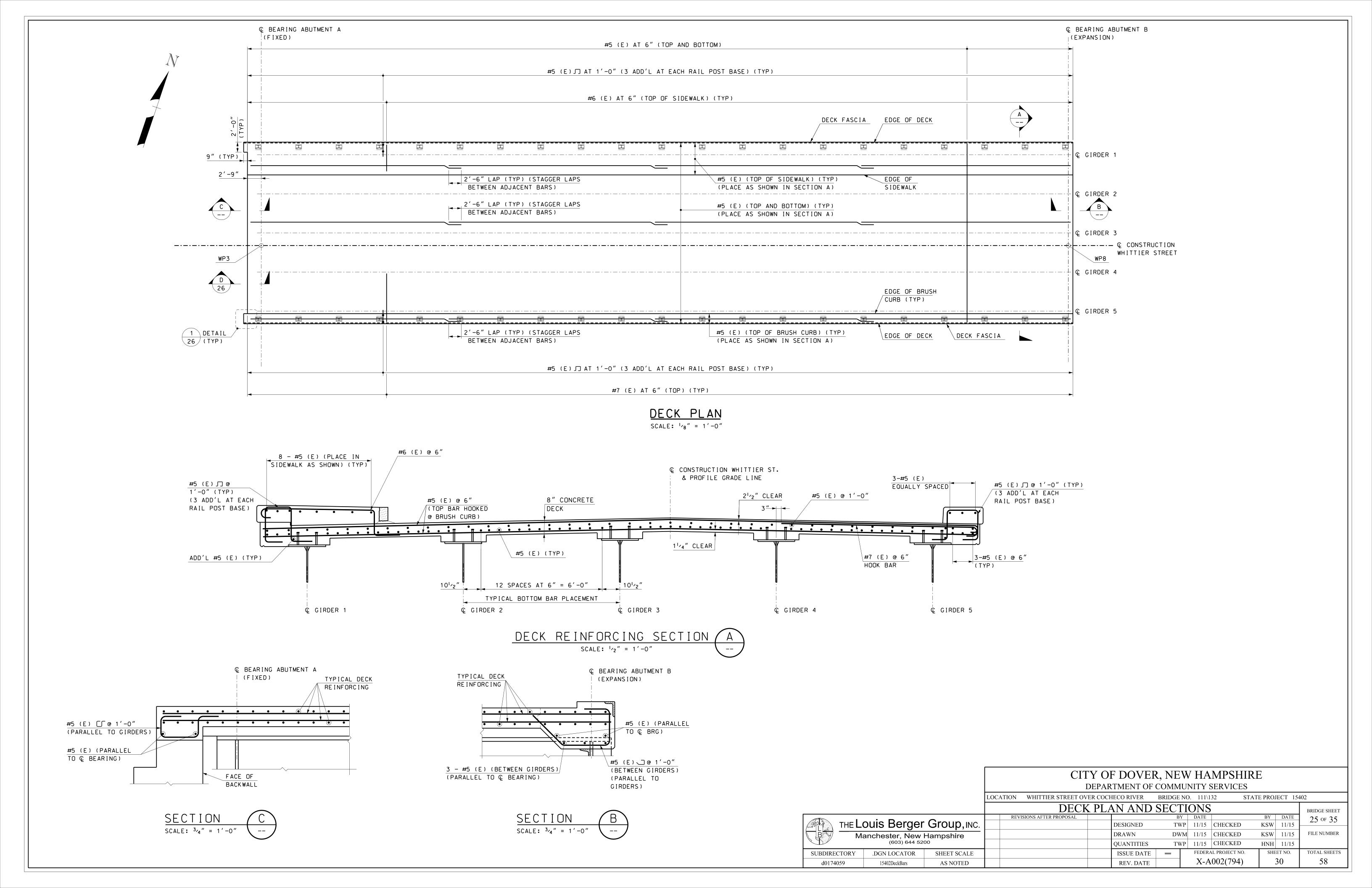
	ELEVATIONS AT BOTTOM OF CONCRETE DECK SLAB (FT)																				
LOCATION	CL BRG ABUT A	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	CL BRG ABUT B
CL GIRDER 1	59.45	59.79	60.12	60.45	60.78	61.09	61.39	61.68	61.96	62.23	62.48	62.72	62.94	63.15	63.35	63.54	63.71	63.88	64.04	64.19	64.34
CL GIRDER 2	59.60	59.94	60.28	60.61	60.94	61.25	61.56	61.85	62.13	62.39	62.65	62.88	63.11	63.32	63.52	63.70	63.88	64.04	64.20	64.35	64.50
CL GIRDER 3	59.76	60.09	60.42	60.74	61.06	61.37	61.66	61.95	62.23	62.49	62.74	62.98	63.21	63.42	63.62	63.81	64.00	64.17	64.33	64.49	64.65
CL GIRDER 4	59.70	60.04	60.37	60.70	61.02	61.33	61.63	61.92	62.19	62.46	62.71	62.95	63.17	63.38	63.58	63.77	63.95	64.12	64.29	64.44	64.60
CL GIRDER 5	59.55	59.87	60.18	60.50	60.80	61.10	61.39	61.67	61.94	62.20	62.45	62.69	62.92	63.14	63.35	63.55	63.74	63.92	64.10	64.27	64.44

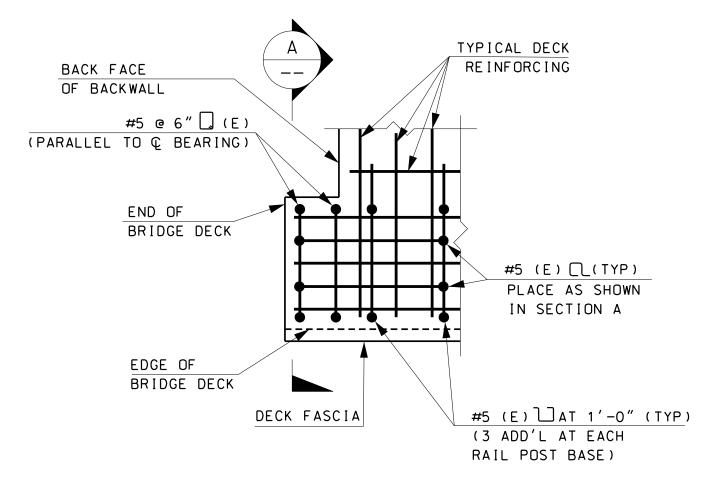




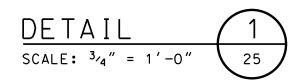
CURB ANCHOR DETAIL NOT TO SCALE

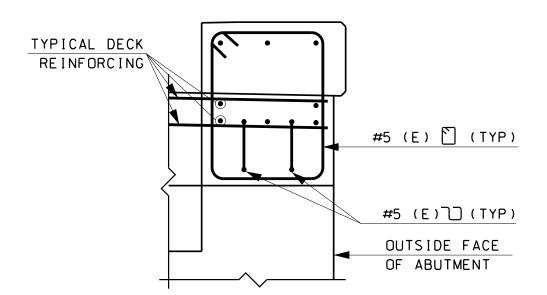
				CITY OF DOVER, NEW HAMPSHIRE											
				DEPARTMENT OF COMMUNITY SERVICES											
			LOCA	CATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402											
				TYPICAL SECTION AND DECK SLAB DETAILS BRIDGE SHEET											
				REVISIONS AFTER PROPOSAL			BY	Z DATE	BY DATE	24 of 35					
THE LO	ouis Berger	Group, INC.				DESIGNED	TW	P 11/15 CHECKED	KSW 11/15	24 % 33					
Ma Ma	anchester, New	Hampshire				DRAWN	DW	M 11/15 CHECKED	KSW 11/15	FILE NUMBER					
	(603) 644 52	00				QUANTITIES	TW	P 11/15 CHECKED	HNH 11/15						
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE				ISSUE DATE	_ [FEDERAL PROJECT NO.	SHEET NO.	TOTAL SHEETS					
d0174059	15402DeckSect	AS NOTED				REV. DATE		X-A002(794)	29	58					





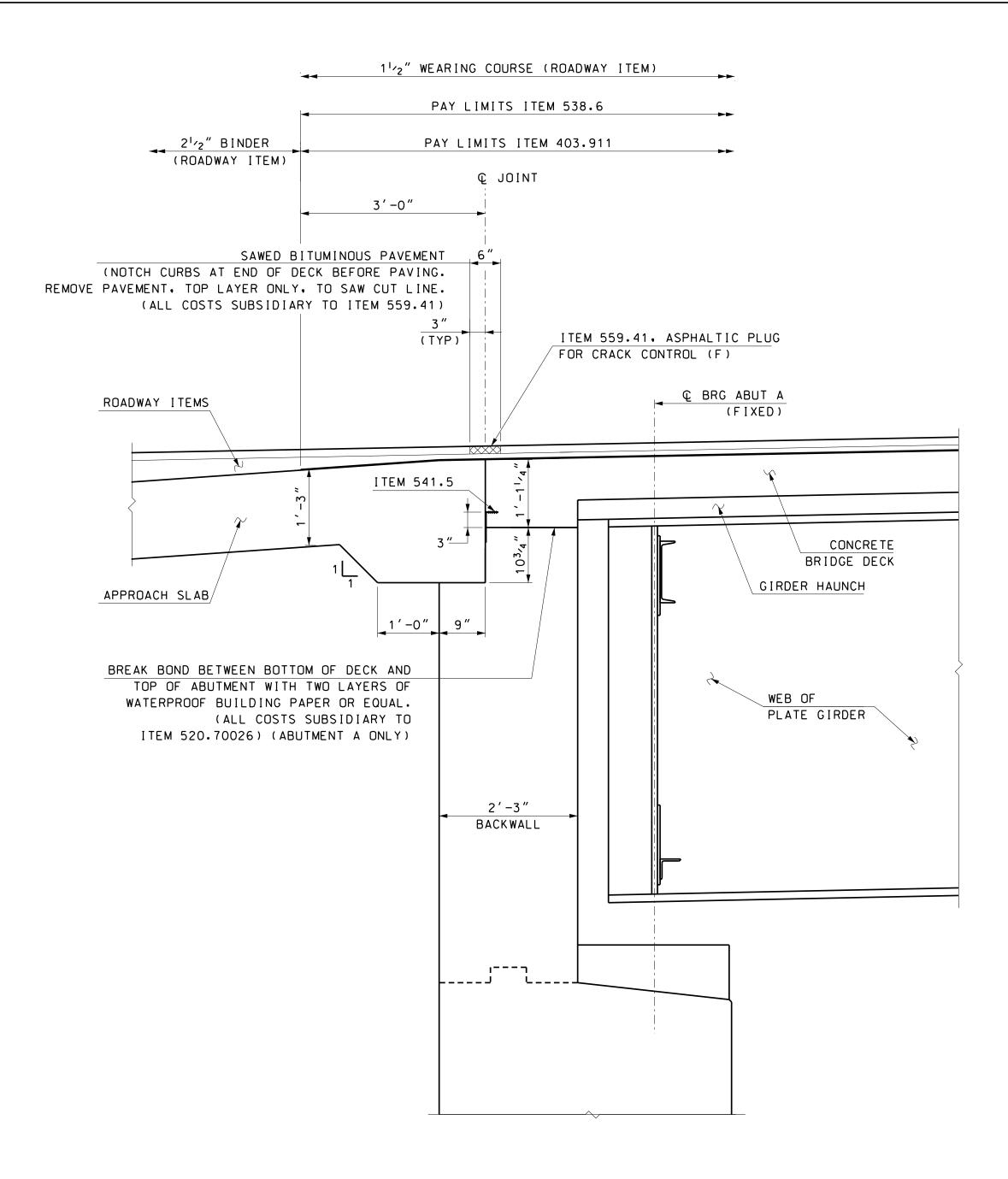
END OF DECK PLAN

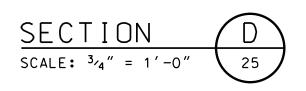




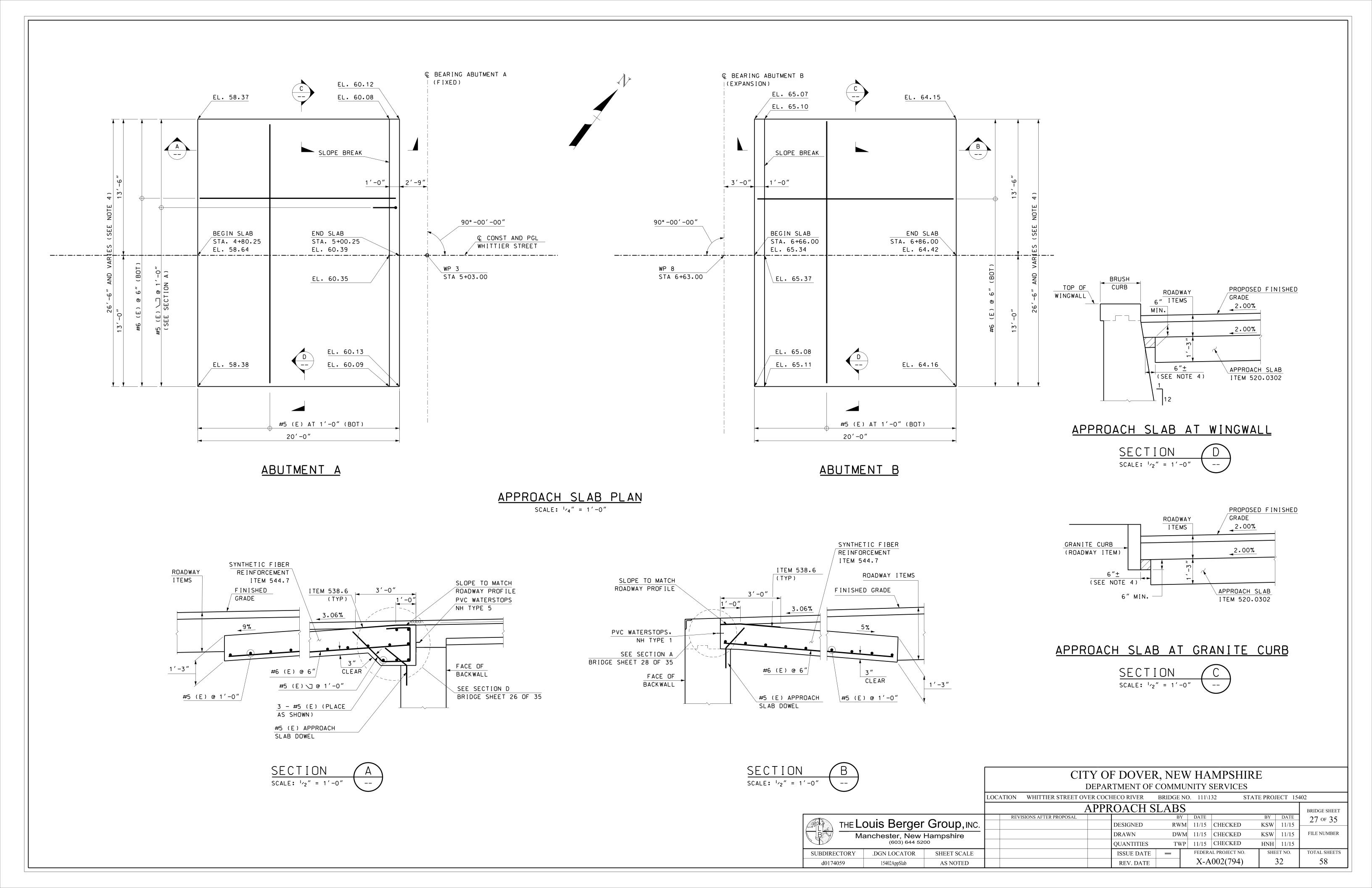
SECTION A

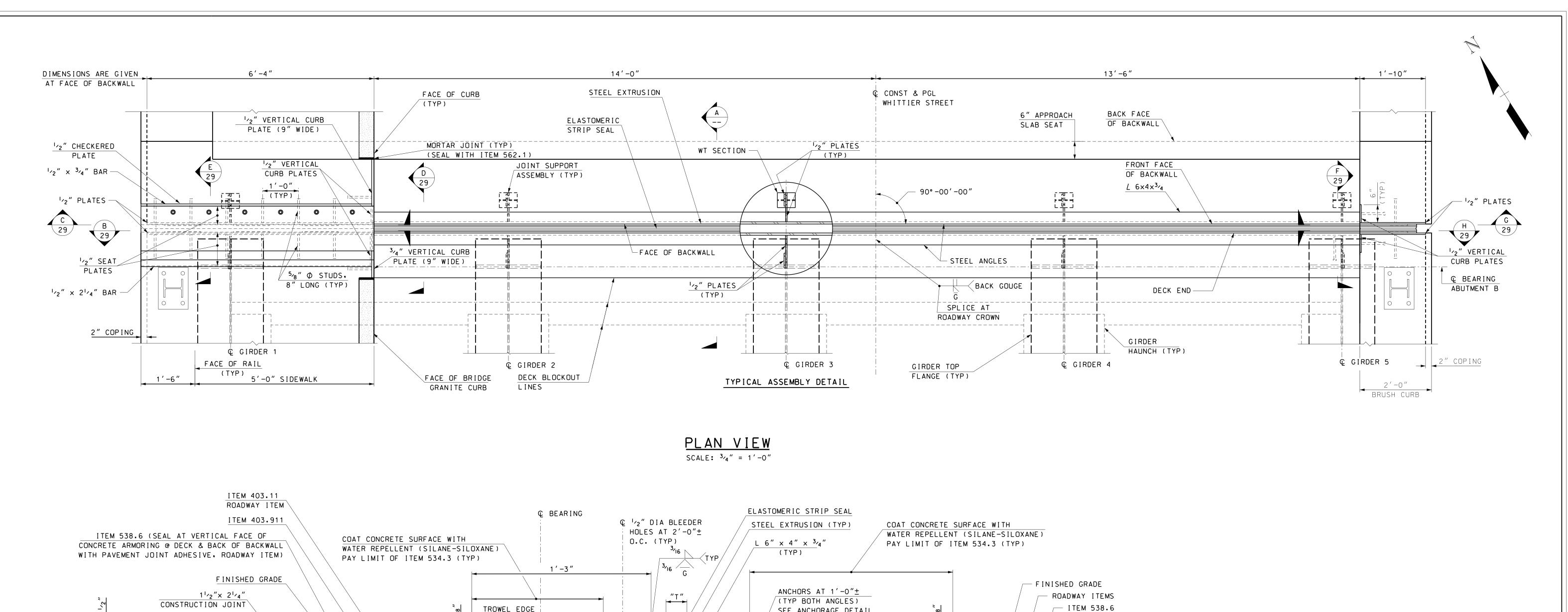
SCALE: 3/4" = 1'-0" --

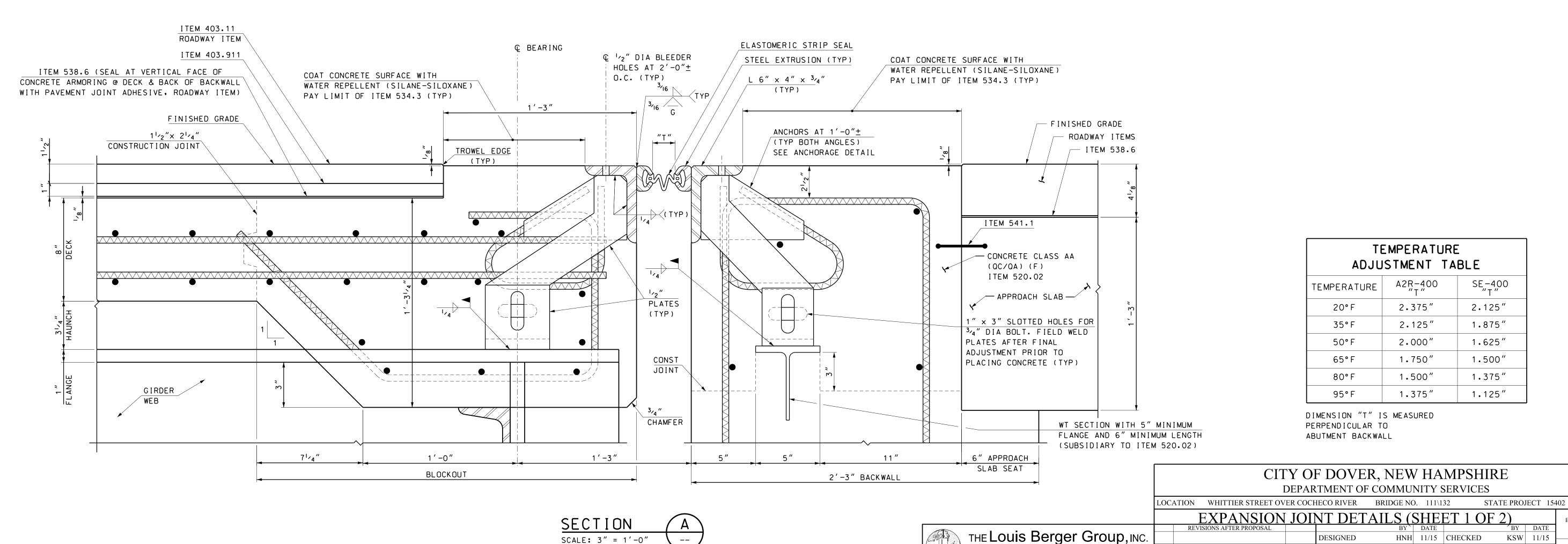




				CIT	Y O	F DOVER	R, NEV	V HA	MPSHIR	RE			
				I	DEPAR	TMENT OF	COMMU	INITY S	SERVICES				
	LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402												
					DEC	K DETA	ILS 1					BRIDGE SHEET	
				REVISIONS AFTER PROPOSAL			BY	DATE		BY	DATE	26 of 35	
THE LO	ouis Berger	Group, INC.				DESIGNED	TWP	11/15	CHECKED	KSW	11/15	20 - 33	
	anchester, New	· · · · · · · · · · · · · · · · · · ·				DRAWN	DWM	1 11/15	CHECKED	KSW	11/15	FILE NUMBER	
	(603) 644 52	00				QUANTITIES	TWP	11/15	CHECKED	HNH	11/15		
SUBDIRECTORY	.DGN LOCATOR	SHEET SCALE				ISSUE DATE	= [FEDER/	AL PROJECT NO.	SHE	ET NO.	TOTAL SHEETS	
d0174059	15402DeckDetls01	AS NOTED				REV DATE		X-A	.002(794)		31	58	







Manchester, New Hampshire

SHEET SCALE

AS NOTED

.DGN LOCATOR

SUBDIRECTORY

d0174059

BRIDGE SHEET

 $28 \, \mathrm{OF} \, 35$

FILE NUMBER

TOTAL SHEETS

58

KSW 11/15

HNH 11/15

SHEET NO.

DRAWN

QUANTITIES

ISSUE DATE

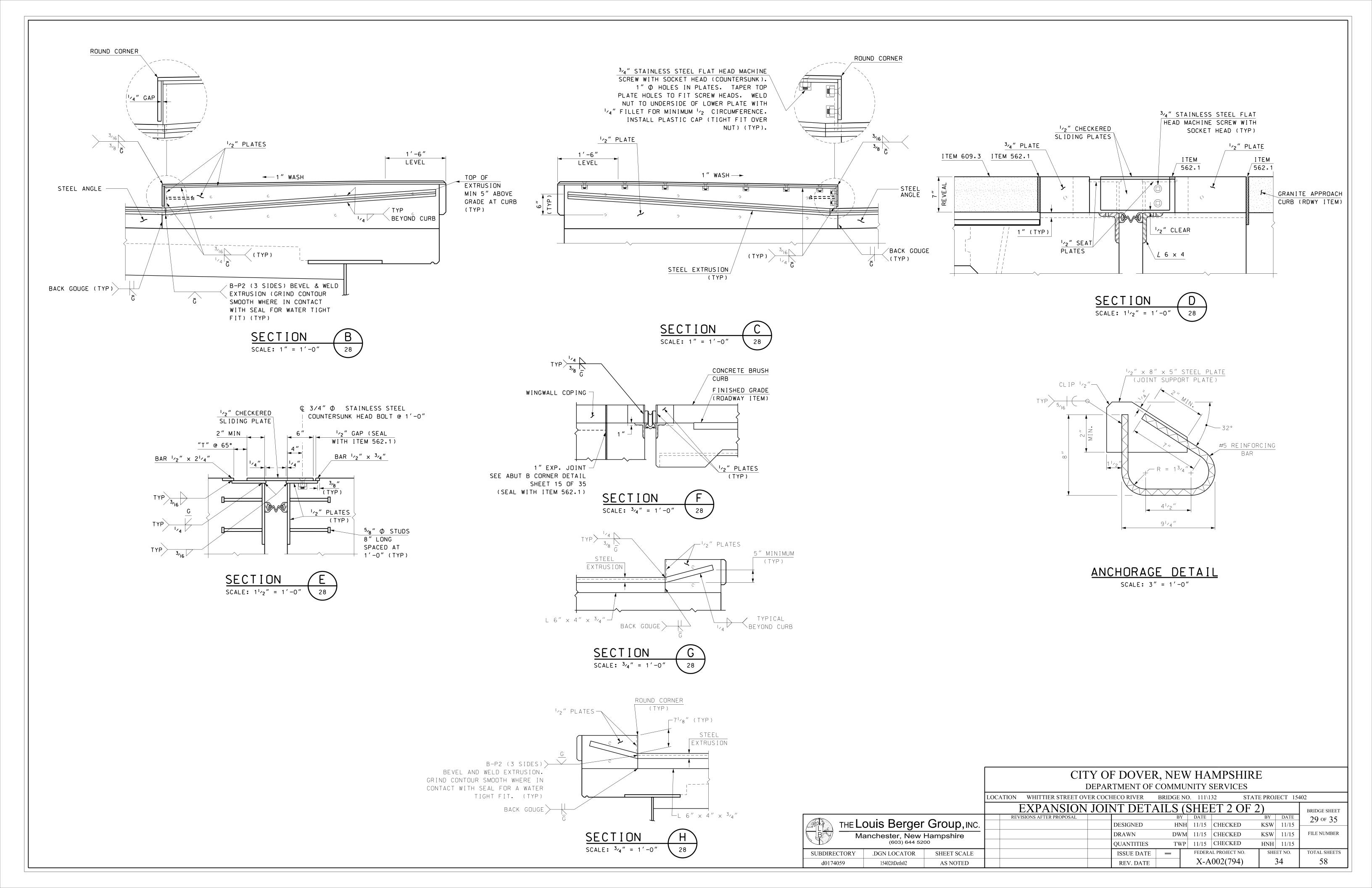
REV. DATE

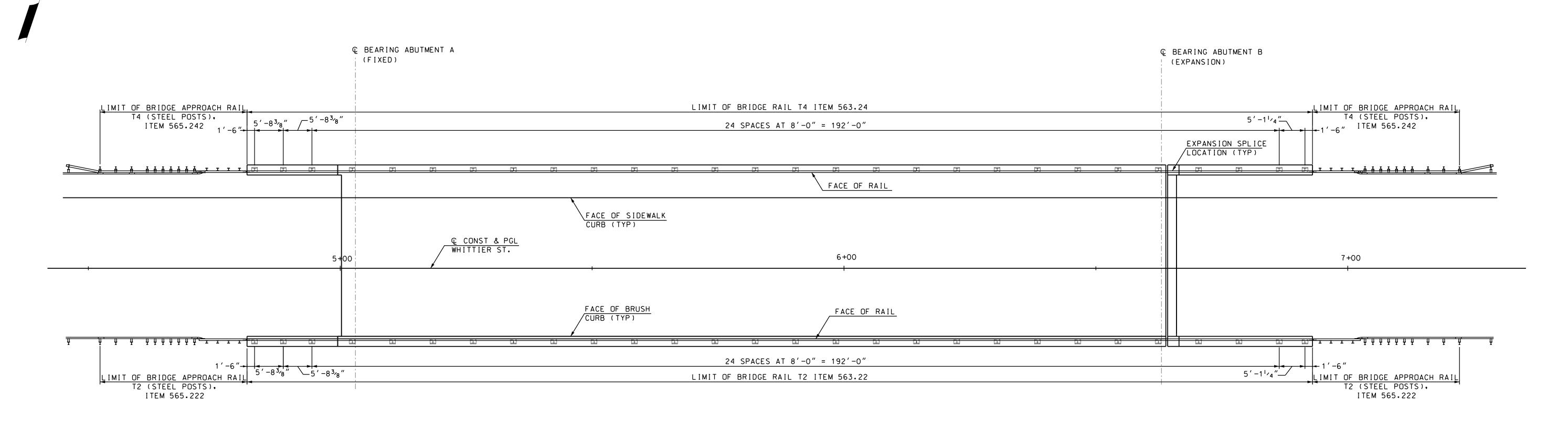
DWM 11/15 CHECKED

TWP 11/15 CHECKED

FEDERAL PROJECT NO.

X-A002(794)



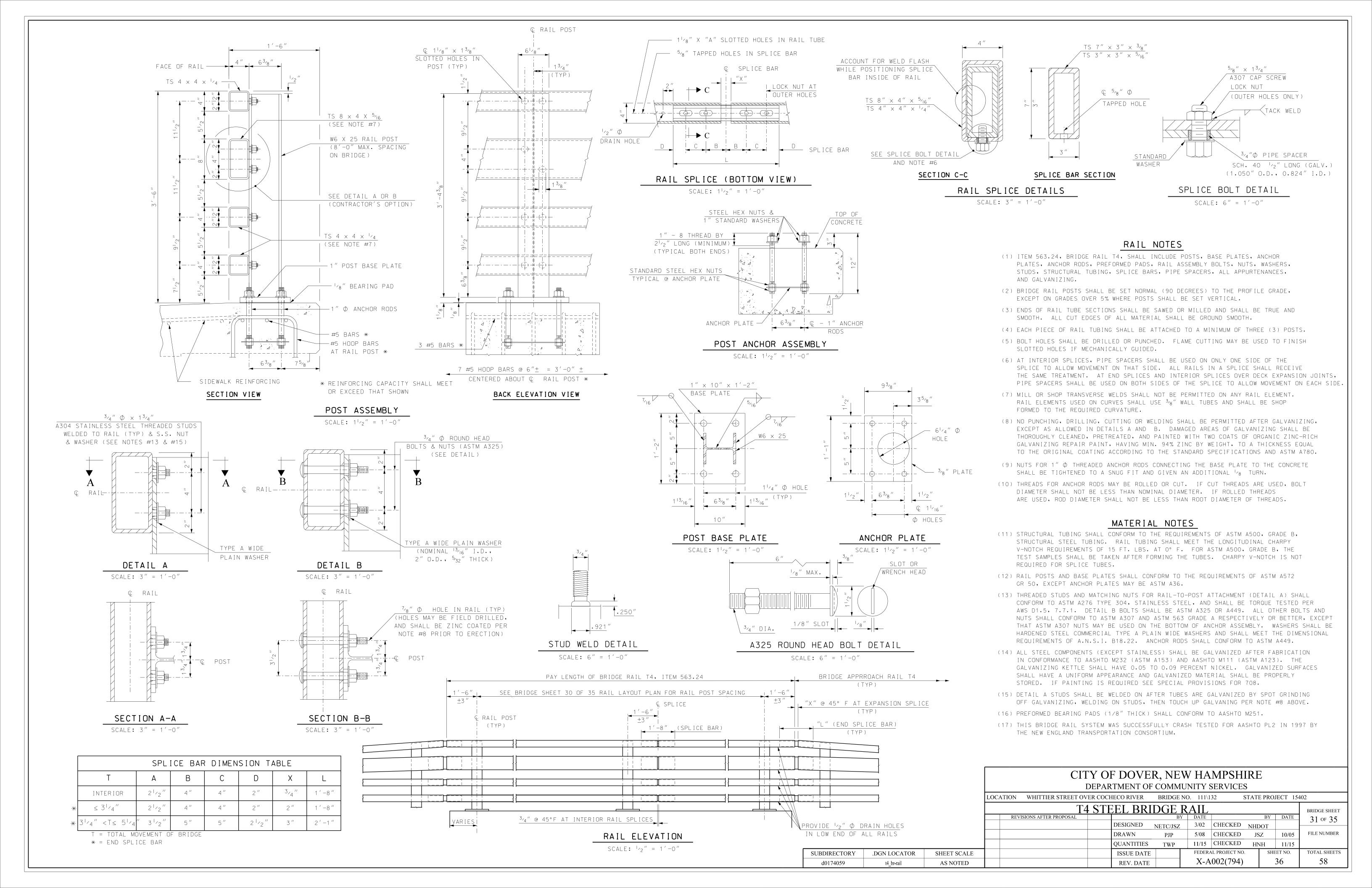


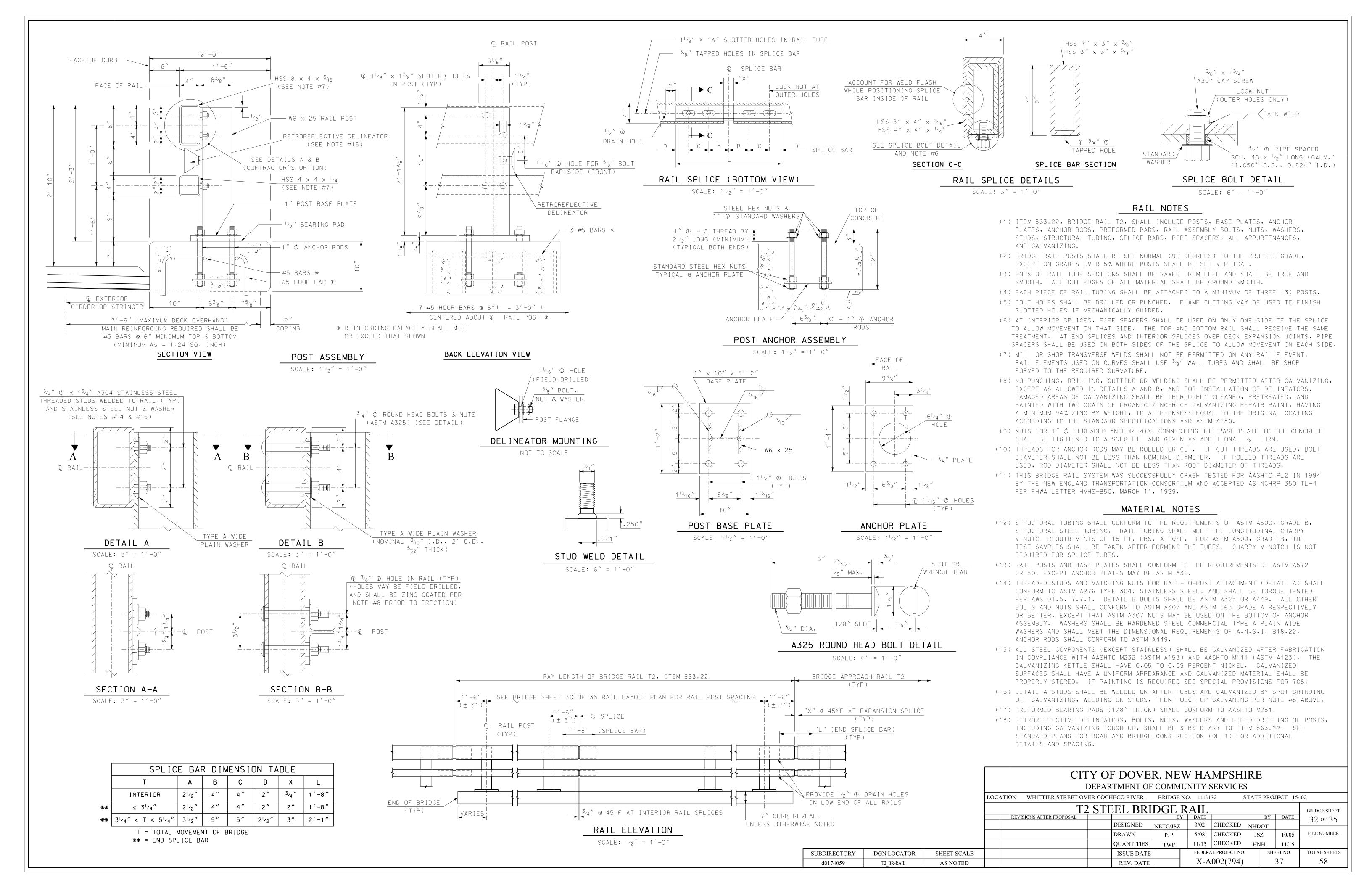
BRIDGE RAIL LAYOUT PLAN

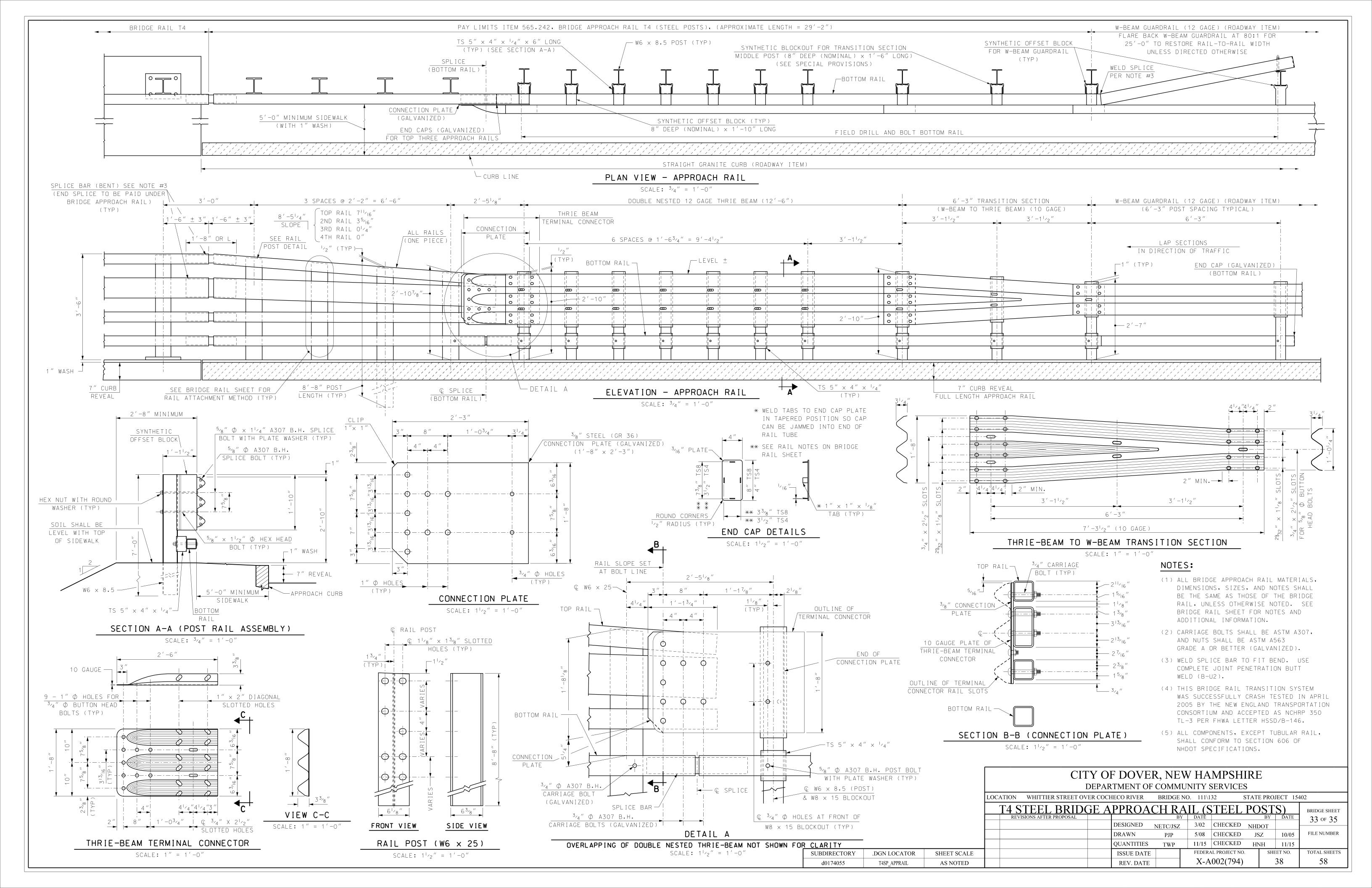
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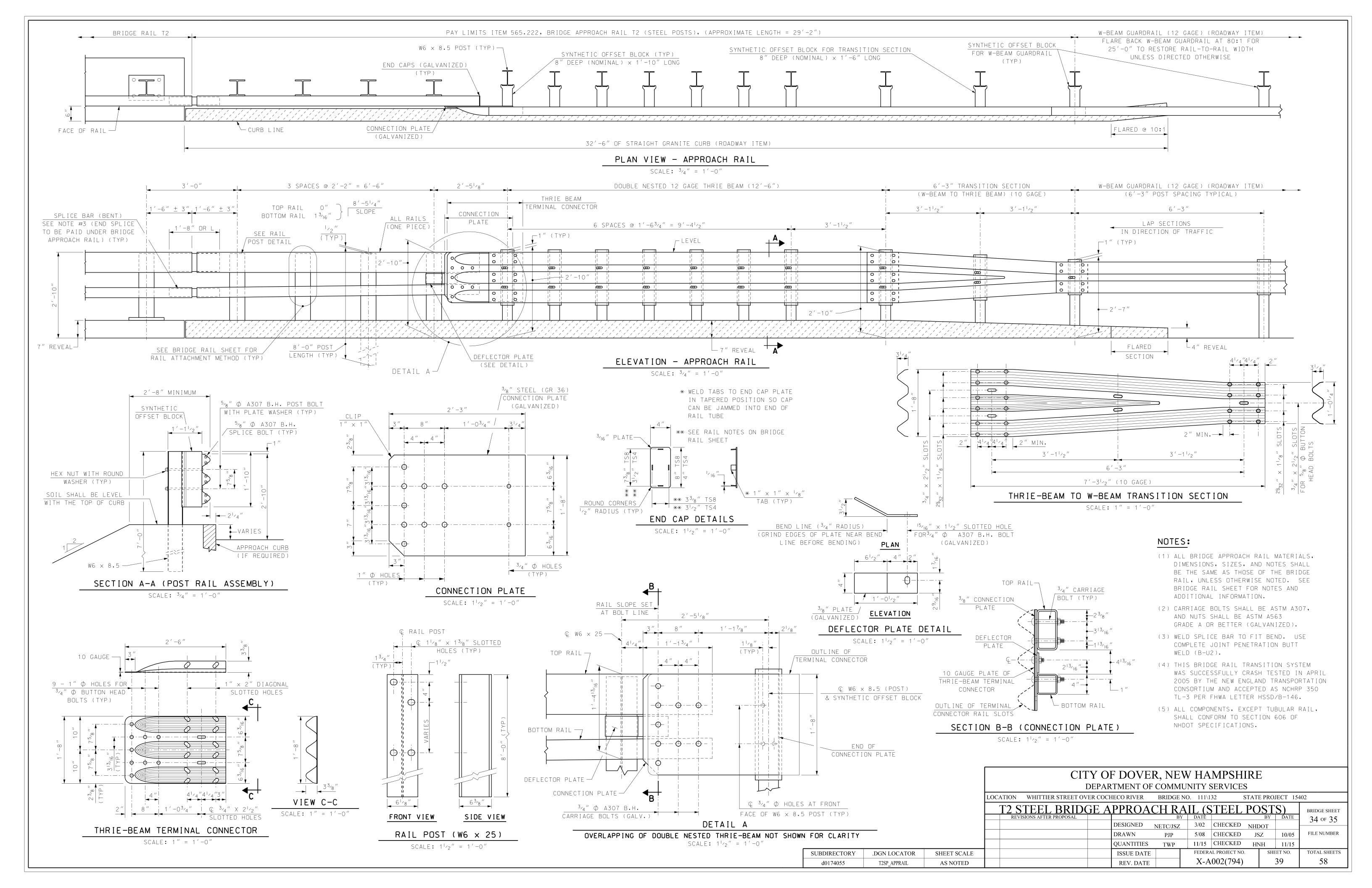
NOTE:
RAIL EXPANSION SPLICES ARE REQUIRED OVER THE BRIDGE DECK
EXPANSION JOINT.

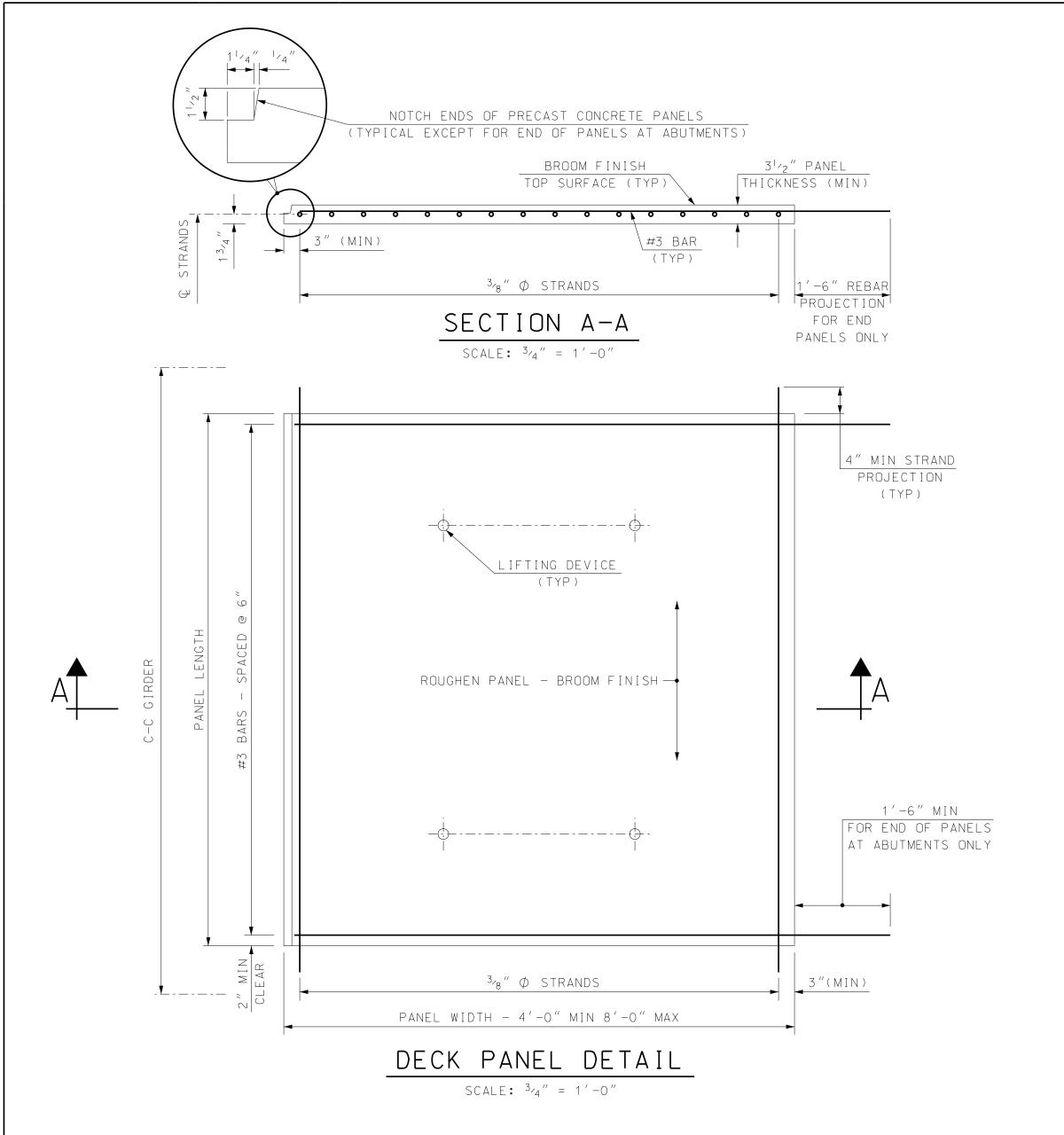
	CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES											
	LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402											
RAIL LAYOUT PLAN BRIDGE SHE												
TUE LOUIS BORGOT GROUP ING	REVISIONS AFTER PROPOSAL	BY DATE BY DATE	30 of 35									
THE Louis Berger Group, INC.	DESIGNI	TED TWP 11/15 CHECKED KSW 11/15										
Manchester, New Hampshire	DRAWN	N DWM 11/15 CHECKED KSW 11/15	FILE NUMBER									
(603) 644 5200	QUANTI	TTIES TWP 11/15 CHECKED HNH 11/15										
SUBDIRECTORY .DGN LOCATOR SHEET SCALE	ISSUE I		TOTAL SHEETS									
d0174055 15402RailLayout AS NOTED	REV. I	DATE X-A002(794) 35	58									

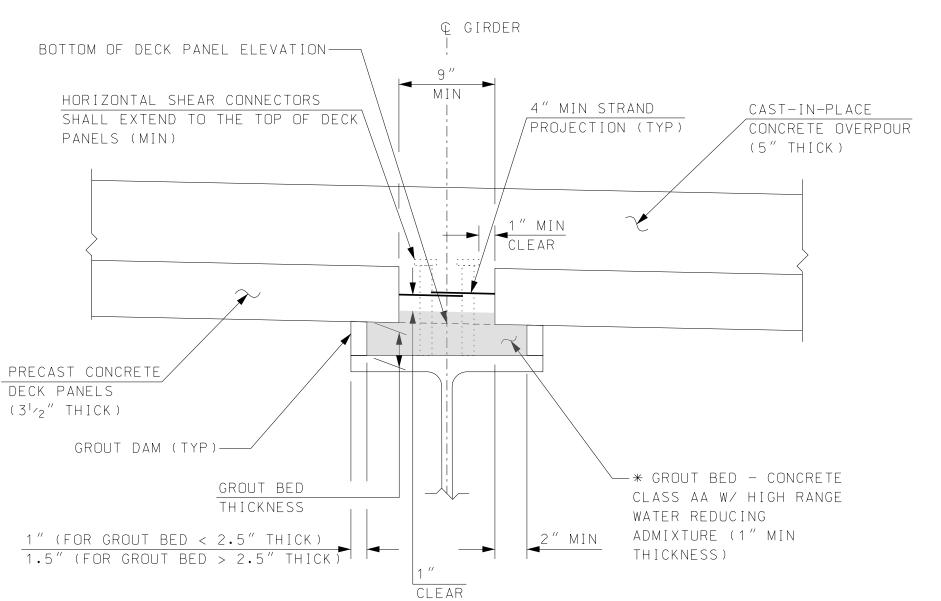












* ENSURE GROUT FLOWS UNDER PANEL FOR COMPLETE BEARING

STEEL GIRDER HAUNCH DETAIL

SCALE: 2'' = 1' - 0''

#7(E) @ 6"

(HOOK BAR)

#5(E) @ 6"

(HOOK BAR)

#5(E) @ 6"

(HOOK BAR)

DECK OVERHANG DETAIL

ℚ GIRDER

SCALE: 1" = 1'-0"

PRESTRESSED CONCRETE DECK PANEL NOTES

- (1) CONCRETE STRENGTH: f'c = 6,000 PSI MINIMUM AT 28 DAYS SEE TABLE A & B f'ci = 4,000 PSI MINIMUM DECK PANEL DESIGN
- (2) PRESTRESSING STRANDS SHALL BE 3/8 in. DIAMETER, GRADE 270 SEVEN WIRE LOW-RELAXATION TYPE, CONFORMING TO THE REQUIREMENTS OF ASTM A416. ALL STRANDS SHALL BE PULLED TO HAVE A NET TENSION OF 17.2 KIPS PER STRAND AFTER ALLOWING FOR CHUCK SLIPPAGE.
- (3) THE TOP SURFACE OF THE DECK PANELS SHALL BE BROOMED TO A SURFACE ROUGHNESS OF 0.06 in. BROOM THE SURFACE PARALLEL TO THE STRAND.
- (4) IF HIGH DENSITY EXPANDED POLYSTYRENE FOAM IS USED AS A TEMPORARY SUPPORT, IT SHALL BE CUT IN THE FIELD TO THE REQUIRED HEIGHT AND AFFIXED TO THE GIRDERS WITH AN APPROVED HIGH STRENGTH ADHESIVE.
- (5) PANEL LIFTING LOCATIONS SHOWN ARE ADVISORY ONLY. ACTUAL LIFTING LOCATIONS SHALL BE DETERMINED BY THE FABRICATOR AND INDICATED ON THE SHOP DRAWINGS.
- (6) CORROSION INHIBITOR (CALCIUM NITRITE) ADMIXTURE SHALL BE USED.
- (7) SEE STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS FOR SECTIONS 520 AND 528 FOR ADDITIONAL INFORMATION.
- (8) IF LEVELING SCREWS ARE USED, THEY SHALL BE COMPLETELY REMOVED AFTER THE GROUTING OPERATIONS AND PRIOR TO DECK PLACEMENT, HOLES LEFT BY LEVELING SCREWS SHALL BE FILLED WITH AN APPROVED GROUT PRIOR TO DECK PLACEMENT.
- (9) TEMPORARY BRACING BETWEEN ENDS OF PANELS SHALL BE SUPPLIED AS REQUIRED TO PREVENT PANEL MOVEMENT TRANSVERSE TO THE GIRDERS.
- (10) THE FOLLOWING DECK PANEL DESIGN INFORMATION SHALL BE USED FOR THIS PROJECT:

 C-C GIRDER SPACING = 7'-9"

 PANEL LENGTH = 7'-0"

PANEL THICKNESS = $3\frac{1}{2}$ "

f'c = 6000 PSI

CONCRETE STRENGTHS f'ci = 4000 PSI

STRAND SPACING = 8"

(11) REINFORCING IN PANELS SHALL BE BLACK BAR EXCEPT FOR END PANELS AT ABUTMENTS WHICH SHALL HAVE EPOXY COATED REBAR AND FOLLOW LAYOUT OF TOP MAT OF STEEL SHOWN ON THE DECK REINFORCING SHEET.

.DGN LOCATOR

15402DeckPanels

SUBDIRECTORY

d0174059

SHEET SCALE

AS NOTED

TABLE A - DECK PANEL DESIGN (STEEL-GIRDER)									
C-C GIRDER SPACING	PANEL LENGTH	PANEL THICKNESS	f'ci (PSI)	f'c (PSI)	STRAND SPACING				
5′-6″	5′-0″	31/2"	4000	6000	8 "				
6′-0″	5′-6″	31/2"	4000	6000	8 "				
6′-6″	6'-0"	31/2"	4000	6000	8 "				
7′-0″	6'-6"	31/2"	4000	6000	8 "				
7′-6″	7'-0"	31/2"	4000	6000	8 "				
8'-0"	7′-6″	31/2"	4000	6000	8 "				
8'-6"	8'-0"	31/2"	4000	6000	6 "				
9'-0"	8'-6"	31/2"	4000	6000	6 "				
9'-6"	9'-0"	31/2"	4000	6000	5 "				
10'-0"	9'-6"	31/2"	5000	6000	41/2"				

TABLE C - GIRDER DEFLECTIONS DUE TO DECK PANEL DEAD LOAD (INCHES)											
	ABUT A	0.1L	0.2L	0.3L	0.4L	0.5L	0.6L	0.7L	0.8L	0.9L	ABUT B
EXTERIOR GIRDER (1 & 5)	0.00	-1.36	-2.57	-3.49	-4.07	-4.27	-4.07	-3.49	-2.57	-1.36	0.00
INTERIOR GIRDER (2 -4)	0.00	-1.63	-3.06	-4.16	-4.86	-5.10	-4.86	-4.16	-3.06	-1.63	0.00

DESIGN CRITERIA:

- LIVE LOAD = HL-93

- ALLOWABLE TENSION IN CONCRETE = 0.19 \(\text{f'c} \)

- MAXIMUM INITIAL COMPRESSION = 0.750 ksi (W/f'ci = 4 ksi)

- C-I-P DECK THICKNESS

- PAVEMENT THICKNESS = $2^{1}/2^{"}$

- STEEL FLANGE WIDTH = 22''- GROUT DAM WIDTH = $1\frac{1}{2}''$

- GROUT BED THICKNESS < 23/4"

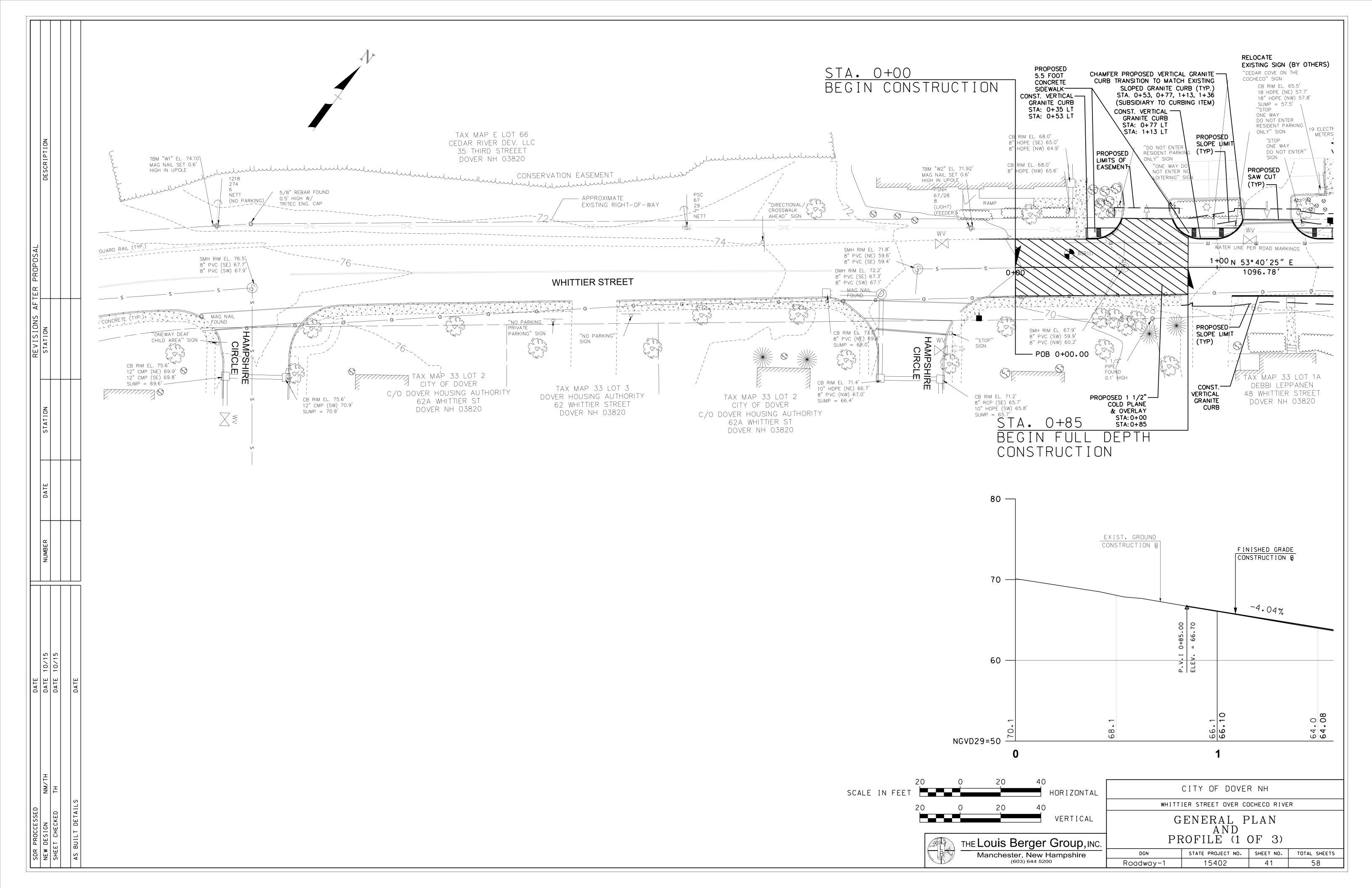
DECK SLAB ELEVATION NOTES

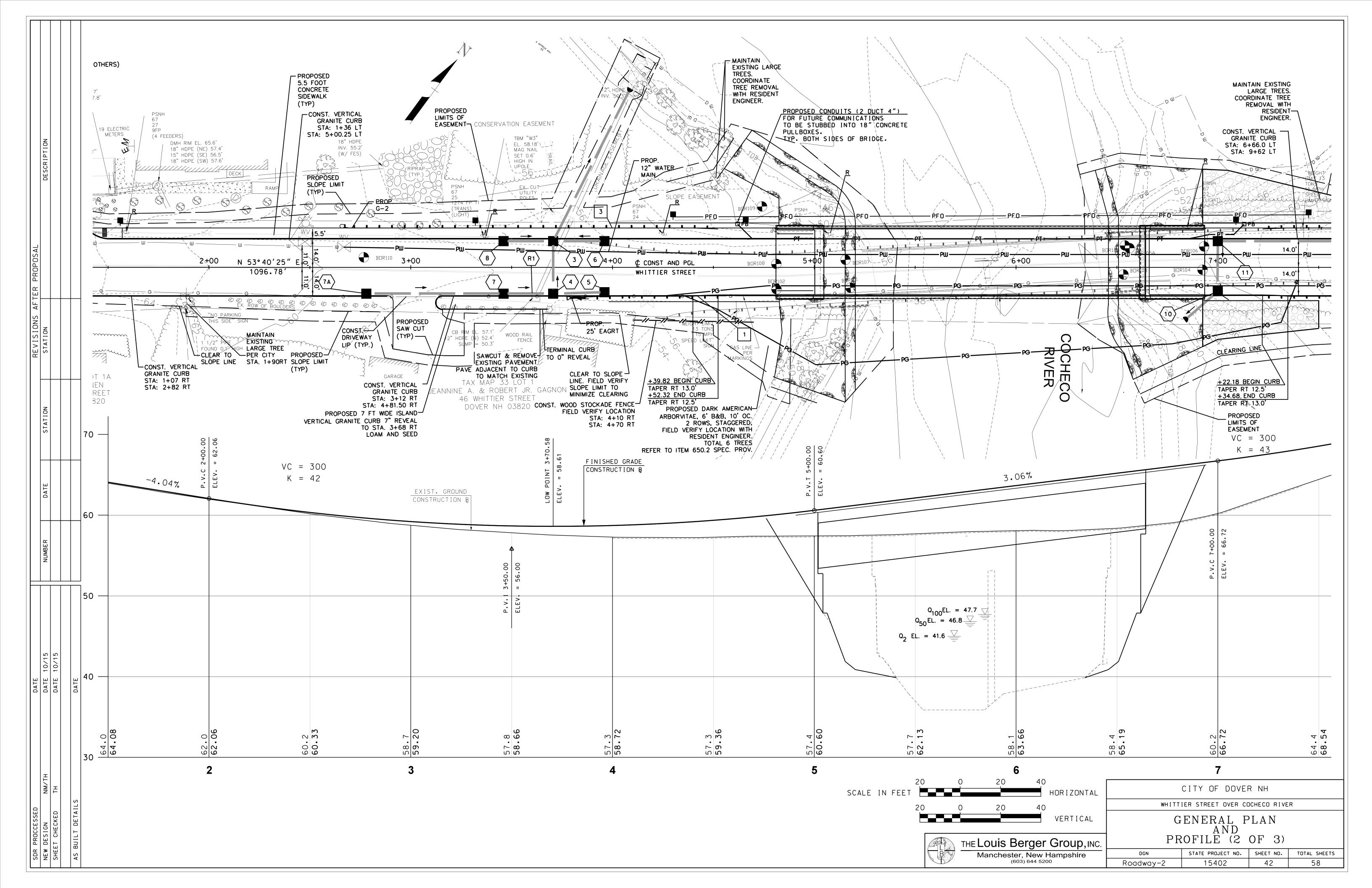
- 1) AFTER THE GIRDERS ARE ERECTED AND BEFORE PRECAST DECK PANELS ARE SET, ELEVATIONS ON THE TOP FLANGE OF THE GIRDERS ARE TO BE OBTAINED AT THE POINTS INDICATED IN "BOTTOM OF SLAB ELEVATION TABLE" DETAILED IN THE PLANS AND GIRDER HAUNCH DETAILS ON THIS SHEET.
- 2) THE BOTTOM OF SLAB ELEVATIONS SHALL BE ADJUSTED BY THE DIFFERENCE BETWEEN THE CAST-IN-PLACE DECK THICKNESS AND THE DIFFERENCE BETWEEN THE CAST-IN-PLACE SLAB DEFLECTION AND DECK PANEL DEAD LOAD DEFLECTION.

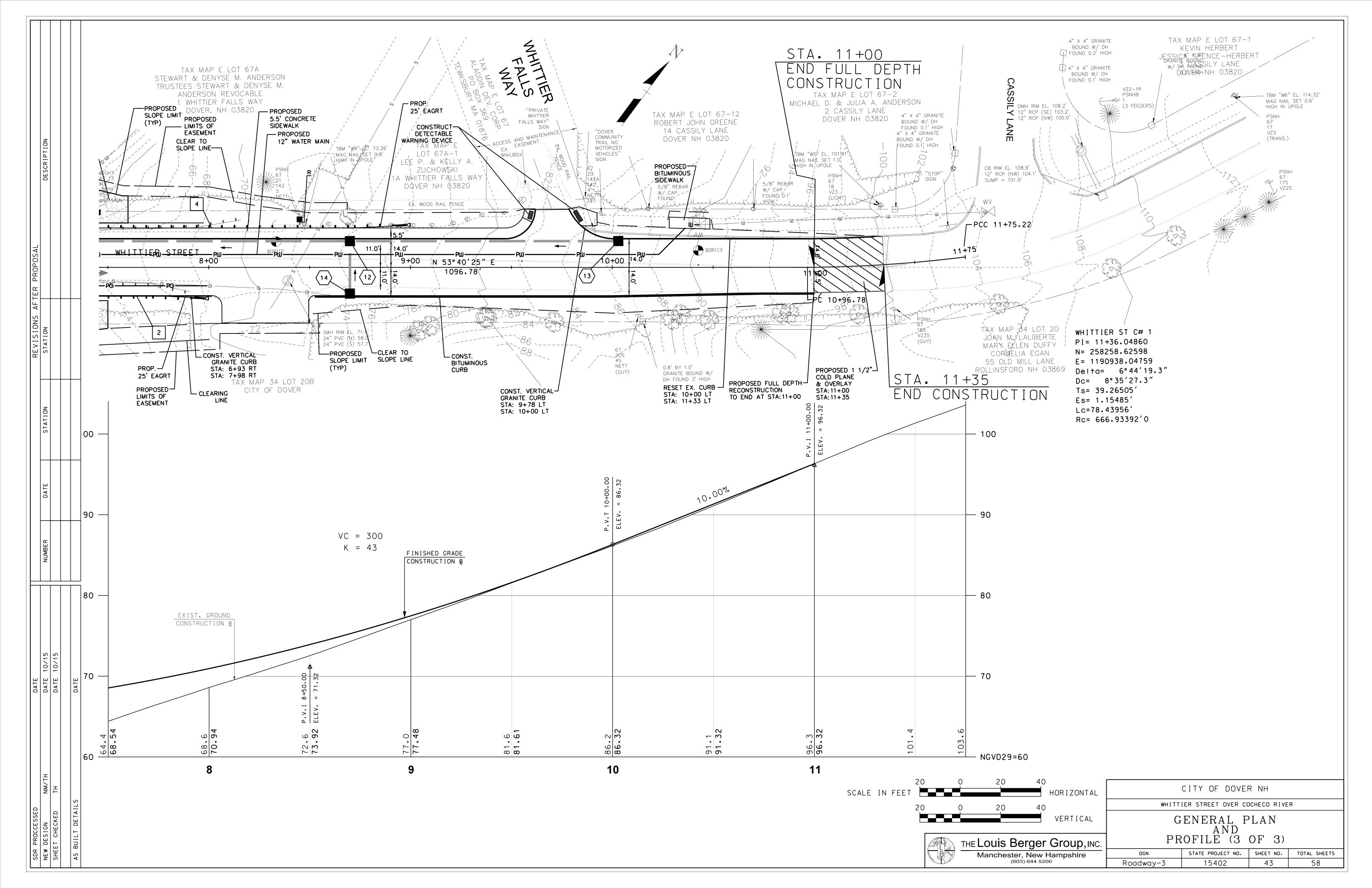
CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES

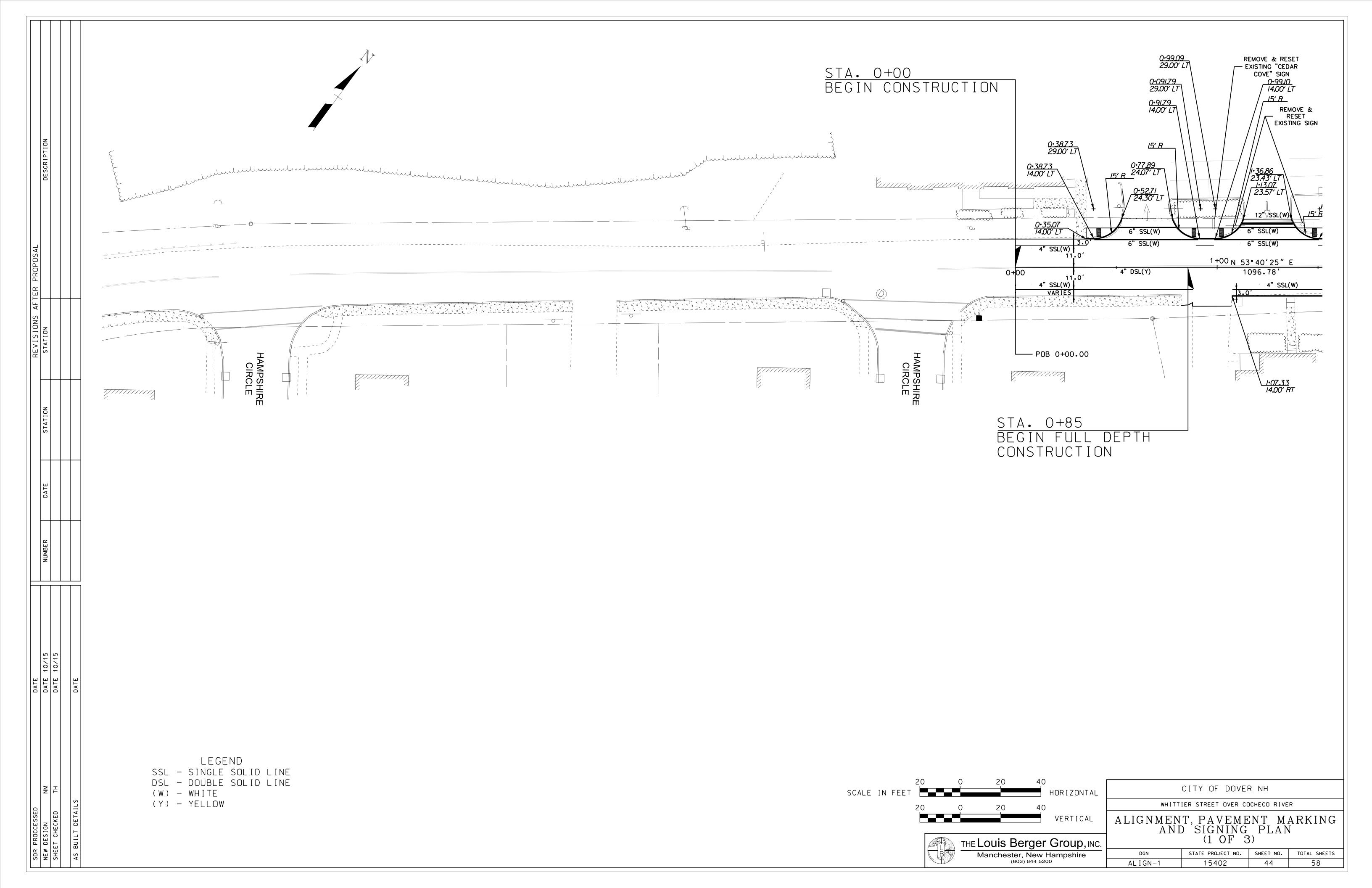
LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111\132 STATE PROJECT 15402

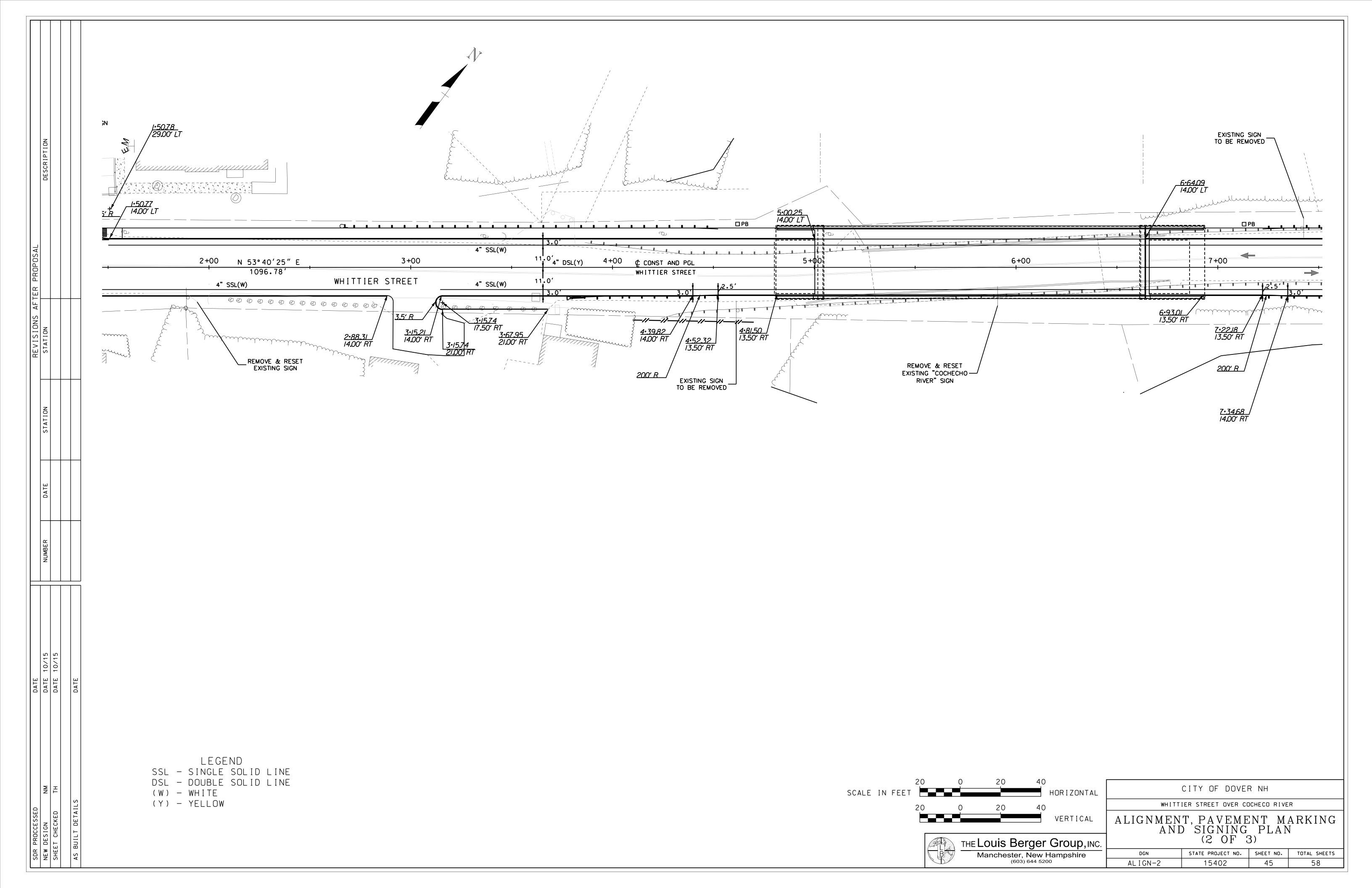
PRECAST CONCRETE DECK PANEL DETAILS											
REVISIONS AFTER PROPOSAL			BY	DATE			BY	DATE	35 of 35		
		DESIGNED	NHDOT	3/02	CHECKED	NHD	ОТ		33 % 33		
		DRAWN	NHDOT	5/08	CHECKED	NHD		10/05	FILE NUMBER		
		QUANTITIES	TWP	11/15 CHECKED HN		Н	11/15				
		ISSUE DATE		FEDERAL PROJECT NO.			SHEET NO.		TOTAL SHEETS		
		REV. DATE		X-A002(794)				40	58		

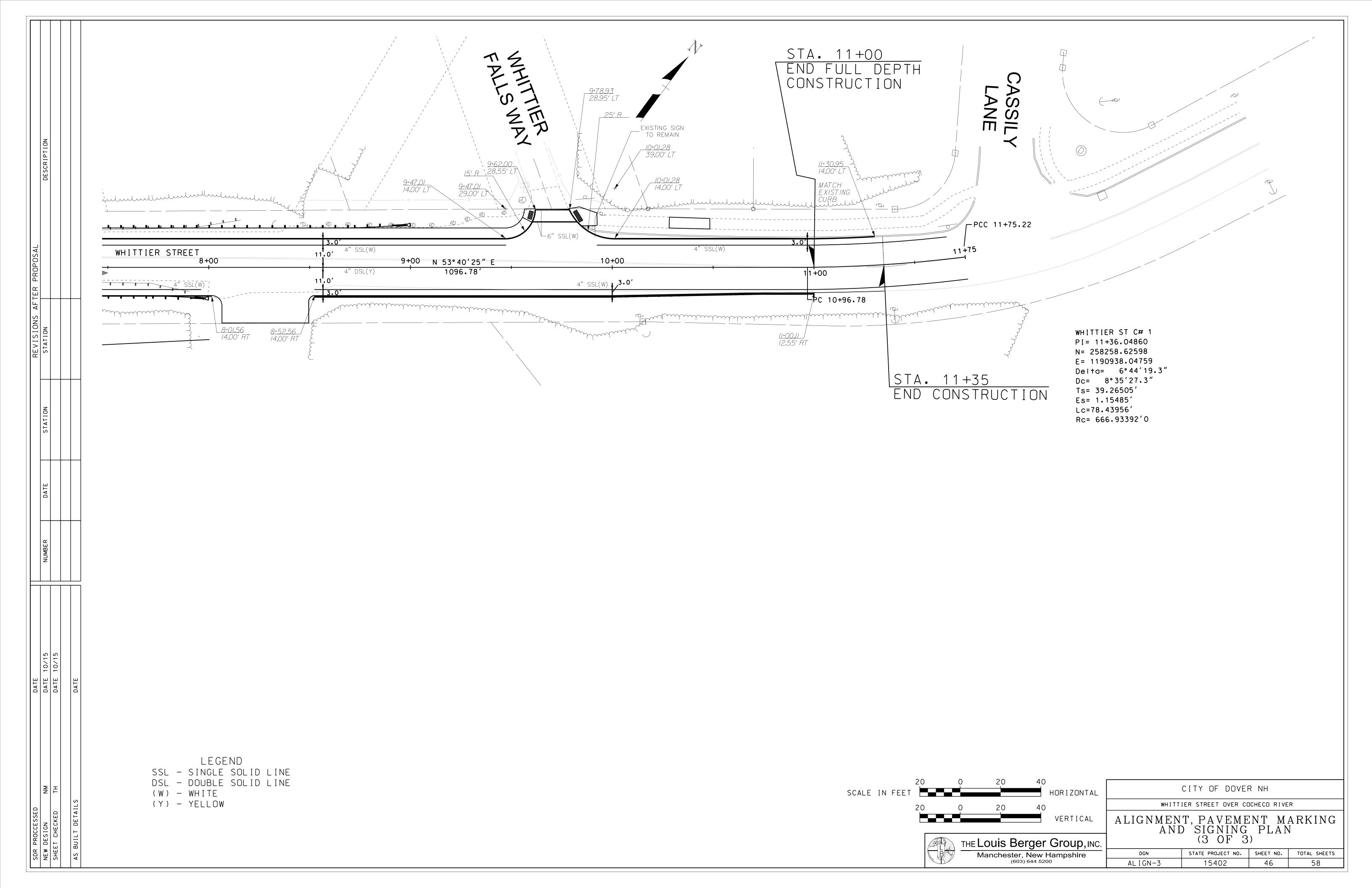


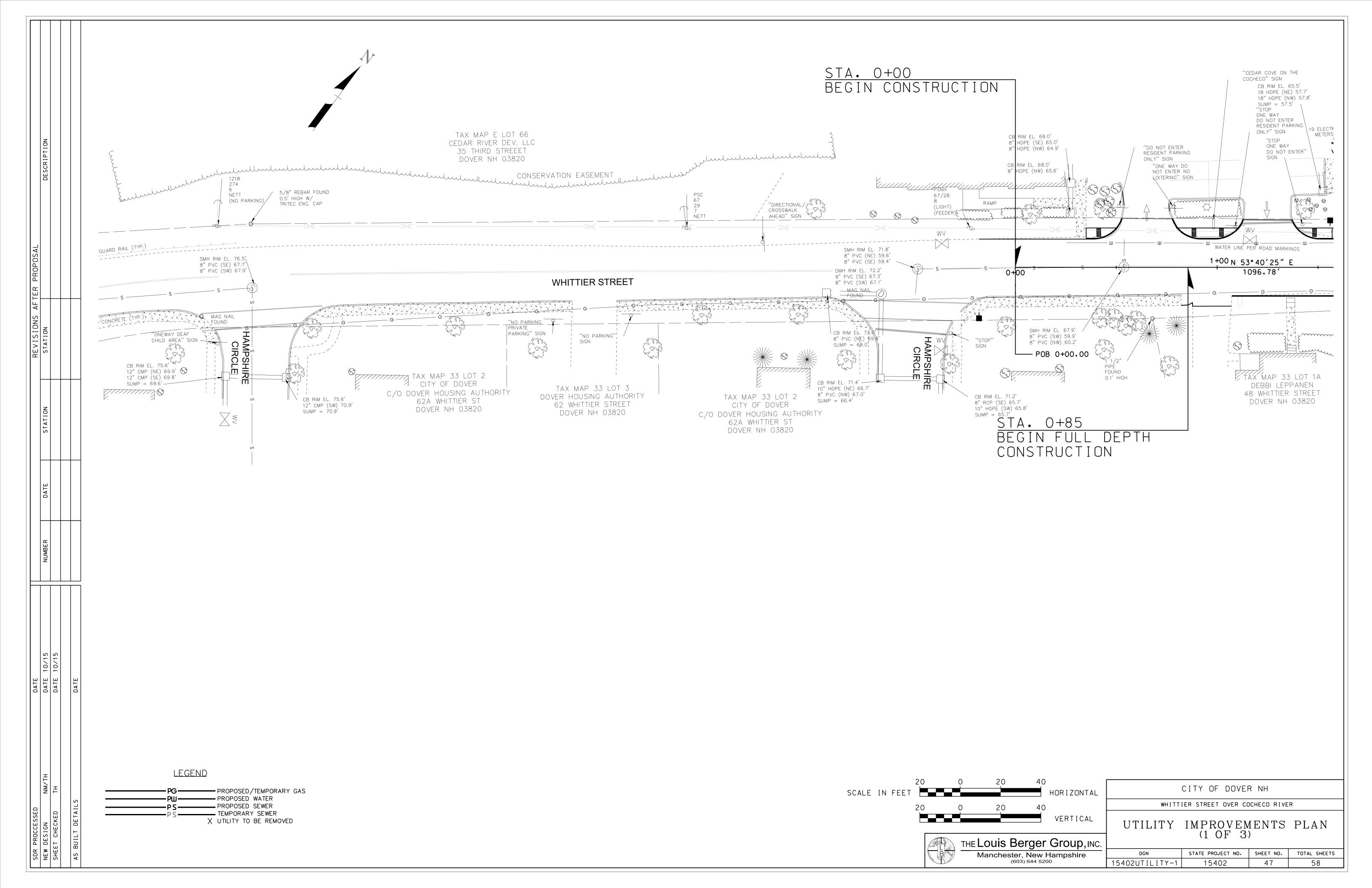


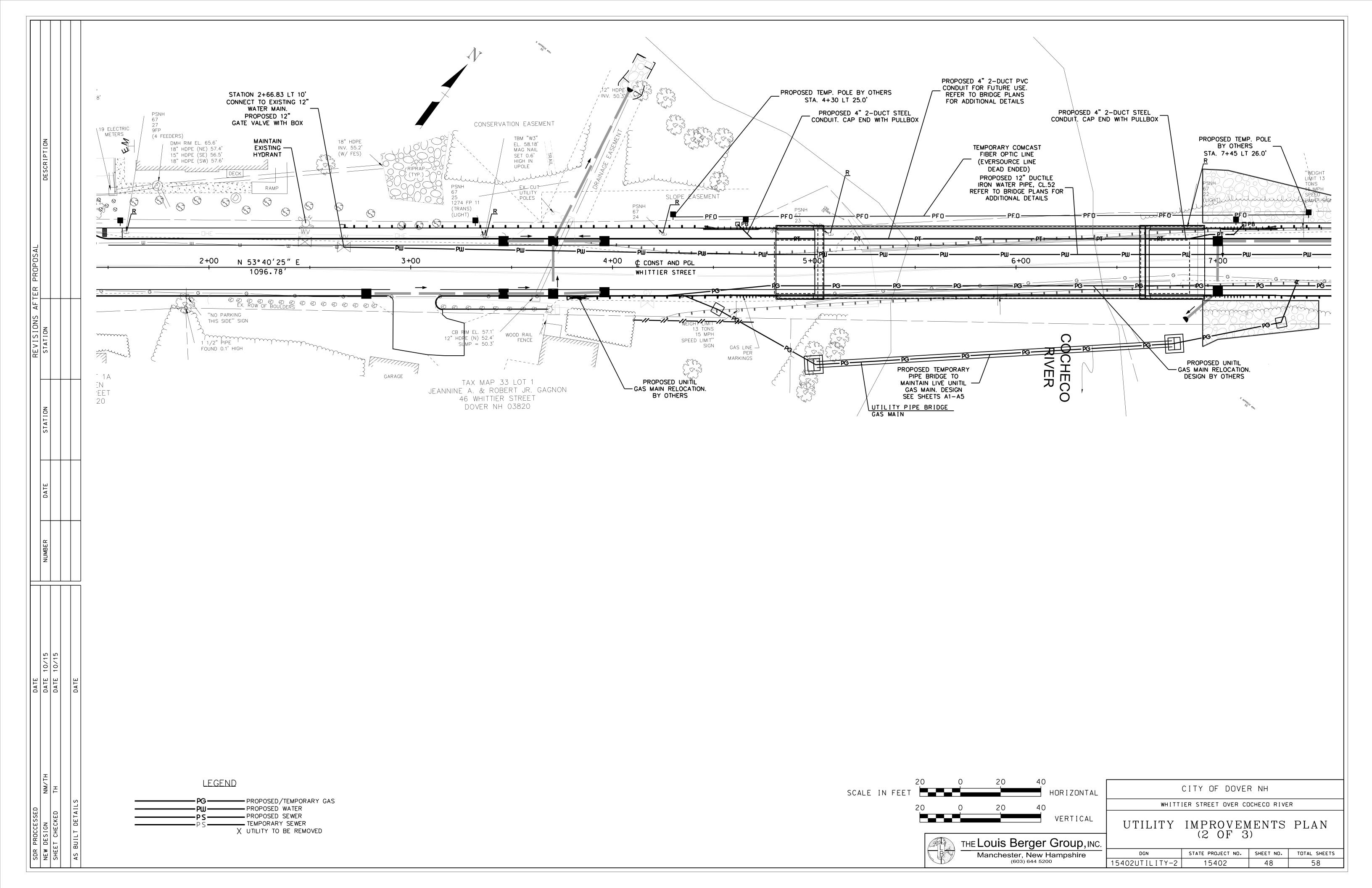


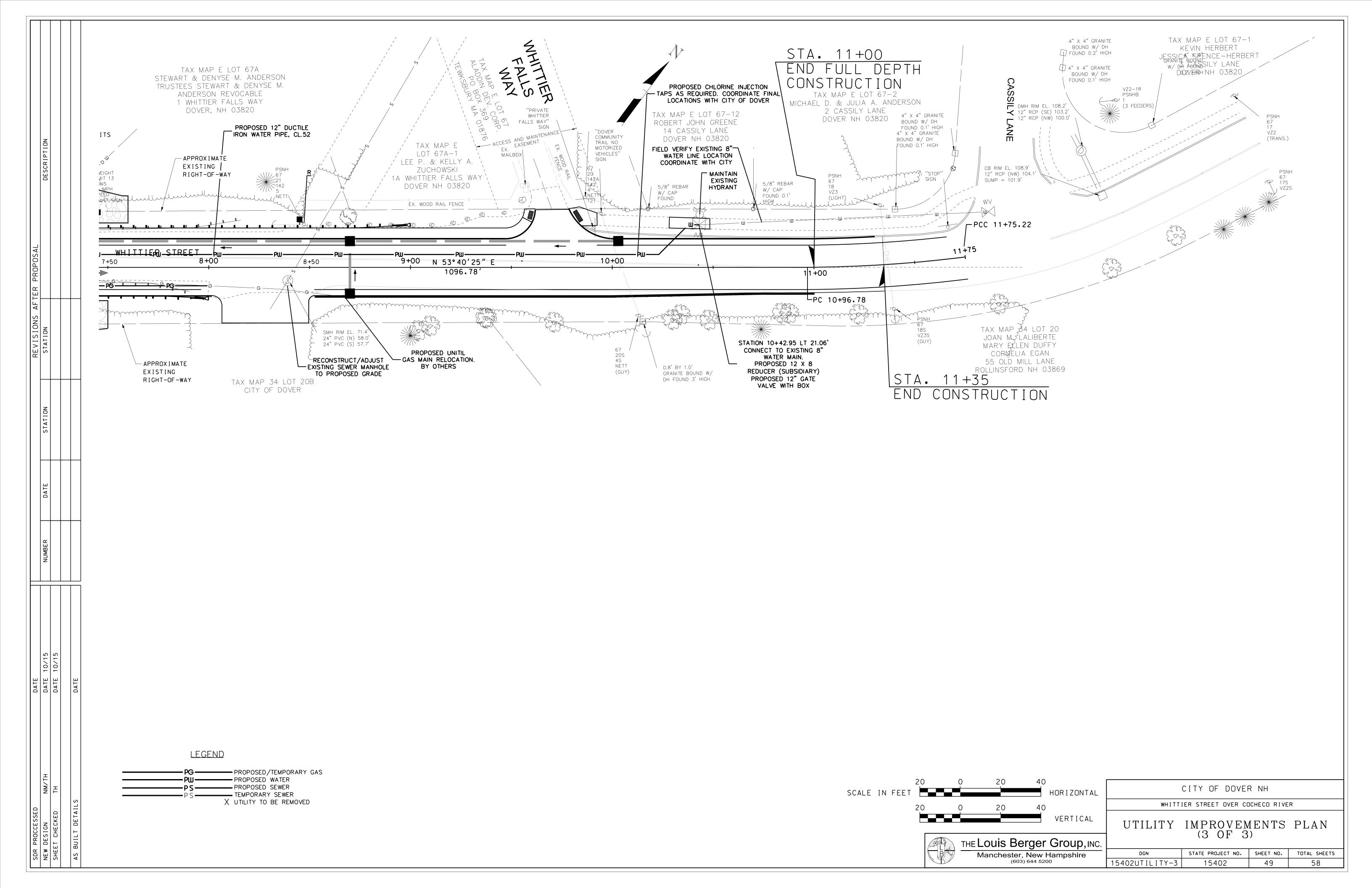


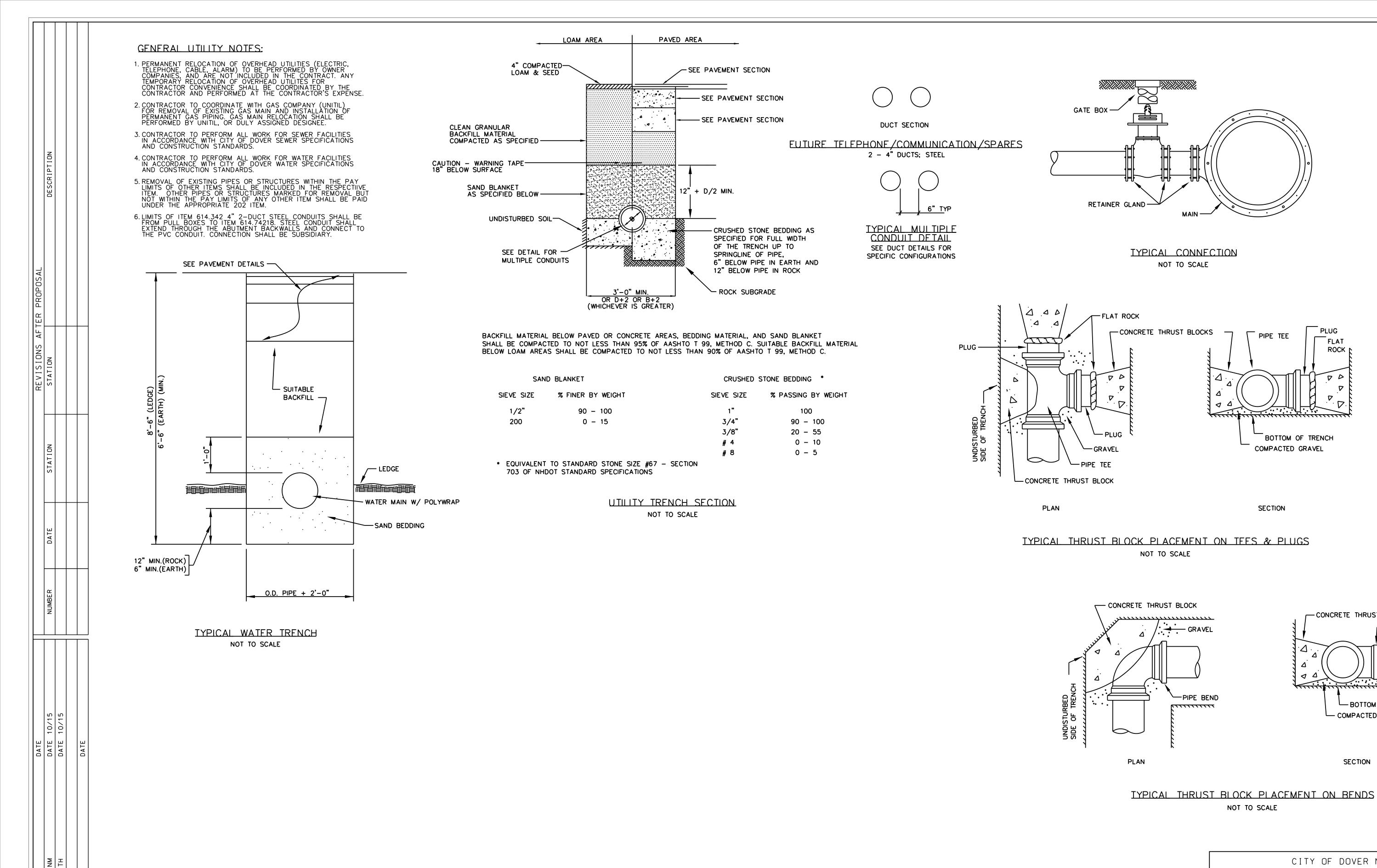












CITY OF DOVER NH WHITTIER STREET OVER COCHECO RIVER UTILITY DETAILS STATE PROJECT NO. DGN SHEET NO. TOTAL SHEETS 15402UTILITY-4 15402 50 58

FLAT

ROCK

- CONCRETE THRUST BLOCK

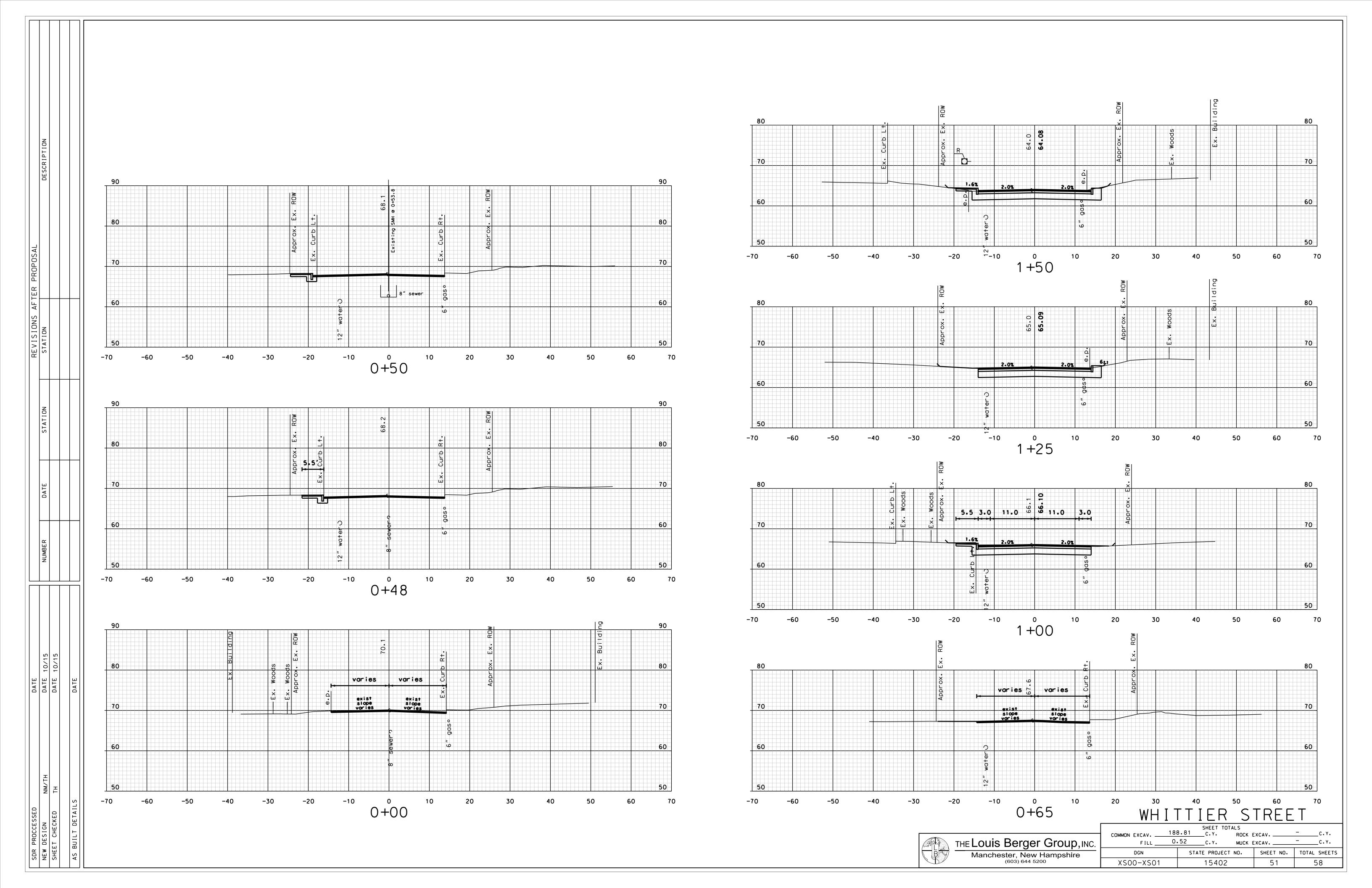
- PIPE BEND

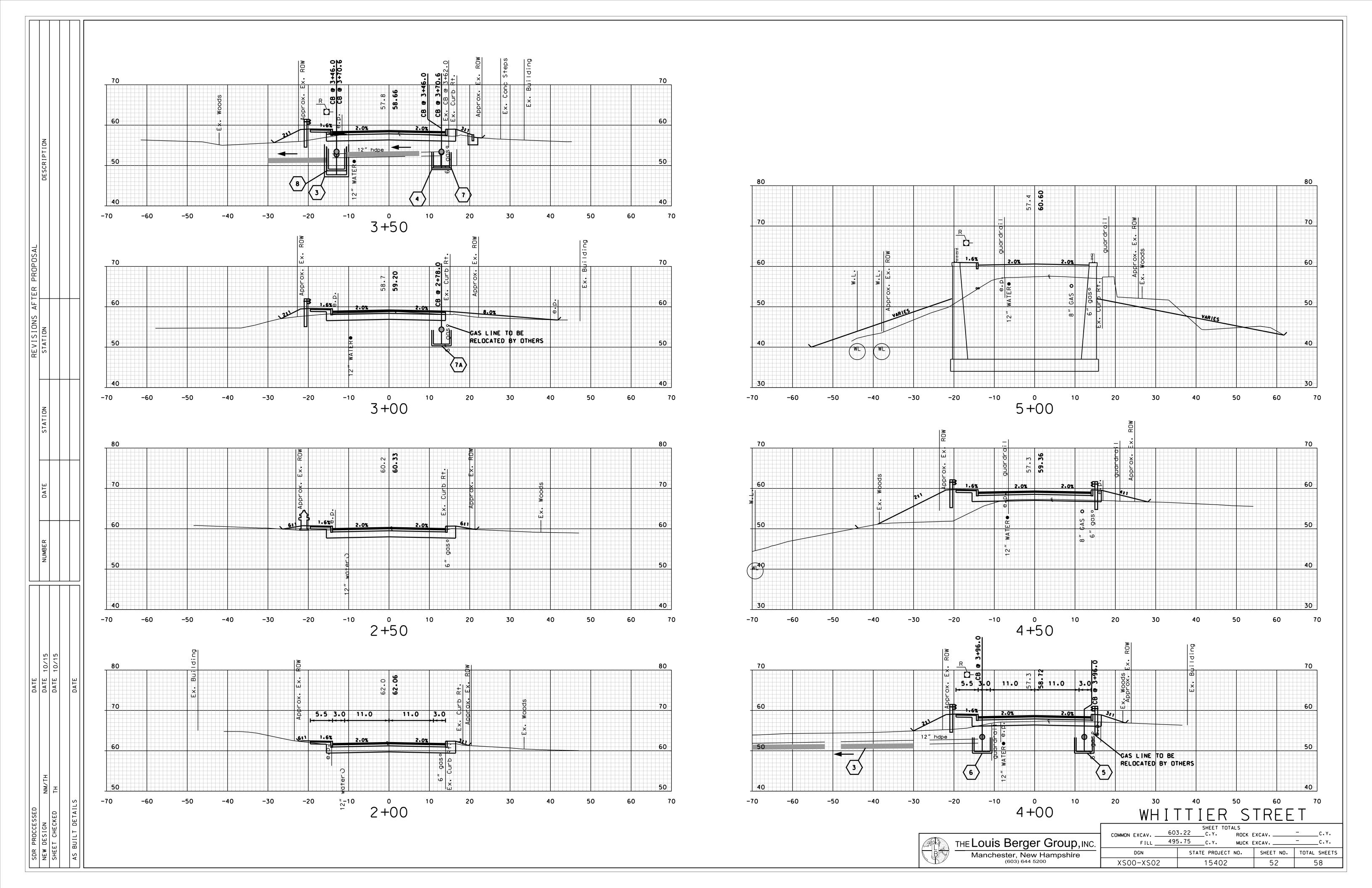
BOTTOM OF TRENCH

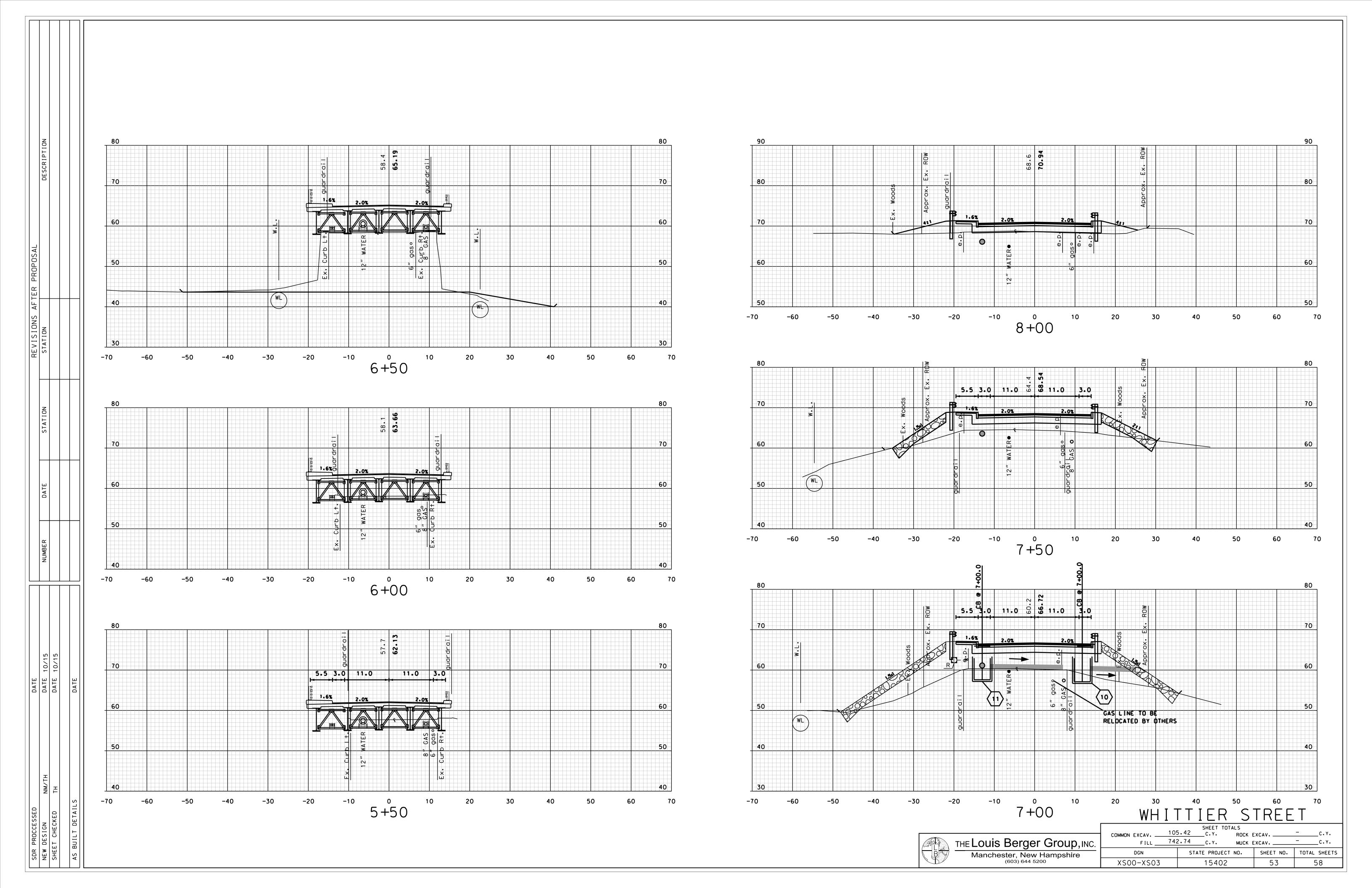
COMPACTED GRAVEL

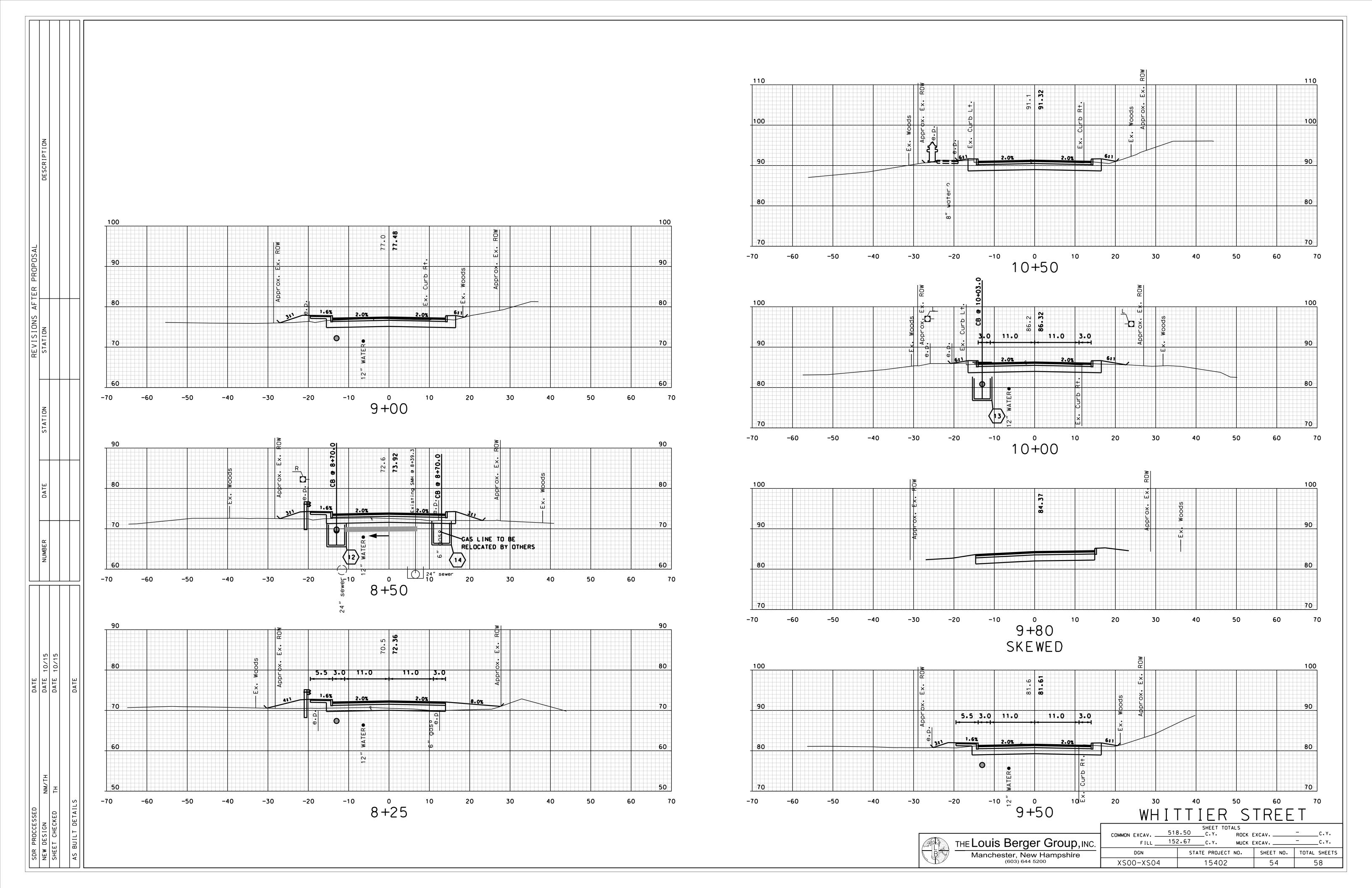
SECTION

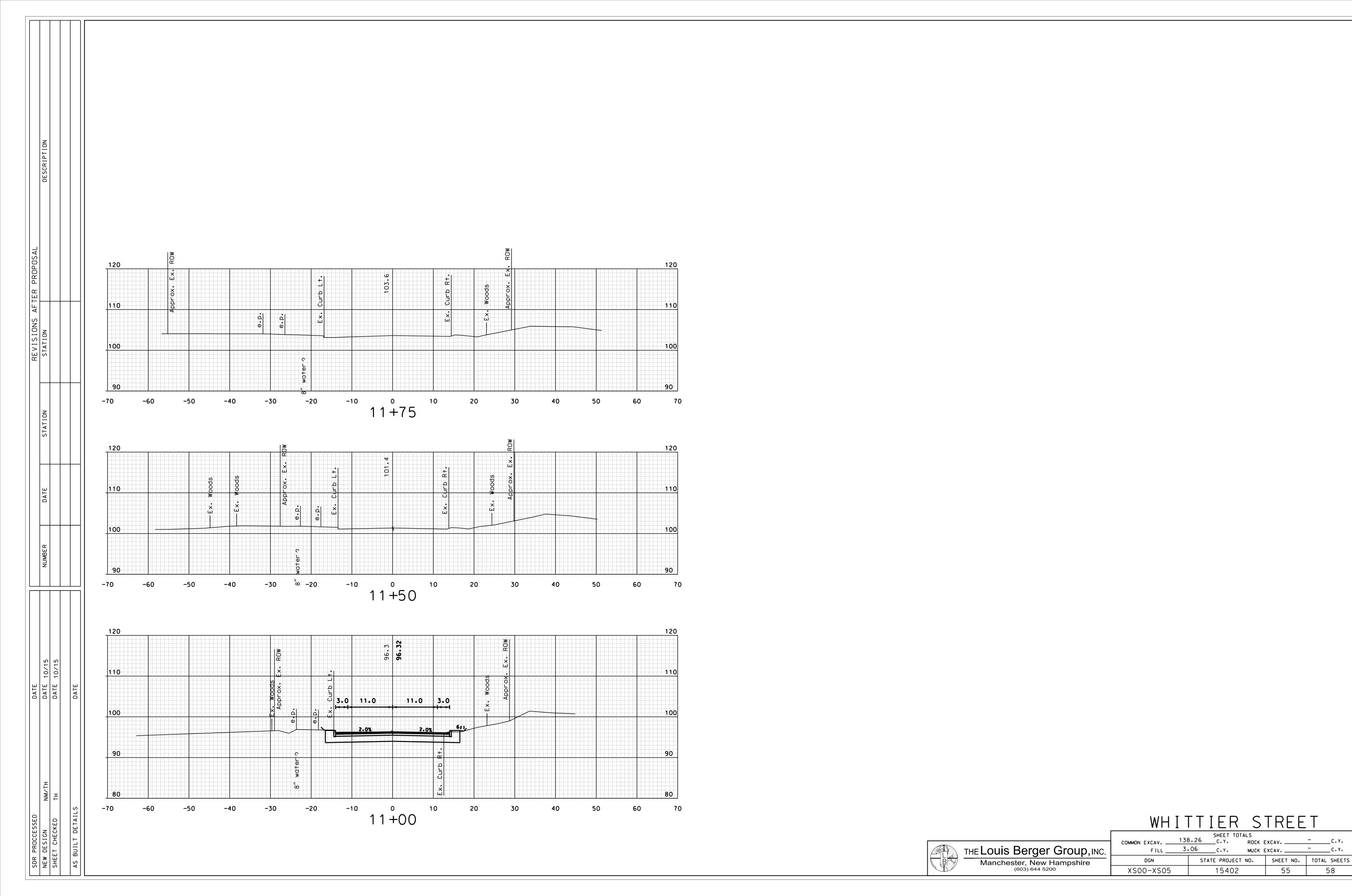
THE Louis Berger Group, INC. Manchester, New Hampshire

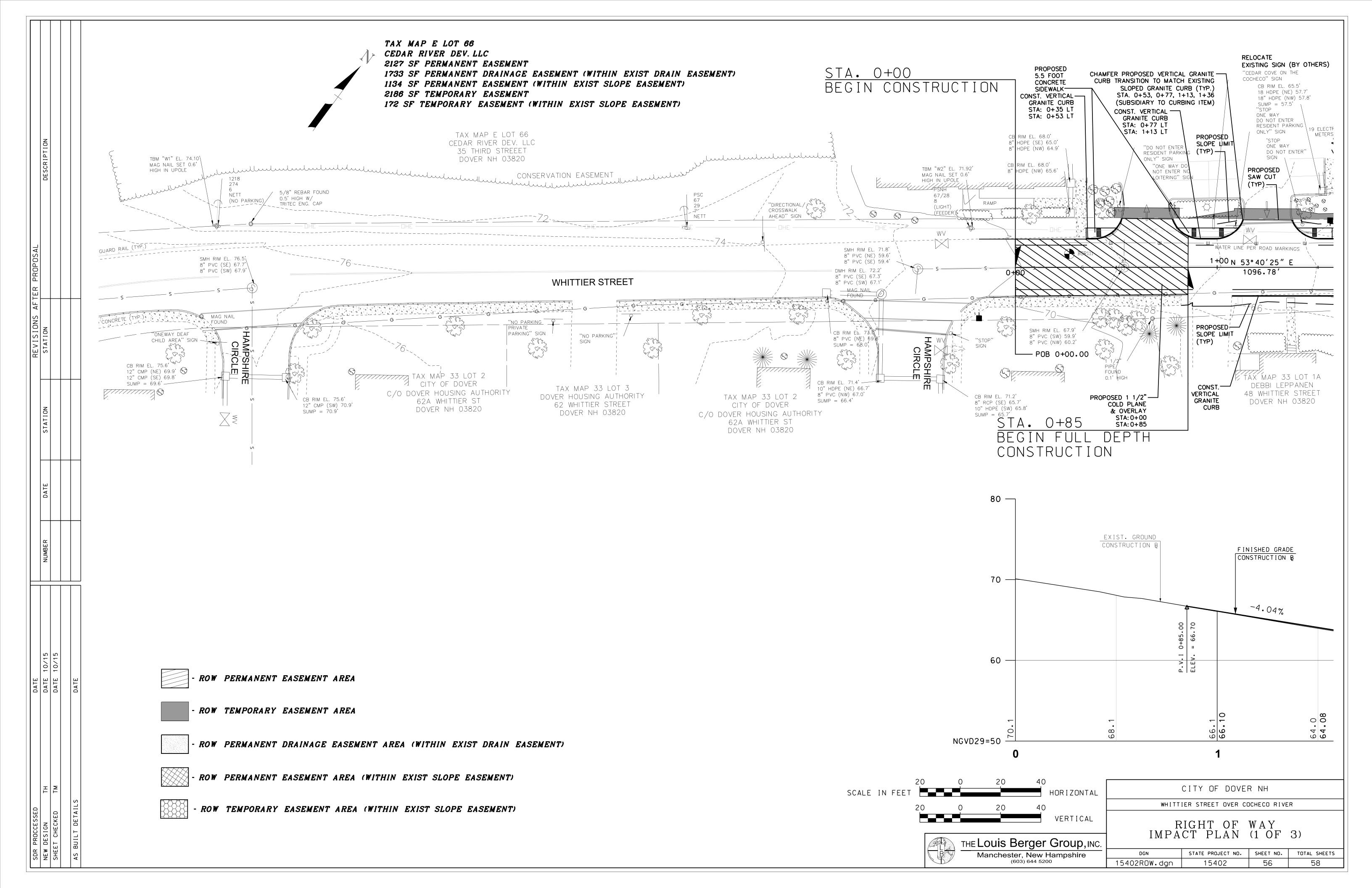


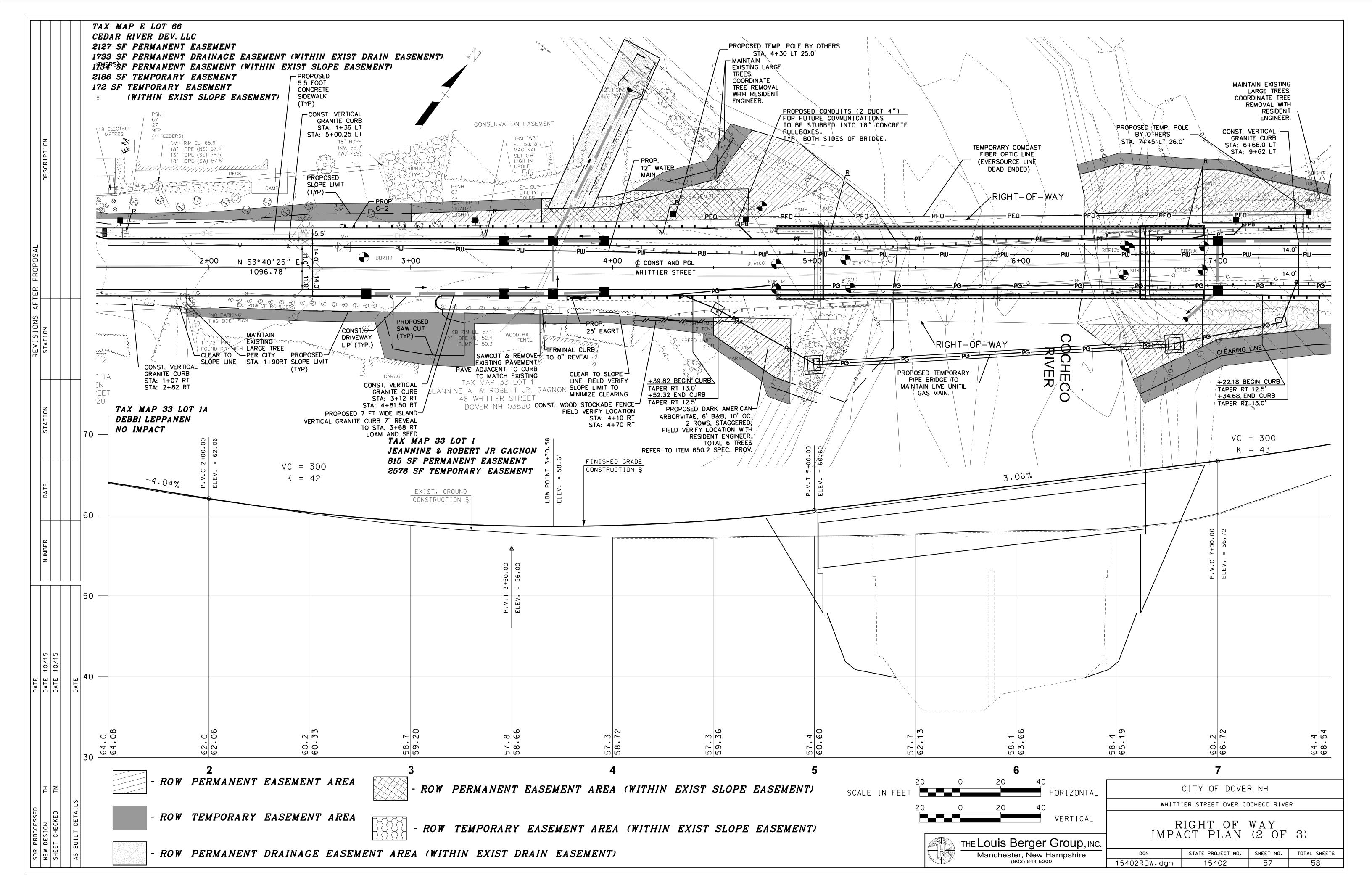


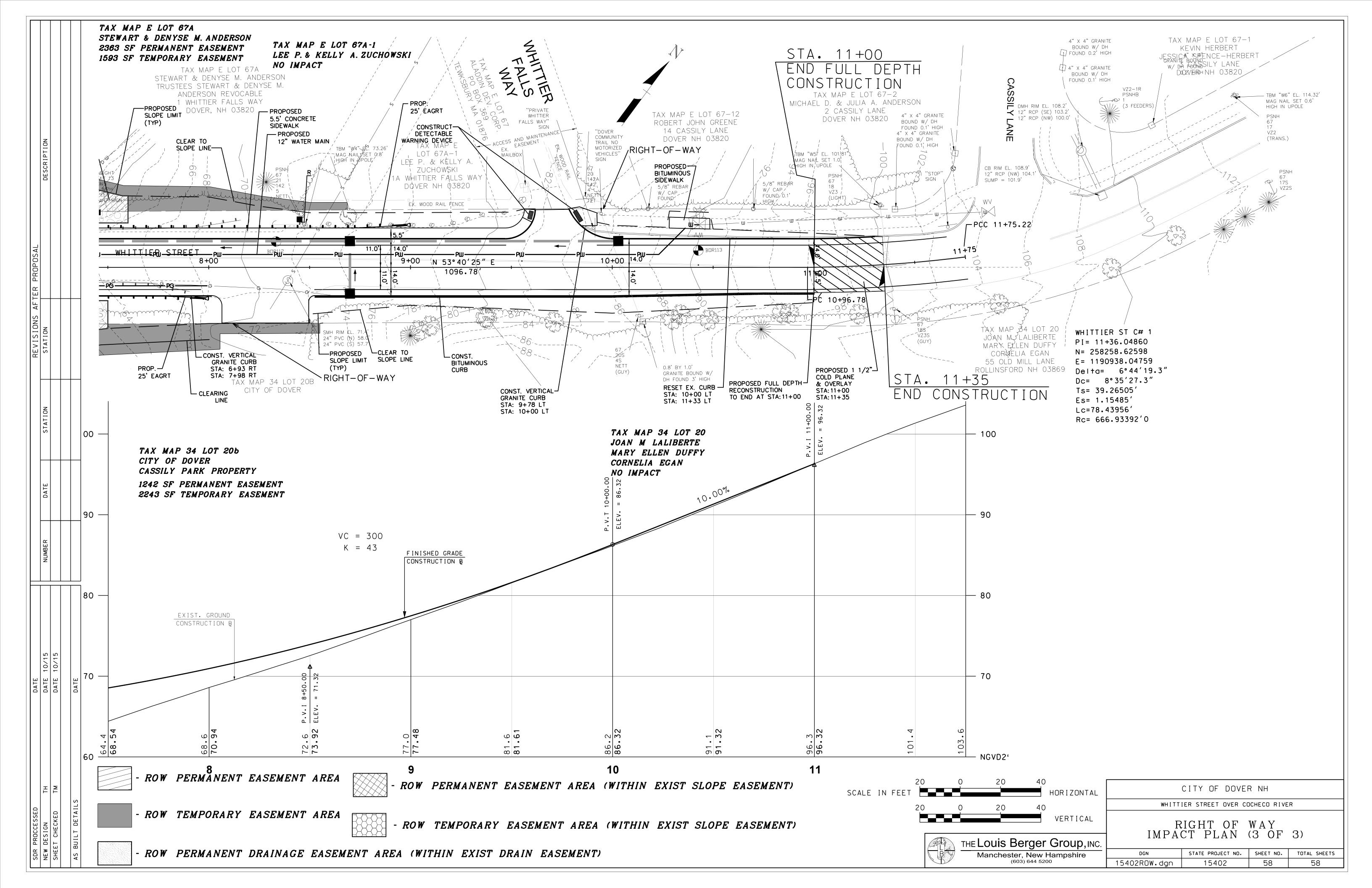


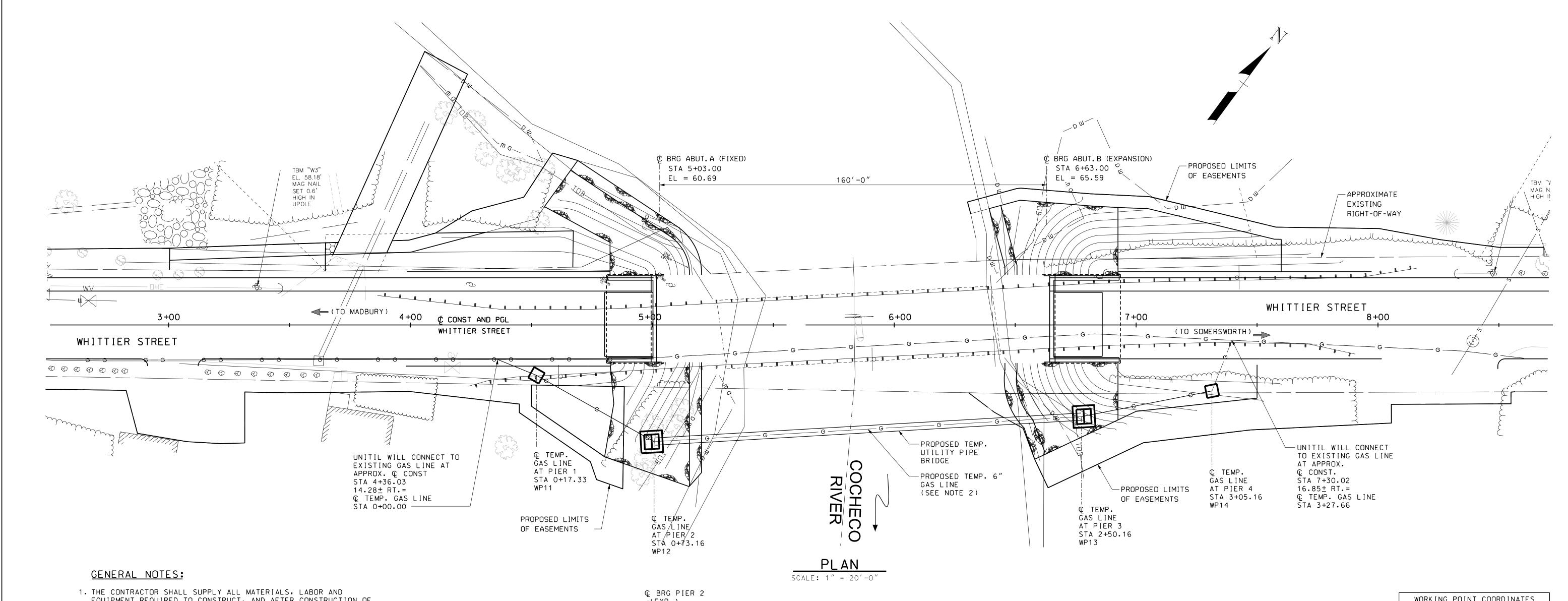




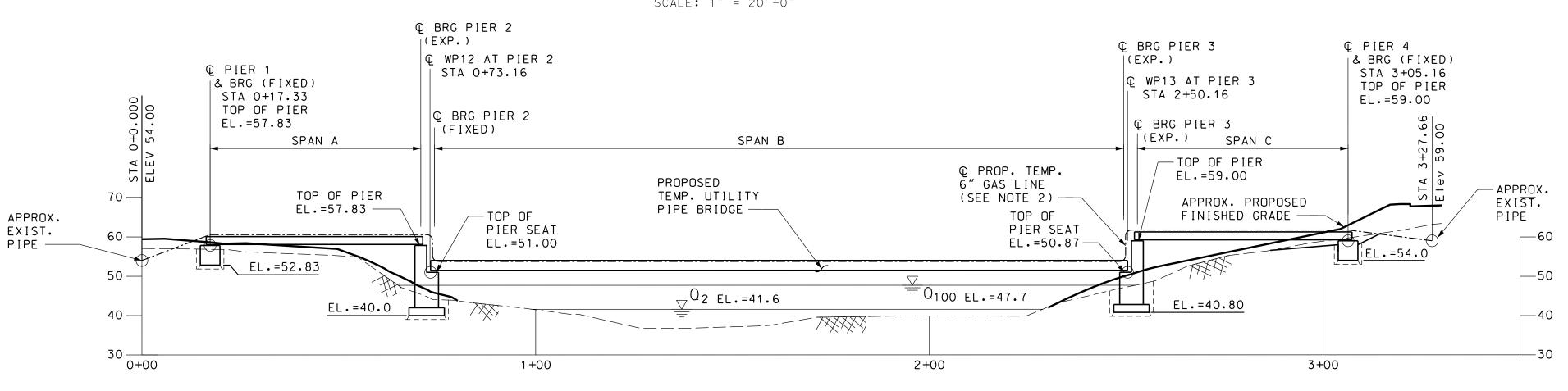








- EQUIPMENT REQUIRED TO CONSTRUCT, AND AFTER CONSTRUCTION OF THE NEW WHITTIER STREET BRIDGE REMOVE, THE TEMPORARY UTILITY PIPE BRIDGE FOR THE TEMPORARY UNITIL GAS LINE AS SHOWN IN THESE PLANS AND IN ACCORDANCE WITH THE SPECIAL PROVISION. ALL COSTS FOR THE TEMPORARY UTILITY PIPE BRIDGE SHALL BE INCLUDED IN ITEM 612.99, UTILITY PIPE BRIDGE - GAS
- 2. ALL MATERIALS. LABOR AND EQUIPMENT REQUIRED TO CONSTRUCT THE TEMPORARY GAS LINE ITSELF AND CONNECT IT TO THE EXISTING GAS LINE ON THE BRIDGE APPROACHES WILL BE SUPPLIED BY UNITIL. THIS INCLUDES THE GAS PIPE, PIPE EXPANSION JOINTS, PIPE VALVES, PIPE ROLLERS, THREADED RODS AND NUTS SUPPORTING PIPE ROLLERS, AND CONNECTION TO THE EXISTING GAS LINE ON THE BRIDGE APPROACHES.
- 3. AT THE CONTRACTOR'S OPTION, THE CONTRACTOR MAY PROPOSE AN ALTERNATE TEMPORARY UTILITY PIPE SUPPORT SYSTEM TO UNITIL FOR REVIEW. ANY ALTERNATE SUPPORT SYSTEM MUST BE CONSTRUCTIBLE WITHIN THE LIMITS OF THE RIGHT-OF-WAY OR THE PROJECT'S CONSTRUCTION EASEMENTS. IF THE CONTRACTOR CHOOSES TO PROPOSE AN ALTERNATE TEMPORARY UTILITY PIPE SUPPORT SYSTEM, THE CONTRACTOR SHALL SUBMIT PLANS AND DESIGN CALCULATIONS FOR THE ALTERNATE SYSTEM STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN NEW HAMPSHIRE TO UNITIL FOR REVIEW.
- 4. AFTER THE PERMANENT GAS LINE IS OPERATIONAL ON THE NEW WHITTIER STREET BRIDGE, THE ENTIRE TEMPORARY UTILITY PIPE BRIDGE SHALL BE REMOVED BY THE CONTRACTOR AND SHALL BECOME THE PROPERTY OF THE CONTRACTOR. PORTIONS OF THE CONCRETE PIERS AND PIER FOOTINGS TWO FEET OR MORE BELOW PROPOSED FINISHED GRADE MAY BE LEFT IN PLACE.
- 5. ALL WORK SHALL BE IN CONFORMANCE WITH THE NHDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION 2010.
- 6. STRUCTURAL STEEL SHALL BE AASHTO M270, GRADE 50W (ASTM A709, GRADE 50W) UNPAINTED UNLESS OTHERWISE NOTED.
- 7. CONCRETE FOR PIERS AND PIER FOOTINGS SHALL BE CLASS A WITH A DESIGN COMPRESSIVE STRENGTH = 4000 PSI.
- 8. REINFORCING STEEL SHALL BE AASHTO M31 (ASTM A615), GRADE 60 UNCOATED.



SCALE: 1'' = 20' - 0''

	0 1 0 1 1 1 1 0 0 0					
WORKING POINT	NORTHING	EASTING				
WP11 WP12 WP13 WP14	257843.20 257956.21	1190399.49 1190454.92 1190591.15 1190628.33				





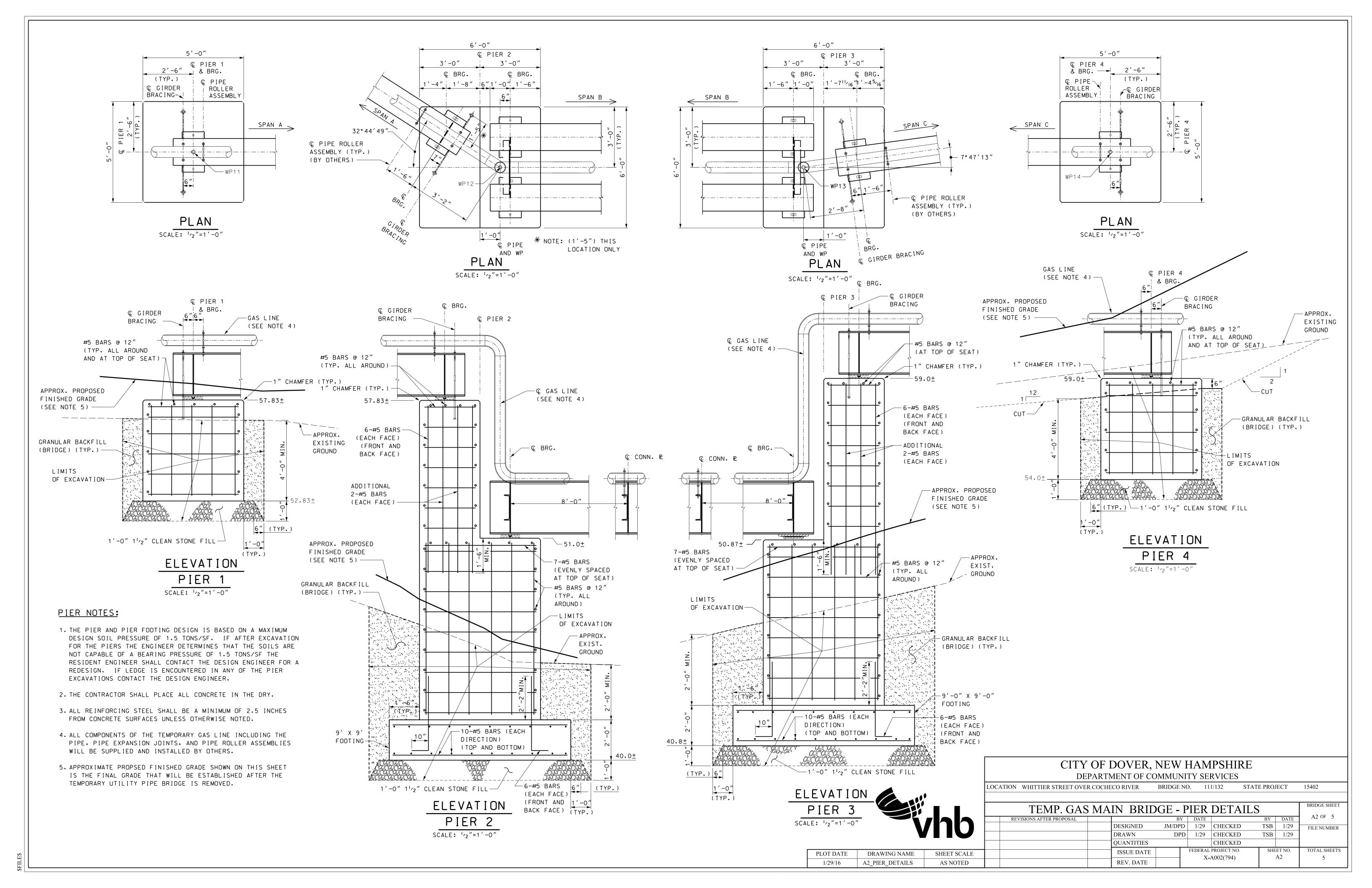
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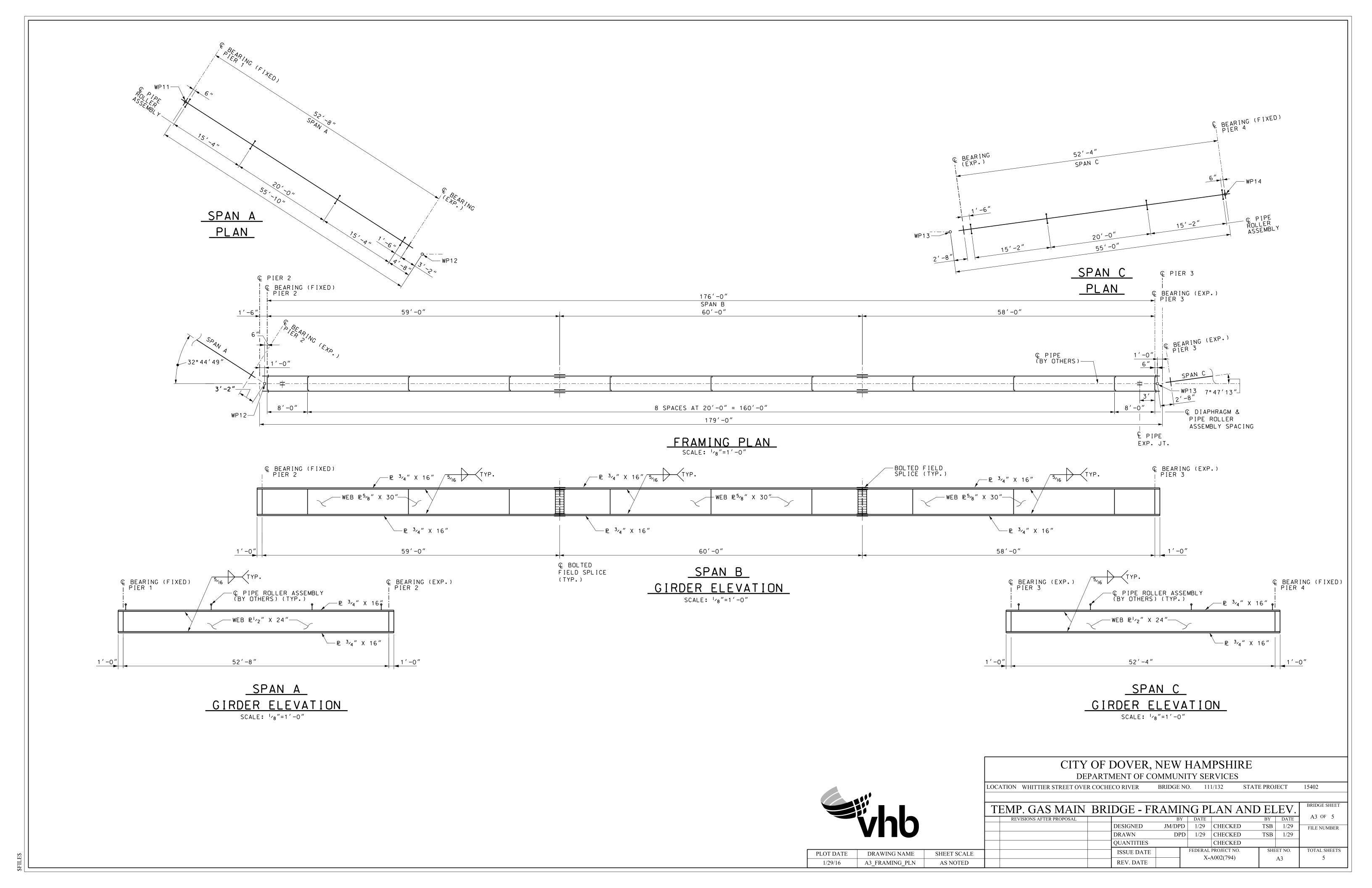
CITY OF DOVER, NEW HAMPSHIRE DEPARTMENT OF COMMUNITY SERVICES LOCATION WHITTIER STREET OVER COCHECO RIVER BRIDGE NO. 111/132 STATE PROJECT

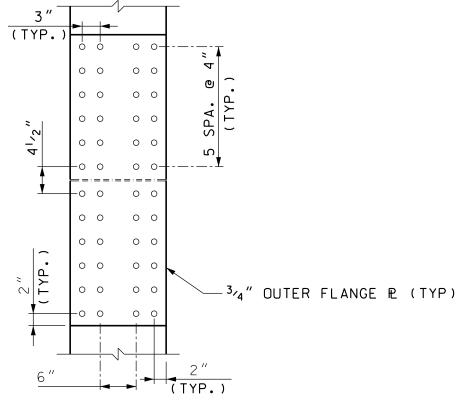
TEMP. GAS MAIN BRIDGE - PLAN AND PROFILE										
REVISIONS AFTER PROPOSAL BY DATE BY DATE										
	DESIGNED	JM/DPD	1/29	CHECKED	TSB	1/29	FILE NUMBER			
	DRAWN	DPD	1/29	CHECKED	TSB	1/29				
	QUANTITIES			CHECKED						
	ISSUE DATE		FEDERAL PROJECT NO. X-A002(794)		SHEET NO.		TOTAL SHEETS			
	REV. DATE		Λ-	4002(794)	A1		S			

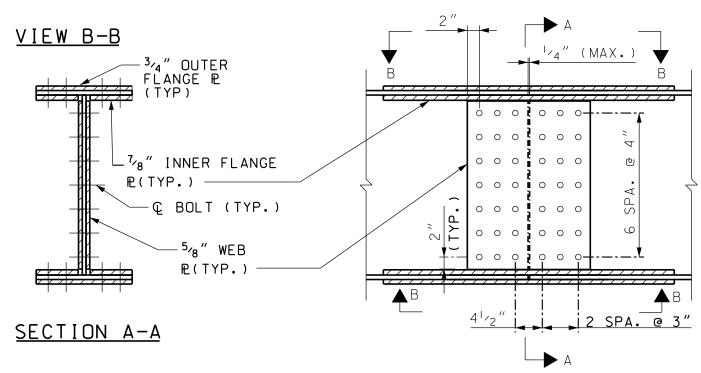
DRAWING NAME

SHEET SCALE A1_PLAN_ELEV AS NOTED

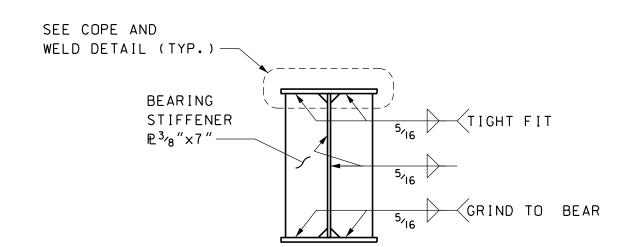






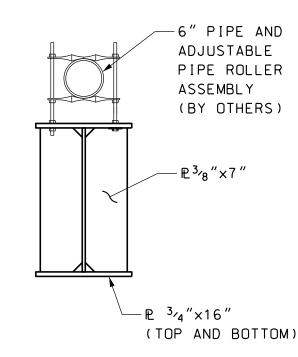


<u>ELEVATION</u> SPAN B BOLTED FIELD SPLICE SCALE: 3/4" = 1'-0"



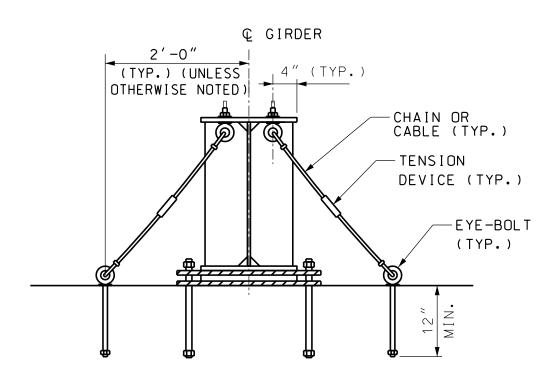
BEARING STIFFENER

(SPAN A & C SHOWN, B SIMILAR) (DIAPHRAM NOT SHOWN) SCALE: 3/4'' = 1'-0''

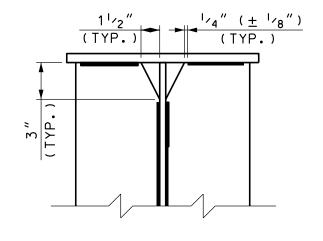


SPAN A & C UTILITY SUPPORT DETAIL

SCALE: 3/4'' = 1'-0''

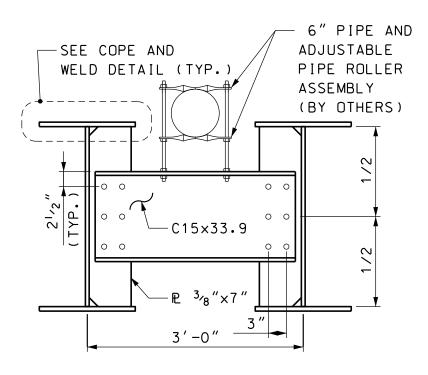


SPAN A & C GIRDER BRACING (PIPE AND ATTACHMENT NOT SHOWN) SCALE: 3/4'' = 1'-0''



COPE AND WELD DETAIL

SCALE: $1^{1}/_{2}^{"} = 1^{'}-0^{"}$



SPAN B UTILITY SUPPORT & DIAPHRAM

SCALE: 3/4'' = 1'-0''

SUPERSTRUCTURE NOTES

- 1. ALL STRUCTURAL STEEL SHALL MEET THE REQUIREMENTS OF ITEM 550,108, STRUCTURAL STEEL (F), INCLUDING THE GIRDERS, CROSS FRAMES, DIAPHRAGMS, GUSSET PLATES, FILL PLATES, CONNECTION PLATES, SPLICE PLATES, STIFFENERS, AND FASTENERS.
- 2. NOTCH TOUGHNESS REQUIREMENTS OF NHDOT STANDARD SPECIFICATIONS SHALL APPLY TO THE WEB AND FLANGES OF GIRDERS AND SPLICE PLATES.
- 3. ALL BOLTED CONNECTIONS SHALL BE SLIP-CRITICAL (CLASS-B) MADE WITH %'' ϕ HIGH STRENGTH BOLTS IN $^{15}\!\!/_6$ " ϕ HOLES. ALL FASTENERS SHALL CONFORM TO REQUIREMENTS FOR AASHTO M164 (ASTM A325) TYPE 3.
- 4. ALL BOLTS IN FIELD SPLICE SHALL HAVE THREADS EXCLUDED FROM THE THICK FLANGES.
- 5. DIRECT TENSION INDICATOR WASHERS SHALL BE INSTALLED WITH HIGH STRENGTH BOLTS.
- 6. GIRDERS SHALL BE CAMBERED FOR FULL DEAD LOAD DEFLECTION EQUAL TO: SPANS A & C = $\frac{1}{2}$ " AT MIDSPAN SPAN B = $21 \frac{1}{2}$ " AT MIDSPAN
- 7. BEARING STIFFENERS AND ENDS OF THE GIRDERS SHALL BE VERTICAL UNDER FULL DEAD LOAD DEFLECTION.
- 8. ALL WELDS SHALL HAVE CORROSION RESISTANCE AND WEATHERING APPEARANCE AS SPECIFIED FOR WEATHERING STRUCTURAL STEEL.
- 9. THE STRUCTURAL STEEL FABRICATOR SHALL ARRANGE FOR NON-DESTRUCTIVE TESTING OF THE WELDS.
- 10. SHOP DRAWINGS SHALL INDICATE THE METHOD AND SEQUENCE TO BE FOLLOWED IN WELDING THE GIRDER COMPONENTS.
- 11. LOCATION OF WELDED SHOP SPLICES SHALL BE: WEB SPLICES SHALL BE LOCATED A MINIMUM OF 9" FROM WELDED FLANGE SPLICES. WEB AND FLANGE SPLICES SHALL BE LOCATED A MINIMUM OF 6" FROM TRANSVERSE STIFFENERS OR CONNECTION PLATES.
- 12. ANY SHOP OR FIELD WELDING OF ATTACHMENTS TO ANY PORTION OF THE PLATE GIRDERS FOR CONSTRUCTION PURPOSES WILL NOT BE PERMITTED, UNLESS APPROVED BY THE ENGINEER.
- 13. THE CONTRACTOR SHALL SUBMIT A HANDLING AND ERECTION PROCEDURE TO THE ENGINEER PRIOR TO HANDLING THE STRUCTURAL STEEL IN ACCORDANCE WITH SECTION 550.3.14 AND 550.3.15. THE ERECTION PROCEDURE SHALL INDICATE THE LOCATION AND NUMBER OF LIFTING POINTS AS DETERMINED BY CHECKING THE L/B RATIOS IN ACCORDANCE WITH SECTION 550.3.14.2.4 TO GUARD AGAINST LATERAL BUCKLING OF THE GIRDERS.
- 14. STEEL ERECTION SHALL NOT BE PERMITTED UNTIL THE PIERS HAVE BEEN BACKFILLED TO THE LEVEL OF THE EXISTING GROUND SHOWN IN THESE PLANS.
- 15. ALL WELDING AND FABRICATION SHALL BE PREFORMED IN CONFORMANCE WITH THE AASHTO/AWS D1.5-08 BRIDGE WELDING CODE, (INCLUDING ALL REVISIONS PUBLISHED BY AASHTO AS OF THE BID OPENING DATE) AND THE NHDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.
- 16. THE CABLE OR CHAIN, TENSION DEVICE, AND ALL ATTACHMENTS USED IN THE GIRDER BRACING SYSTEM, SHALL BE CAPABLE OF WITHSTANDING A 8-KIP TENSILE LOAD.
- 17. THE CABLE OR CHAIN USED IN THE FLANGE SUPPORT SYSTEM SHALL HAVE APPROXIMATELY 2% SLACK UNDER THE NO-WIND CONDITION. THE CABLE OR CHAIN SHALL NOT BE TENSIONED SUCH THAT A DOWNWARD FORCE IS IMPOSED ON THE BEAM UNDER THE NO-WIND CONDITION.
- 18. THE FOLLOWING VERTICAL DEFLECTIONS DUE TO THE DESIGN PIPE LOAD AND ICE LOAD ARE ANTICIPATED ON THE GAS LINE: SPANS A & C = $\frac{1}{8}$ " AT MIDSPAN SPAN B = $3^{3}/4^{\prime\prime}$ AT MIDSPAN
- 19. THE FOLLOWING HORIZONTAL DEFLECTIONS DUE TO THE DESIGN WIND LOAD ARE ANTICIPATED ON THE GAS LINE:

SPANS A & C = 1.5/8" AT MIDSPAN

SPAN B = 2.7/8" AT MIDSPAN

20. HOLES IN THE SPAN A AND SPAN C GIRDER TOP FLANGES, AND THE SPAN B DIAPHRAGM TOP FLANGES, FOR THE THREADED RODS FOR THE PIPE ROLLERS SHALL BE SHOP DRILLED. CONTRACTOR SHALL COORDINATE FINAL LAYOUT AND SIZE OF THESE HOLES WITH UNITIL. THE HOLES SHALL BE SHOWN IN THE GIRDER SHOP DRAWINGS, ALL COSTS FOR THESE HOLES SHALL BE INCLUDED IN ITEM 612,99.

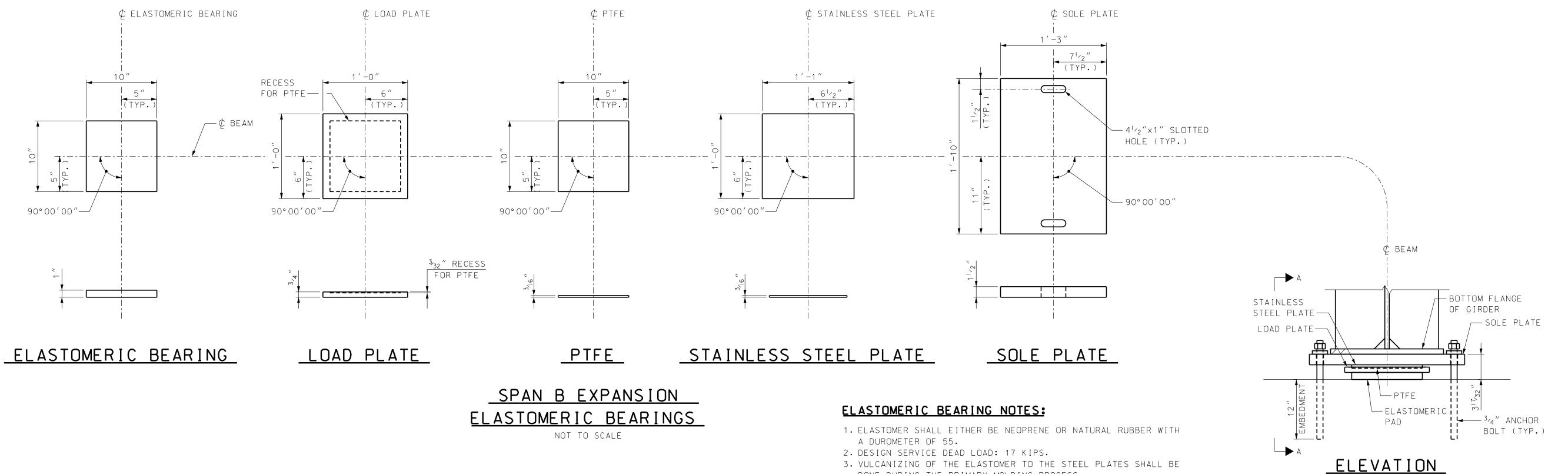


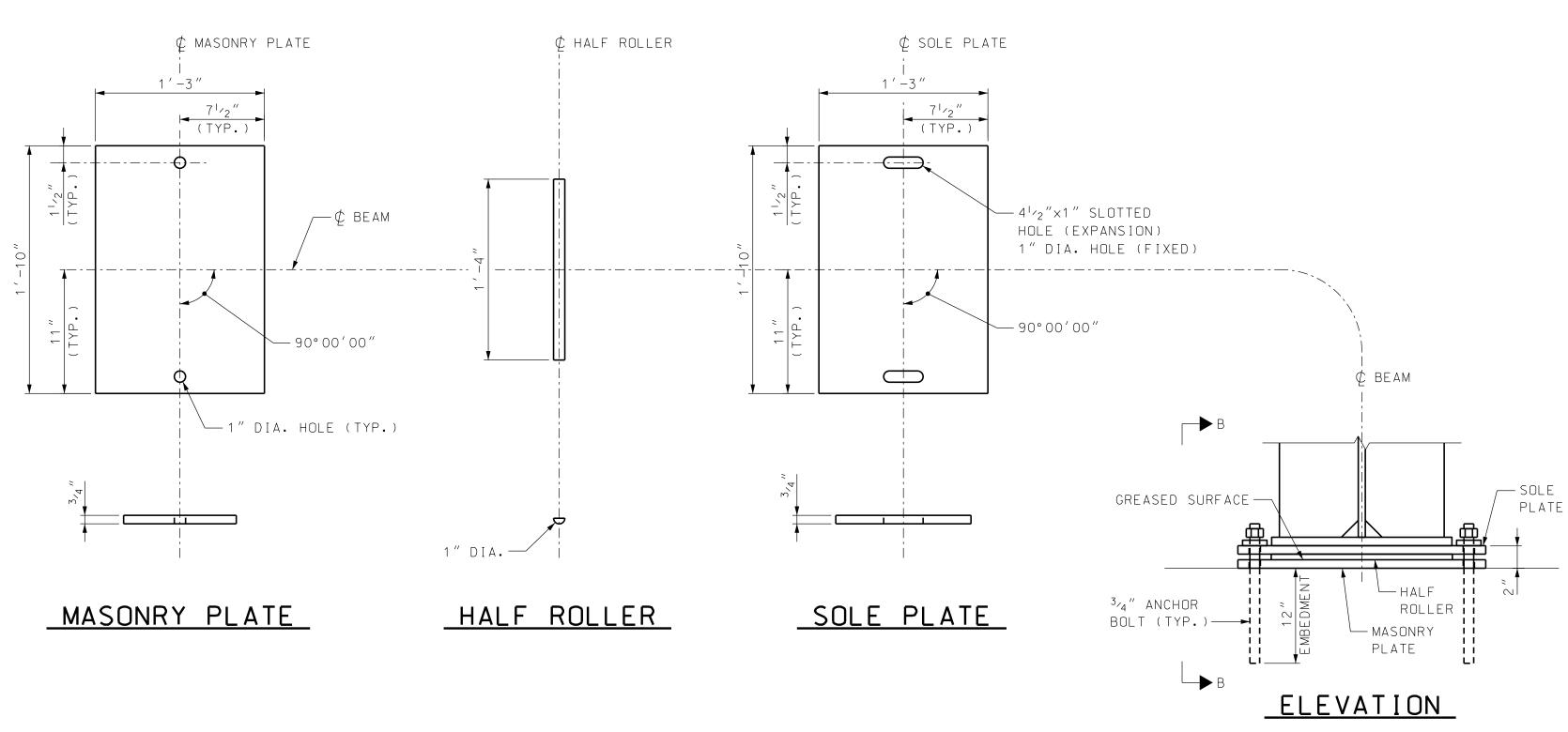
DRAWING NAME

A4 GIRDER DETAILS

CITY OF DOVER, NEW HAMPSHIRE									
DEPARTMENT OF COMMUNITY SERVICES									
LOCATION	WHITTIER STREET OVER COCHECO RIVER	BRIDGE NO.	111/132	STATE PROJECT	15402				

	тт	TEMP. GAS MAIN BRIDGE - GIRDER DETAILS AND NOTES								BRIDGE SHEET	
	11	EMIP. CAS MIAIN DRIDGE - CIRDER DETAILS AND NOTES									
		REVISIONS AFTER PROPOSAL			BY	DATE		BY	DATE	A4 OF 5	
				DESIGNED	JM/DPD	1/29	CHECKED	TSB	1/29	FILE NUMBER	
				DRAWN	DPD	1/29	CHECKED	TSB	1/29		
				QUANTITIES			CHECKED				
SHEET SCALE				ISSUE DATE			PROJECT NO.		EET NO.	TOTAL SHEETS	
AS NOTED				REV. DATE		Λ-	A002(794)		A4	3	





SPAN A, SPAN B (FIXED ONLY), AND SPAN C STEEL BEARINGS

¢ BRG. GREASED SURFACE — BOTTOM FLANGE OF BEAM — SOLE PLATE HALF ROLLER — - MASONRY PLATE 3/4" ANCHOR BOLT ──

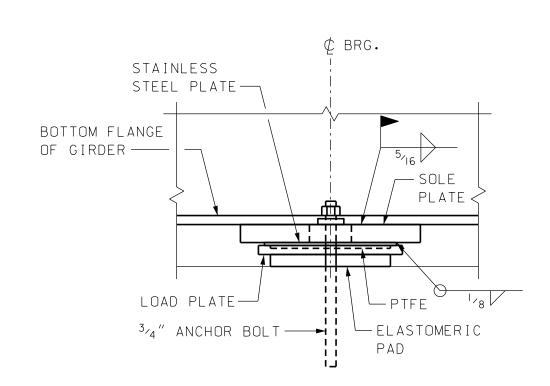
- DONE DURING THE PRIMARY MOLDING PROCESS. 4. SOLE PLATES AND LOAD PLATES SHALL MEET THE REQUIREMENTS OF
- ASTM A709, GRADE 50W.
- 5. HORIZONTAL PTFE AT EXPANSION BEARINGS SHALL BE DIMPLED AND LUBRICATED. LUBRICATION SHALL BE SUPPLIED AND INSTALLED PER THE MANUFACTURES RECOMMENDATION TO ENSURE A COEFFICIENT OF FRICTION LESS THAN OR EQUAL TO 0.03 AT 68 DEGREES F.
- 6. THE BEARINGS ARE DESIGNED SO THAT THE SUPERSTRUCTURE MAY BE ERECTED WHEN THE AMBIENT AIR TEMPERATURE IS WITHIN A RANGE OF 30 DEGREES F AND 90 DEGREES F. IF THE AMBIENT AIR TEMPERATURE IS OUTSIDE THIS RANGE, THE BEARING SHALL BE RESET AS DIRECTED BY THE RESIDENT.
- 7. BEARINGS SHALL BE COVERED DURING TRANSIT.
- 8. ALL NECESSARY PRECAUTIONS SHALL BE TAKEN TO PROTECT BEARING COMPONENTS FROM FIELD WELD FLASH AND SPLATTER, HEAT FROM WELDING OPERATION SHALL BE CONTROLLED SUCH THAT STEEL ADJACENT TO THE ELASTOMER DOES NOT EXCEED 200 DEGREES F. THE TEMPERATURE SHALL BE VERIFIED BY THE USE OF TEMPERATURE INDICATING CRAYONS OR OTHER SUITABLE MEANS.

ANCHOR ROD NOTES:

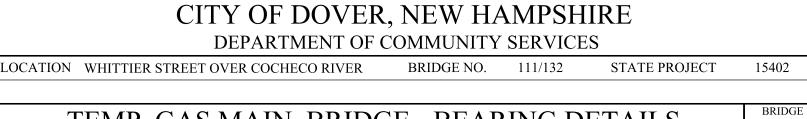
- 1. ANCHOR RODS SHALL MEET THE REQUIREMENTS OF ASTM F1554, GRADE 55 AND SHALL BE SWEDGED ON THE EMBEDDED PORTION OF THE ROD.
- 2. ANCHOR RODS, WASHERS, AND NUTS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153.
- 3. LEAVE NUTS 1/4" ABOVE WASHERS TO ALLOW FOR GIRDER ROTATION. UPSET THE THREADS ON THE ANCHOR RODS AFTER THE TEMPORARY PLATE WASHER HAS BEEN REMOVED TO PREVENT NUT MOVEMENT. TOUCH UP DAMAGED GALVANIZED SURFACES WITH ZINC-RICH PAINT AS DIRECTED BY THE RESIDENT.
- 4. ANCHOR RODS SHALL BE SET BY TEMPLATE PRIOR TO POURING THE PIER.

STEEL BEARING NOTES:

- 1. SOLE PLATES AND MASONRY PLATES SHALL MEET THE REQUIREMENTS OF ASTM A709, GRADE 50W.
- 2. ROLLERS SHALL MEET THE REQUIREMENTS OF ASTM A668/A668M CLASS F OR CLASS G.
- 3. THE MASONRY PLATE AND ROLLER SHALL BE SMOOTH AND FREE OF NICKS, BURRS, GOUGES, AND DEBRIS BEFORE INSTALLATION.
- 4. MASONRY PLATE AND ROLLER SHALL BE LUBRICATED WITH A SILICONE GREASE WHICH SATISFIES SOCIETY OF AUTOMOTIVE
- ENGINEERS SPECIFICATION SAE-AS8660. 5. BEARINGS SHALL BE CHECKED TO ENSURE THEY ARE PROPERLY FUNCTIONING EVERY THREE MONTHS.



VIEW A-A



BRIDGE SHEET TEMP. GAS MAIN BRIDGE - BEARING DETAILS A5 OF 5 JM/DPD 1/29 CHECKED DESIGNED TSB | 1/29 FILE NUMBER DRAWN TSB | 1/29 DPD 1/29 CHECKED **QUANTITIES** CHECKED FEDERAL PROJECT NO. TOTAL SHEETS ISSUE DATE SHEET NO. DRAWING NAME SHEET SCALE X-A002(794) A5 BEARING DETAILS AS NOTED REV. DATE

